GENERAL:

1. ALL MATERIALS, CONSTRUCTION, AND DETAILS SHALL CONFORM WITH THE FOLLOWING:
   PLANS AND SPECIFICATIONS
   INTERNATIONAL BUILDING CODE-2009 WITH SEPTEMBER 1, 2011 WISCONSIN
   AMENDED I-CODE INSERTS
   OSHA REGULATIONS
2. THE GENERAL CONTRACTOR AND SUBCONTRACTORS SHALL BE FAMILIAR WITH THE ENTIRE SET OF CONSTRUCTION DOCUMENTS (ARCHITECTURAL, CIVIL, ELECTRICAL, PLUMBING, STRUCTURAL, ETC.) IN ORDER TO PROVIDE ALL CONSTRUCTION AND MATERIALS FOR THIS PROJECT.
3. THE CONTRACTOR SHALL REFER TO OTHER DRAWINGS CONTAINED IN THE CONSTRUCTION DOCUMENTS FOR ADDITIONAL SPECIFIED MEMBERS, DIMENSIONS, ELEVATIONS, DETAILS, OPENINGS, INSERTS, SLEEVES, DEPRESSIONS, ETC. NOT SHOWN ON THE STRUCTURAL DRAWINGS REQUIRED TO CONSTRUCT THIS PROJECT.
4. DETAILS SHOWN ON STRUCTURAL DRAWINGS SHALL BE APPLICABLE TO ALL PORTIONS OF THE CONTRACT DOCUMENTS UNLESS NOTED OTHERWISE.
5. NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS.
6. DO NOT SCALE PLANS.
7. IN NO CASE SHALL STRUCTURAL ALTERATIONS OR WORK AFFECTING A STRUCTURAL MEMBER BE MADE UNLESS APPROVED BY THE STRUCTURAL ENGINEER.
8. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND CONSTRUCTION SEQUENCE IN ORDER TO ENSURE THE SAFETY OF THE BUILDING AND WORKMEN DURING CONSTRUCTION (MEANS & METHODS OF CONSTRUCTION). THIS INCLUDES, BUT IS NOT LIMITED TO: SHORING, UNDERPINNING, TEMPORARY BRACING, ETC.
9. CONSTRUCTION DOCUMENTS SHOW DIMENSIONS AND ELEVATIONS TO SIGNIFICANT WORKING POINTS (COLUMN CENTERLINES, OUTSIDE FACE OF WALLS, TOP OF FRAMING MEMBERS, ETC.) MATERIAL SUPPLIERS AND DESIGNERS ARE RESPONSIBLE FOR ALL OTHER INFORMATION IN ORDER TO DETAIL/FABRICATE THEIR WORK. CONTACT THE ARCHITECT WITH ANY DISCREPANCIES.
10. IN THE EVENT OF ANY DISCREPANCIES BETWEEN THE STRUCTURAL DRAWINGS AND ANY OTHER PLANS CONTAINED IN THIS SET OF CONSTRUCTION DOCUMENTS, THE CONTRACTOR SHALL BRING THE DISCREPANCY TO THE ARCHITECTS ATTENTION IN WRITING IMMEDIATELY.
11. NO PROVISIONS HAVE BEEN MADE IN THE DESIGN OF THIS STRUCTURE FOR FUTURE EXPANSION.

FOUNDATION & EARTHWORK:

1. ALL EXTERIOR FOOTINGS MUST BEAR AT A MINIMUM DEPTH OF 4'-0" BELOW ADJACENT FINISH EXTERIOR GRADE.
2. DO NOT PLACE ANY FOOTINGS ON FROZEN SUBGRADE.
3. BACK FILLING SHALL BE DONE SIMULTANEOUSLY ON BOTH SIDES OF FOUNDATION WALLS.
4. DO NOT PLACE BACK FILL AGAINST BASEMENT WALLS UNTIL THE TOP AND BOTTOM OF THE WALL ARE ADEQUATELY BRACED BY THE SLAB ON GRADE AND THE FLOOR FRAMING AT THE TOP OF THE WALL.
5. REMOVE ANY EXISTING CONCRETE 2'-0" BELOW NEW CONCRETE FOOTINGS AND SLABS ON GRADE, UNLESS NOTED OTHERWISE.
6. SHORING/OR UNDERPINNING SHALL BE DESIGNED TO LIMIT HORIZONTAL AND VERTICAL MOVEMENT OF EXISTING CONSTRUCTION TO 1/4" MAXIMUM IN ANY DIRECTION.
7. CENTER PIER AND COLUMN FOOTINGS ON COLUMN CENTERLINES AND WALL FOOTINGS ON WALL CENTERLINES UNLESS SPECIFICALLY NOTED OTHERWISE.
8. ALL BACK FILL WITHIN 3'-0" OF RETAINING WALLS AND BASEMENT WALLS SHALL BE FREE DRAINING GRANULAR MATERIAL APPROVED BY A SOILS ENGINEER AND COMPACTED TO 90% STANDARD PROCTOR.
9. TOP OF FOOTING ELEVATIONS SHOWN ON THESE CONSTRUCTION DOCUMENTS REPRESENT MINIMUM FOOTING DEPTHS FOR FROST PROTECTION AND BEST JUDGMENT OF A SUITABLE BEARING STRATUM. ACTUAL GRADE CONDITIONS AND SUITABLE BEARING STRATUM MUST BE VERIFIED BY THE CONTRACTOR AND A SOILS ENGINEER AT THE TIME OF EXCAVATION.
10. FOOTING EXCAVATIONS MUST EXTEND TO COMPETENT BEARING MATERIAL. CONTRACTOR SHALL HIRE A SOILS ENGINEER TO FIELD VERIFY NET ALLOWABLE SOIL BEARING CAPACITY STATED ON THESE CONSTRUCTION DOCUMENTS AND IN GEOTECHNICAL REPORT FOR THIS PROJECT. IF SUITABLE BEARING STRATUM DOES NOT EXIST AT FOOTING ELEVATIONS STATED ON CONSTRUCTION DOCUMENTS, EXCAVATIONS SHALL BE EXTENDED UNTIL SOIL WITH STATED BEARING CAPACITY IS REACHED. PLACE COMPACTED FILL BELOW FOOTINGS OR EXTEND FOOTINGS DOWN TO SUITABLE BEARING STRATUM. ENGINEERED FILL BELOW SLABS ON GRADE AND FOOTINGS SHALL BE FREE DRAINING GRANULAR MATERIAL COMPACTED TO 95% MODIFIED PROCTOR AND PLACED PER THE SOIL ENGINEERS RECOMMENDATIONS.
11. REFER TO DESIGN DATA FOR DESCRIPTION OF SOIL CONDITIONS, GEOTECHNICAL RECOMMENDATIONS, AND DESIGN VALUES.
12. WHERE NEW FOOTINGS ABUT EXISTING FOOTINGS, STEP THE NEW FOOTING AS REQUIRED TO HAVE NEW BOTT/FTG ELEVATION MATCH EXISTING BOTT/FTG ELEVATION. CONTRACTOR SHALL FIELD VERIFY EXISTING BOTT/FTG ELEVATION.

PRECAST CONCRETE:

1. PRECAST CONCRETE SUPPLIER SHALL DESIGN AND PROVIDE ALL LINTELS ACROSS OPENINGS IN NEW PRECAST CONCRETE WALL PANELS. THIS INCLUDES ANY HOT-ROLLED STEEL REQUIRED TO SUPPORT PRECAST CONCRETE MEMBERS.
2. PRECAST CONCRETE WALL PANELS SHALL BE DESIGNED TO RESIST ALL LATERAL LOADS STATED ON THE CONSTRUCTION DOCUMENTS. THIS INCLUDES ALL CONNECTIONS BETWEEN PRECAST ELEMENTS, CONNECTIONS TO THE FOUNDATION, AND CONNECTIONS TO THE ROOF & FLOORS.
3. PRECASTER SHALL DESIGN PRECAST CONCRETE TEE'S, PLANK, COLUMNS, AND WALL PANELS TO RESIST VERTICAL AND LATERAL LOADS SPECIFIED ON THE CONSTRUCTION DOCUMENTS. PRECASTER SHALL DESIGN ALL CONNECTIONS BETWEEN PRECAST ELEMENTS AND CONNECTIONS TO THE FOUNDATION TO RESIST ALL LOADS SPECIFIED.
4. PRECASTER SHALL GROUT PRECAST PLANK SOLID AND ADD STEEL REINFORCEMENT TO PRECAST PLANK AS REQUIRED TO SUPPORT ALL LOADS STATED ON THESE CONSTRUCTION DOCUMENTS.
5. PRECAST CONCRETE MEMBERS SHALL BE DESIGNED TO SUPPORT ALL VERTICAL LOADS SPECIFIED ON THESE CONTRACT DOCUMENTS. THIS INCLUDES ANY SUPERIMPOSED LOADS FROM PERMANENT EQUIPMENT (FOLDING PARTITIONS, MECHANICAL UNITS, SURGICAL LIGHTS, ETC.).
6. DESIGN PRECAST CONCRETE PLANK TO SUPPORT ALL STAIR STRINGER REACTIONS. COORDINATE STEEL STAIR CONNECTIONS TO PRECAST CONCRETE PLANK WITH STEEL SUPPLIER ON THIS PROJECT.
7. FINISH CONCRETE TOPPING ON TOP OF PRECAST CONCRETE PLANK TO A LEVEL SURFACE.
8. PLACE CONCRETE TOPPING ON TOP OF PRECAST CONCRETE PLANK BEFORE CONSTRUCTION OF ANY PARTITION WALLS.

CAST-IN-PLACE REINFORCED CONCRETE:

1. CONCRETE WORK SHALL CONFORM TO THE CURRENT EDITION OF ACI 318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" AND ACI 302 "GUIDE FOR CONCRETE FLOOR AND SLAB CONSTRUCTION".
2. CONTRACTOR SHALL ELECTRONICALLY SUBMIT STEEL REBAR SHOP DRAWINGS FOR APPROVAL PRIOR TO CONSTRUCTION. CONTRACTOR SHALL REVIEW AND STAMP ALL SHOP DRAWINGS BEFORE SUBMITTING TO THE ARCHITECT.
3. GROUT BELOW BASE PLATES AND BEARING PLATES SHALL BE NON-SHRINK, NON-METALLIC GROUT 1" THICK MINIMUM.
4. STEEL REINFORCING BARS SHALL CONFORM TO ASTM A615 (GRADE 60). DEFORMED WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.
5. CONTRACTOR SHALL PROVIDE SUITABLE WIRE SPACERS, CHAIRS, TIES, ETC FOR SUPPORTING REINFORCING STEEL IN THE PROPER POSITION WHILE PLACING CONCRETE.
6. PROVIDE (2)-#5 BARS AROUND ALL OPENINGS AND (2)-#5 BARS DIAGONALLY AT ALL OPENING CORNERS. EXTEND BARS 2'-6" PAST OPENING.
7. PROVIDE 1/2" EXPANSION JOINT MATERIAL AT INTERIOR LOCATIONS WHERE SLABS ABUT WALLS, COLUMNS, AND OTHER VERTICAL SURFACES UNLESS NOTED OTHERWISE.
8. PROVIDE A 1" CHAMFER ON EXPOSED CORNERS OF CONCRETE UNLESS NOTED OTHERWISE.
9. DO NOT PLACE CONDUITS, PIPES, DUCTS, OR FIXTURES IN STRUCTURAL CONCRETE UNLESS NOTED OTHERWISE.
10. SLEEVES, CONDUITS, OR PIPING PASSING THROUGH CONCRETE SLABS AND WALLS SHALL BE PLACED SO THAT THEY ARE NOT CLOSER THAN THREE DIAMETERS ON CENTER AND SO THAT THEY DO NOT DISPLACE REINFORCING.
11. CONTROL JOINTS SHALL BE PLACED IN SLAB ON GRADE AND SLAB ON METAL DECK CONSTRUCTION WITHIN 24 HOURS OF INITIAL POUR. REFER TO PLAN NOTES FOR ADDITIONAL INFO.
12. CONTRACTOR SHALL BE RESPONSIBLE FOR REPAIR OF ANY IRREGULARITIES OR DEFECTS IN CONCRETE SLABS (CRACKS, BUMPS, FLOOR CURLING, ETC.) BEFORE ANY FLOOR FINISHES ARE APPLIED.
13. REFER TO REINFORCEMENT DEVELOPMENT AND LAP SPLICE SCHEDULE FOR LAP SPLICES IN REINFORCING STEEL.
14. ALL LAPS IN REINFORCING STEEL SHALL BE CLASS "B" LAP SPLICES UNLESS OTHERWISE NOTED.
15. CONTRACTOR SHALL HIRE A MATERIALS TESTING LABORATORY TO CAST AND TEST CONCRETE CYLINDERS. ALL TESTING SHALL BE IN ACCORDANCE WITH ACI 318-05 SECTION 5.6. RESULTS OF CYLINDER TESTS SHALL BE SUBMITTED TO THE ARCHITECT. CONCRETE TEST REPORTS SHALL STATE THE FOLLOWING INFORMATION:
    LOCATION ON PROJECT WHERE THE CONCRETE IS USED
    7 DAY COMPRESSIVE STRENGTH
    28 DAY COMPRESSIVE STRENGTH
    AIR CONTENT
    SLUMP
    AMOUNT OF WATER ADDED ON JOB SITE
    MIX USED
16. ADDITION OF JOBSITE WATER TO CONCRETE SHALL BE PER ASTM C94.
17. TIME BETWEEN CONCRETE BATCHING AND PLACEMENT SHALL BE IN ACCORDANCE WITH ASTM C94.
18. CONCRETE TEST REPORTS SHALL DIRECTLY STATE WHETHER OR NOT THE TEST RESULT COMPLIES WITH THE CONSTRUCTION DOCUMENTS AND SPECIFICATIONS.
19. CLASS C FLY ASH OR SLAG MAY BE SUBSTITUTED FOR CEMENT ON A POUND TO POUND BASIS UP TO 10% OF THE TOTAL CEMENTITIOUS CONTENT.
20. ALL CONCRETE SLABS SHALL BE WET CURED PER ACI RECOMMENDATIONS FOR NO LESS THAN SEVEN DAYS OR AN APPROPRIATE CURING COMPOUND MAY BE APPLIED.
21. CALCIUM CHLORIDE OR ADMIXTURES CONTAINING CALCIUM CHLORIDE ARE NOT PERMITTED IN ANY CONCRETE MIX.
22. PROVIDE THE FOLLOWING CLEAR COVER DISTANCES FOR REINFORCEMENT IN CONCRETE:
    CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:_____3"
    CONCRETE EXPOSED TO EARTH OR WEATHER:
    NO. 6 THROUGH NO. 18 BARS:_____2"
    NO. 5 BAR AND SMALLER:_____1 1/2"
    CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:
    SLABS, WALLS, JOISTS: NO. 11 BAR AND SMALLER:_____1"
    BEAMS AND COLUMNS:_____1 1/2"
23. CONTRACTOR SHALL USE SMOOTH FORMS FOR EXPOSED CONCRETE SURFACES. ANY CONCRETE SURFACE REPAIRS SHALL BE PERFORMED BY THE CONTRACTOR AS REQUIRED. REPAIR AND PATCH DEFECTIVE AREAS WITH PROPRIETARY PATCHING COMPOUND IMMEDIATELY AFTER REMOVAL OF FORMS.

MASONRY:

1. MASONRY CONSTRUCTION SHALL CONFORM TO THE CURRENT EDITION OF ACI 530 "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" AND ACI 530.1 "SPECIFICATION FOR MASONRY STRUCTURES".
2. PROVIDE VERTICAL WALL CONTROL JOINTS IN MASONRY WALLS AS FOLLOWS:
    10'-0" MAXIMUM FROM CORNER OF WALLS
    24'-0" o/c MAXIMUM
    AT CHANGES IN WALL HEIGHT AND THICKNESS
    AT WALLS ABUTTING COLUMNS
3. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATION OF ALL VERTICAL CONTROL JOINTS IN EXTERIOR WALLS.
4. ALL MASONRY (BOND BEAMS, PILASTERS, LINTELS) SHALL BE GROUTED SOLID WITH CONCRETE HAVING f'c=3000 PSI UNLESS NOTED OTHERWISE. COARSE AGGREGATE SHALL BE PEA GRAVEL.
5. LAP VERTICAL WALL REINFORCEMENT A MINIMUM OF 40 BAR DIAMETERS OR 12", WHICH EVER IS GREATER.
6. HORIZONTAL REINFORCING AND BOND BEAM REINFORCING AT CORNERS SHALL BE LAPPED A MINIMUM OF 48 BAR DIAMETERS OR 24", WHICH EVER IS GREATER.
7. CLEANOUTS SHALL BE PROVIDED IN THE BOTTOM COURSE OF GROUTED CELLS WHERE WALL CONSTRUCTION EXCEEDS 5'-0" ABOVE BOTTOM OF INTENDED GROUT LIFT.
8. GROUT LIFTS SHALL NOT EXCEED 5'-0". CONSOLIDATE GROUT AT TIME OF PLACEMENT.
9. FACE SHELLS AND WEBS SHALL BE FULL-BEDDED IN THE STARTING COURSE ON FOUNDATIONS AND IN ALL COURSES OF PIERS AND PILASTERS.
10. PROVIDE STANDARD (W1.7) HORIZONTAL JOINT REINFORCING AT 16" o.c. VERTICALLY (8" o.c. IN PARAPET WALLS) UNLESS NOTED OTHERWISE.
11. SOLID OR SOLID-GROUTED CMU SHALL BE PROVIDED IN COURSES IMMEDIATELY ABOVE AND BELOW ANY CHANGES IN WYTHE THICKNESS.
12. CONTRACTOR SHALL GROUT MASONRY SOLID AT ALL EXPANSION ANCHOR LOCATIONS.
13. ALL MASONRY WALLS SHALL BE CONSTRUCTED IN A RUNNING BOND PATTERN AS DESCRIBED BY ACI 530 UNLESS NOTED OTHERWISE ON THE CONSTRUCTION DOCUMENTS.
14. PROVIDE (1)-VERTICAL BAR AT CORNERS AND ON EACH SIDE OF CONTROL JOINTS.

STRUCTURAL STEEL:

1. DESIGN, FABRICATION, AND ERECTION SHALL CONFORM TO THE CURRENT EDITION OF AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION) "MANUAL OF STEEL CONSTRUCTION".
2. STEEL DETAILING AND CONNECTIONS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT EDITION OF AISC 360 "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS".
3. WELDING SHALL BE PERFORMED BY CERTIFIED WELDERS HOLDING CURRENT AWS CERTIFICATES IN THE TYPES OF WELDING SPECIFIED ON THESE CONSTRUCTION DOCUMENTS.
4. CONTRACTOR SHALL ELECTRONICALLY SUBMIT STEEL SHOP DRAWINGS FOR APPROVAL PRIOR TO FABRICATION. CONTRACTOR SHALL REVIEW AND STAMP ALL SHOP DRAWINGS BEFORE SUBMITTING TO THE ARCHITECT.
5. CONTRACTOR SHALL DESIGN AND PROVIDE ANY TEMPORARY BRACING OR GUYS REQUIRED TO ERECT STEEL MEMBERS. TEMPORARY BRACING SHALL BE LEFT IN PLACE UNTIL THE PERMANENT STRUCTURE IS IN PLACE AND SECURE.
6. PROVIDE 3/16" CAP PLATE AT THE ENDS OF ALL EXPOSED TUBE AND PIPE MEMBERS, UNLESS NOTED OTHERWISE.
7. STAIRS, HANDRAILS, AND GUARDRAILS SHALL BE DESIGNED BY THE STEEL SUPPLIER.
8. ALL STEEL BEAMS SHALL BE FABRICATED WITH THE NATURAL CAMBER (WITHIN MILL TOLERANCE) IN THE UPWARD VERTICAL DIRECTION.
9. THE STEEL SUPPLIER SHALL COORDINATE HIS WORK WITH THE STEEL JOIST SUPPLIER ON THE PROJECT.
10. CONNECTION SELECTION:
    10.1. FABRICATOR SHALL PROVIDE CONNECTION WHERE SPECIFIC DETAILS ARE PROVIDED (i.e. MOMENT CONNECTIONS)
    10.2. STRUCTURAL STEEL CONNECTIONS SHALL BE SELECTED OR COMPLETED BY AN EXPERIENCED STEEL DETAILER FOR ALL CONNECTIONS NOT SPECIFICALLY DETAILED.
    10.3. DETAILER SHALL USE TABLES AND SCHEMATIC INFORMATION PROVIDED IN THE DRAWINGS OR TABLES IN THE AISC STEEL CONSTRUCTION MANUAL IN THE SELECTION OR COMPLETION OF THE CONNECTIONS.
    10.4. SELECT CONNECTIONS BASED ON ASD DESIGN CRITERIA. REACTIONS SHOWN ON PLAN ARE SERVICE LOAD LEVEL, UNLESS NOTED OTHERWISE.
11. ALTERNATE CONNECTIONS FROM WHAT IS SPECIFIED ON THE CONSTRUCTION DOCUMENTS WILL NOT BE ACCEPTED WITHOUT WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER.
12. PROVIDE STIFFENER PLATES ON BOTH SIDES OF BEAM WEBS AT ALL CONCENTRATED LOADS ABOVE AND BELOW A BEAM. UNLESS NOTED OTHERWISE, FRAME THE LARGEST BEAM OVER COLUMNS AT BEAM TO BEAM INTERSECTIONS.

STEEL JOISTS & JOIST GIRDERS:

1. DESIGN, FABRICATION, AND ERECTION SHALL CONFORM TO THE CURRENT EDITION OF "STEEL JOIST INSTITUTE SPECIFICATIONS".
2. JOIST MANUFACTURER SHALL BE A MEMBER OF THE SJI (STEEL JOIST INSTITUTE).
3. CONTRACTOR SHALL ELECTRONICALLY SUBMIT STEEL JOIST SHOP DRAWINGS FOR APPROVAL PRIOR TO FABRICATION. CONTRACTOR SHALL REVIEW AND STAMP ALL SHOP DRAWINGS BEFORE SUBMITTING TO THE ARCHITECT.
4. PROVIDE SJI STANDARD BRIDGING AS SHOWN ON THE CONSTRUCTION DOCUMENTS OR AS REQUIRED BY DESIGN.
5. DO NOT CUT OR DRILL THROUGH ANY JOIST.
6. ALL CONCENTRATED LOADS SHALL BE APPLIED AT A JOIST PANEL POINT UNLESS SPECIFICALLY NOTED OTHERWISE.
7. JOIST MANUFACTURER SHALL DESIGN JOISTS FOR ROOF TOP UNIT LOADS AND SUSPENDED UNIT LOADS SHOWN ON CONSTRUCTION DOCUMENTS. COORDINATE EXACT LOCATION OF APPLIED LOAD WITH CONTRACTOR.
8. DESIGN JOIST, JOIST GIRDERS, AND BRIDGING TO RESIST A NET WIND UPLIFT LOAD OF 5 PSF.
9. PROVIDE CAMBER IN JOIST AS RECOMMENDED BY SJI SPECIFICATIONS.
10. JOIST SUPPLIER SHALL COORDINATE HIS WORK WITH THE STEEL SUPPLIER ON THE PROJECT.
11. DESIGN JOIST GIRDERS FOR L/240 LIVE LOAD DEFLECTION UNLESS NOTED OTHERWISE.

METAL DECK:

1. DECK, ACCESSORIES, AND ATTACHMENTS SHALL CONFORM WITH THE CURRENT EDITION OF "STEEL DECK INSTITUTE SPECIFICATIONS".
2. PROVIDE SUPPORT AT COLUMNS AS REQUIRED FOR DECK SUPPORT. PROVIDE L2x2x3/16 MINIMUM.
3. AT OPENINGS IN DECK LESS THAN 12"x12", PROVIDE A 16 GAUGE COVER PLATE FASTENED TO DECK WITH #12 TEK SCREWS.
4. AT CHANGE IN DECK DIRECTION, PROVIDE A 22 GAUGE x 12" WIDE CONTINUOUS PLATE. PROVIDE SAME PLATE AT ALL RIDGES, VALLEYS, AND HIPS BENT TO MATCH PROFILE OF ROOF.

DEFLECTION LIMITS:

ROOF MEMBERS:	LIVE	SNOW OR WIND	DEAD + LIVE OR SNOW
SUPPORTING GYPSUM BOARD CEILINGS	L/360	L/360	L/240
SUPPORTING FLEXIBLE CEILINGS	L/360	L/360	L/240
NOT SUPPORTING CEILING	L/240	L/240	L/180
SUPPORTING RIGID MATERIALS (BRICK, MASONRY, ETC)	L/600	L/600	L/600

FLOOR MEMBERS:	LIVE	SNOW OR WIND	DEAD + LIVE OR SNOW
SUPPORTING RIGID MATERIALS (BRICK, MASONRY, ETC)	L/600	N/A	L/600
SUPPORTING FLEXIBLE MATERIALS	L/360	N/A	L/240

LINTEL/HEADER/BEAM MEMBERS:	LIVE	SNOW OR WIND	DEAD + LIVE OR SNOW
SUPPORTING RIGID MATERIALS (BRICK, MASONRY, ETC)	L/600	L/600	L/600
SUPPORTING FLEXIBLE MATERIALS (EIFS, SIDING, ETC.)	L/360	L/360	L/240

EXTERIOR WALLS:	LIVE	SNOW OR WIND	DEAD + LIVE OR SNOW
WITH RIGID FINISHES (BRICK, MASONRY, ETC)	N/A	L/600	N/A
WITH FLEXIBLE FINISHES (EFIS, SIDING, ETC)	N/A	L/360	N/A

DESIGN DATA:

APPLICABLE CODES/STANDARDS:

-INTERNATIONAL BUILDING CODE-2009 WITH SEPTEMBER 1, 2011 WISCONSIN AMENDED I-CODE INSERTS
-INTERNATIONAL EXISTING BUILDING CODE-2009
-ASCE 7-05 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, ASCE/SEI 2005

STRUCTURAL DESIGN STANDARDS (DESIGN SHALL CONFORM TO THE CURRENT EDITION UNDER THE APPLICABLE CODE):

-ACI 318 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE AND COMMENTARY
-ACI 530/530.1 BUILDING CODE REQUIREMENTS AND SPECIFICATIONS FOR MASONRY STRUCTURES (AND RELATED COMMENTARIES)
-ANSI/AISC 360 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS
-AISC SEISMIC DESIGN MANUAL
-WS D1.1/D1.1M STRUCTURAL WELDING CODE-STEEL
-NDS-NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION ASD/LRFD
-NDS-NATIONAL DESIGN SPECIFICATION SUPPLEMENT, DESIGN VALUES FOR WOOD CONSTRUCTION

BUILDING CLASSIFICATION CATEGORY: _____ II

BUILDING DESIGN LOADS/CRITERIA:

DESIGN DEAD LOADS:
FIRST FLOOR DEAD LOAD _____ 110 PSF
UPPER FLOOR DEAD LOAD _____ 124 PSF
ROOF DEAD LOAD _____ 25 PSF

DESIGN LIVE LOADS:
FLOOR FRAMING (RETAIL, OFFICE, RESTAURANT, RECREATIONAL) _____ 100 PSF
FLOOR FRAMING (LIGHT STORAGE) _____ 125 PSF
STAIRWAYS, CORRIDORS, LOBBIES (OTHER AREAS) _____ 100 PSF
CATWALKS _____ 40 PSF

HANDRAIL ASSEMBLIES & GUARDS:
200LB LOAD OR 50 PLF LOAD APPLIED IN ANY DIRECTION AT TOP OF HANDRAIL ASSEMBLY OR GUARD & TO TRANSFER THIS LOAD THROUGH SUPPORTS TO THE STRUCTURE

ROOF SNOW LOADS & DESIGN DATA:
DESIGN ROOF SNOW LOAD _____ 31.0 PSF (BALANCED SNOW LOAD)
FLAT ROOF SNOW LOAD (Pf)=(0.7*Ce*Ct*Is*Pg) _____ 30.8 PSF
SNOW EXPOSURE FACTOR (Ce) _____ 1.0
SNOW LOAD IMPORTANCE FACTOR (Is) _____ 1.0
ROOF THERMAL FACTOR (Ct) _____ 1.1
GROUND SNOW (Pg) _____ 40 PSF
RAIN ON SNOW SURCHARGE _____ 0
SLOPED ROOF FACTOR (Cs) _____ 1.0

WIND DESIGN DATA:
WIND IMPORTANCE FACTOR (Iw) _____ 1.0
BASIC WIND SPEED (3-SECOND GUST) _____ 90 mph
WIND DIRECTIONALITY FACTOR (Kd) _____ 0.85
MEAN ROOF HEIGHT _____ 30 FT
WIND EXPOSURE CATEGORY _____ C
WIND EXPOSURE CLASSIFICATION _____ ENCLOSED
INTERNAL PRESSURE COEFFICIENT _____ ±0.18
BUILDING LENGTH (L) _____ 110 FT
LEAST WIDTH (B) _____ 74 FT
VELOCITY PRESSURE EXPOSURE COEFFICIENT Kh (CASE 1) _____ 0.95
VELOCITY PRESSURE EXPOSURE COEFFICIENT Kh (CASE 2) _____ 0.95
TOPOGRAPHIC FACTOR Kzt _____ 1.0
EDGE STRIP (a) _____ 7.4 FT
END ZONE (2a) _____ 14.8 FT
DESIGN PROCEDURE _____ METHOD 1 (SIMPLIFIED PROCEDURE)

WIND LOADS - COMPONENTS & CLADDING (NOMINAL WIND PRESSURES):

ROOF:	SURFACE PRESSURE (PSF)		
AREA	10 SF	50 SF	100 SF
NEGATIVE ZONE 1	-19.8 PSF	-18.6 PSF	-18.1 PSF
NEGATIVE ZONE 2	-33.3 PSF	-25.0 PSF	-21.5 PSF
NEGATIVE ZONE 3	-33.3 PSF	-25.0 PSF	-21.5 PSF
ALL POSITIVE ZONES	10.0 PSF	10.0 PSF	10.0 PSF
OVERHANG ZONE 1 & 2	-28.6 PSF	-27.4 PSF	-27.1 PSF
OVERHANG ZONE 3	-28.6 PSF	-27.4 PSF	-27.1 PSF

WALL:	SURFACE PRESSURE (PSF)		
AREA	10 SF	100 SF	500 SF
NEGATIVE ZONE 4	-19.7 PSF	-17.0 PSF	-15.1 PSF
NEGATIVE ZONE 5	-24.2 PSF	-18.9 PSF	-15.1 PSF
POSITIVE ZONE 4 & 5	18.1 PSF	15.5 PSF	13.6 PSF

PARAPET:	SOLID PARAPET PRESSURE	10 SF	100 SF	500 SF
CASE A = PRESSURE TOWARDS BUILDING	CASE A: INTERIOR ZONE	46.6 PSF	31.8 PSF	29.9 PSF
	CASE A: CORNER ZONE	46.6 PSF	31.8 PSF	29.9 PSF
CASE B = PRESSURE AWAY FROM BUILDING	CASE B: INTERIOR ZONE	-32.6 PSF	-27.2 PSF	-23.3 PSF
	CASE B: CORNER ZONE	-37.3 PSF	-29.1 PSF	-23.3 PSF

EARTHQUAKE DESIGN DATA:
OCCUPANCY CATEGORY _____ II
SEISMIC IMPORTANCE FACTOR (Ie) _____ 1
MAPPED SPECTRAL ACCELERATIONS AT SHORT PERIODS (Ss) _____ 0.036
MAPPED SPECTRAL ACCELERATIONS AT (1)-SECOND PERIODS (S1) _____ 0.021
SITE CLASSIFICATION _____ D
DESIGN SPECTRAL RESPONSE COEFFICIENT AT SHORT PERIODS (Sds) _____ 0.038
DESIGN SPECTRAL RESPONSE COEFFICIENT AT (1)-SECOND PERIODS (Sd1) _____ 0.034
SEISMIC DESIGN CATEGORY _____ A
BASIC SEISMIC-FORCE-RESISTING SYSTEM _____ ORDINARY STEEL MOMENT FRAMES
DESIGN BASE SHEAR _____ 0.011W KIPS
SEISMIC RESPONSE COEFFICIENT (Cs) _____ 0.011
SEISMIC MODIFICATION FACTOR (R) _____ 3.5
ANALYSIS PROCEDURE FOR SEISMIC DESIGN _____ EQUIVALENT LATERAL FORCE ANALYSIS
BUILDING IS IN LA CROSSE COUNTY

SOIL DESIGN VALUES:
SOIL UNIT WEIGHT _____ 110 PCF (ASSUMED)
LATERAL EARTH PRESSURE
ACTIVE (RETAINING WALLS) _____ 34 PSF/FT OF DEPTH
AT-REST (BASEMENT WALLS) _____ 53 PSF/FT OF DEPTH
PASSIVE _____ 350 PSF
COEFFICIENT OF SLIDING FRICTION _____ 0.50
SUBGRADE MODULUS _____ 150 PCI (ASSUMED
ALLOWABLE SOIL BEARING PRESSURE _____ 6,000 PSF

REFER TO SOILS REPORT NO.6934.14.WIL DATED MAY 29, 2015 PREPARED BY CHOSEN VALLEY TESTING, INC. FOR DESCRIPTION OF SOIL CONDITIONS, GEOTECHNICAL RECOMMENDATIONS, AND DESIGN VALUES

TkWA
The Kubala Washatko Architects, Inc.
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PROJECT

La Crosse Distillery

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OWNER

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CONSTRUCTION MANAGER

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p: 920.924.2900

DISTILLERY PROCESS ARCHITECT

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STRUCTURAL ENGINEER

PIERCE ENGINEERS
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Milwaukee, WI 53202
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REVISIONS

DATE
October 30, 2017

PROJECT NUMBER
216216.00

SHEET TITLE

GENERAL NOTES

SHEET NUMBER

S0.1

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PE PROJECT 17574

MATERIAL STRENGTHS:

CAST-IN-PLACE CONCRETE:

FOOTINGS
- MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS — f'c = 3000 PSI
- MAXIMUM WATER-CEMENTITIOUS RATIO — 0.59
- MAXIMUM AGGREGATE SIZE — 1 1/2"
- SLUMP LIMIT — 5" ± 1"
- AIR CONTENT — NO

FOUNDATION FROST WALLS
- MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS — f'c = 4000 PSI
- MAXIMUM WATER-CEMENTITIOUS RATIO — 0.48
- MAXIMUM AGGREGATE SIZE — 3/4"
- SLUMP LIMIT — 4" ± 1"
- AIR CONTENT — YES 4 TO 6%

EXTERIOR PIERS, WALLS, AND COLUMNS
- MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS — f'c = 4000 PSI
- MAXIMUM WATER-CEMENTITIOUS RATIO — 0.48
- MAXIMUM AGGREGATE SIZE — 3/4"
- SLUMP LIMIT — 4" ± 1"
- AIR CONTENT — YES 4 TO 6%

INTERIOR SLABS ON GRADE
- MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS — f'c = 4000 PSI
- MAXIMUM WATER-CEMENTITIOUS RATIO — 0.48
- MAXIMUM AGGREGATE SIZE — 3/4"
- SLUMP LIMIT — 4" ± 1"
- AIR CONTENT — NO

EXTERIOR SLABS ON GRADE
- MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS — f'c = 4000 PSI
- MAXIMUM WATER-CEMENTITIOUS RATIO — 0.48
- MAXIMUM AGGREGATE SIZE — 3/4"
- SLUMP LIMIT — 4" ± 1"
- AIR CONTENT — YES 4 TO 6%

SLABS ON METAL DECK
- MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS — f'c = 4000 PSI
- MAXIMUM WATER-CEMENTITIOUS RATIO — 0.48
- MAXIMUM AGGREGATE SIZE — 3/4"
- SLUMP LIMIT — 4" ± 1"
- AIR CONTENT — NO

CONCRETE TOPPINGS
- MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS — f'c = 4000 PSI
- MAXIMUM WATER-CEMENTITIOUS RATIO — 0.48
- MAXIMUM AGGREGATE SIZE — 3/4"
- SLUMP LIMIT — 4" ± 1"
- AIR CONTENT — NO

STAIR LANDINGS & TREADS
- MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS — f'c = 4000PSI
- MAXIMUM WATER-CEMENTITIOUS RATIO — 0.48
- MAXIMUM AGGREGATE SIZE — 3/4"
- SLUMP LIMIT — 4" ± 1"
- AIR CONTENT — NO

SLURRY
- MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS — f'c = 1000 PSI
- MAXIMUM WATER-CEMENTITIOUS RATIO — 0.55
- MAXIMUM AGGREGATE SIZE — 1 1/2"
- SLUMP LIMIT — 6" ± 1"
- AIR CONTENT — NO

FIBER REINFORCEMENT:
- MACROSYNTHETIC FIBERS ENGINEERED & DESIGNED FOR USE IN CONCRETE SLABS COMPLYING WITH ASTM C 1116, TYPE III, 1 1/2" TO 2 1/2" LONG

REINFORCING STEEL:
- ALL-ASTM A 615, GRADE 60, DEFORMED — Fy = 60,000 PSI
- STEEL WELDED WIRE REINFORCEMENT, FLAT SHEETS — Fy = 60,000 PSI

STRUCTURAL STEEL:
- ROLLED WIDE FLANGE SHAPES, ASTM A 992 GRADE 50 — Fy = 50,000 PSI
- CHANNELS, ANGLES, & S SHAPES, ASTM A 36 — Fy = 36,000 PSI
- PLATE & BAR, ASTM A 36 — Fy = 36,000 PSI
- TUBE SHAPES, ASTM A 500 GRADE B — Fy = 46,000 PSI
- PIPE, ASTM A 53, TYPE E OR S, GRADE B — Fy = 46,000 PSI
- ALL OTHER ROLLED SHAPES, ASTM A 36 — Fy = 36,000 PSI

COLD-FORMED METAL FRAMING

GAUGE STEEL:
- 18 GAUGE AND LIGHTER — Fy = 33,000 PSI
- 16 GAUGE AND HEAVIER — Fy = 50,000 PSI
- ALL TRACK 16 GAUGE — Fy = 50,000 PSI

GALVANIZED COATING — G90

STRUCTURAL BOLTS:
- HIGH STRENGTH BOLTS, NUTS, & WASHERS, ASTM A 325
- ZINC-COATED HIGH STRENGTH BOLTS, NUTS, & WASHERS, ASTM A 325
- STAINLESS STEEL BOLTS, NUTS, & WASHERS, ASTM F 593
- SHEAR CONNECTORS, ASTM A 108 GRADES 1015 THRU 1020
- THREADED RODS, ASTM A 36
- CLEVISES & TURNBUCKLES, ASTM A 108, GRADE 1035
- EYE BOLTS & NUTS, ASTM A 108, GRADE 1030
- ANCHOR BOLTS, ASTM F 1554, GRADE 36

WELDED CONNECTIONS:
- WELDING ELECTRODES — E70XX; E80XX FOR WELDING REINFORCING

MASONRY: — f'm = 2000 PSI

MASONRY MORTAR:
- TYPE "M" MORTAR BELOW GRADE
- TYPE "M" OR "S" ABOVE GRADE

GROUT BELOW BASE PLATES & BEARING PLATES:
- NONMETALLIC, SHRINKAGE-RESISTANT ASTM C 1107

WALL FOOTING SCHEDULE

MARK	SIZE (WIDTH x CONT)	THICKNESS	REINFORCEMENT		REMARKS
			LONGITUDINAL	TRANSVERSE	
W16	1-6"xCONT	1'-0"	(2) #5xCONT		
W20	2'-0"xCONT	1'-0"	(2) #5xCONT		
W30	3'-0"xCONT	1'-0"	(2) #5xCONT		
TS20	2'-0"xCONT	1'-0"	(2) #5xCONT		THICKENED SLAB

WALL FOOTING SCHEDULE NOTES:
1. REFER TO STRUCTURAL NOTES SHEET FOR MINIMUM COVER REQUIREMENTS.
2. REFER TO FOUNDATION PLAN FOR TOP OF FOOTING ELEVATIONS.
3. NET ALLOWABLE SOIL BEARING CAPACITY=2000 PSF. CONTRACTOR TO HIRE SOILS ENGINEER TO FIELD VERIFY AT TIME OF FOOTING EXCAVATION.
4. ALL LAPS IN STEEL REINFORCING SHALL BE CLASS "B" LAP SPLICES UNLESS NOTED OTHERWISE.

CONCRETE PIER SCHEDULE

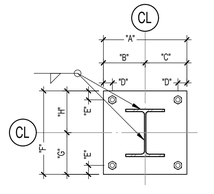
MARK	SIZE	VERTICAL REINFORCEMENT	PIER TIES	DETAIL	DOWELS	REMARKS
P1	18"x18"	(6) #6	#3 AT 12" o/c			

CONCRETE PIER SCHEDULE NOTES:
1. REFER TO PLAN FOR TOP OF CONCRETE PIER ELEVATION.
2. AT TOP OF CONCRETE PIER, PROVIDE (3) #3 TIES AT 3" o/c.
3. WHERE NO DOWELS ARE SHOWN FROM THE CONCRETE PIER TO THE CONCRETE FOOTING, EMBED VERTICAL PIER REINFORCEMENT TO BOTTOM OF FOOTING w/ 3" CONCRETE COVERAGE AND PROVIDE A STANDARD 90 DEGREE HOOK.
4. CENTER CONCRETE PIER BELOW COLUMN ABOVE UNLESS DETAILED OTHERWISE.
5. LAP VERTICAL REINFORCEMENT 30 BAR DIAMETERS OR 24", WHICH EVER IS GREATER.

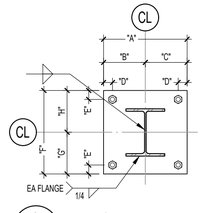
STEEL COLUMN BASE PLATE SCHEDULE

MARK	THICKNESS	ANCHOR ROD DIAMETER	ANCHOR ROD EMBEDMENT	BASE PLATE DETAIL	A (in)	B (in)	C (in)	D (in)	E (in)	F (in)	G (in)	H (in)
BP1	3/4"	3/4"	12"	1/S002	18"	9"	9"	1 1/2"	1 1/2"	18"	9"	9"
BP2	1/2"	3/4"	9"	1/S002	14"	7"	7"	1 1/2"	1 1/2"	14"	7"	7"

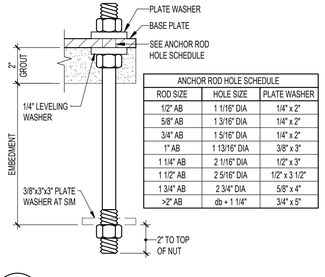
BASE PLATE SCHEDULE NOTES:
1. PROVIDE ANCHOR BOLTS w/ A NUT & WASHER ABOVE BASE PLATE, LEVELING NUT & WASHER BELOW BASE PLATE, & A NUT TACK WELDED AT EMBEDDED END UNLESS NOTED OTHERWISE. SETTING PLATES USED AT FABRICATOR'S OPTION.
2. PROVIDE A 2" MIN THICK NON-SHRINK, NON-METALLIC GROUT BED BELOW BASE PLATE UNLESS NOTED OTHERWISE.
3. CENTER STEEL COLUMNS ON BASE PLATES AND PIERS/FOOTINGS, UNLESS NOTED OTHERWISE.
4. MILL TOP OF BASE PLATES PER AISC 360-05, SEC M2.8 IF FLATNESS TOLERANCE IS EXCEEDED.
5. GROUT BELOW BASE PLATE TO HAVE MINIMUM COMPRESSIVE STRENGTH OF 8,000 psi.



3 S0.2 WF BASE PLATE DETAIL



2 S0.2 WF BASE PLATE DETAIL



ANCHOR ROD HOLE SCHEDULE

ROD SIZE	HOLE SIZE	PLATE WASHER
1/2" AB	1 1/16" DIA	1/4" x 2"
5/8" AB	1 3/16" DIA	1/4" x 2"
3/4" AB	1 5/16" DIA	1/4" x 2"
1" AB	1 13/16" DIA	3/8" x 3"
1 1/4" AB	2 1/16" DIA	1/2" x 3"
1 1/2" AB	2 5/16" DIA	1/2" x 3 1/2"
1 3/4" AB	2 3/4" DIA	5/8" x 4"
>2" AB	db + 1 1/4"	3/4" x 5"

1 S0.2 ANCHOR ROD DETAIL

COLUMN FOOTING SCHEDULE

MARK	SIZE (WIDTH x LENGTH)	THICKNESS	BOTTOM REINFORCEMENT		TOP REINFORCEMENT		REMARKS
			LONG	SHORT	LONG	SHORT	
F30	3'-0"x3'-0"	1'-0"	(3) #5	(3) #5			
F40	4'-0"x4'-0"	1'-0"	(4) #5	(4) #5			
F4660	4'-6"x6'-0"	1'-0"	(5) #5	(5) #5			
F50	5'-0"x5'-0"	1'-6"	(5) #5	(5) #5			

COLUMN FOOTING SCHEDULE NOTES:
1. REFER TO STRUCTURAL NOTES SHEET FOR MINIMUM COVER REQUIREMENTS.
2. REFER TO FOUNDATION PLAN FOR TOP OF FOOTING ELEVATIONS.
3. NET ALLOWABLE SOIL BEARING CAPACITY=2000 PSF. CONTRACTOR TO HIRE SOILS ENGINEER TO FIELD VERIFY AT TIME OF FOOTING EXCAVATION.
4. ALL LAPS IN STEEL REINFORCING SHALL BE CLASS "B" LAP SPLICES UNLESS NOTED OTHERWISE.

TkWA

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PROJECT

La Crosse Distillery

129 Vine Street
La Crosse, WI 54601

OWNER

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CONSTRUCTION MANAGER

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REVISIONS

DATE

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SHEET TITLE

STRUCTURAL SCHEDULES AND NOTES

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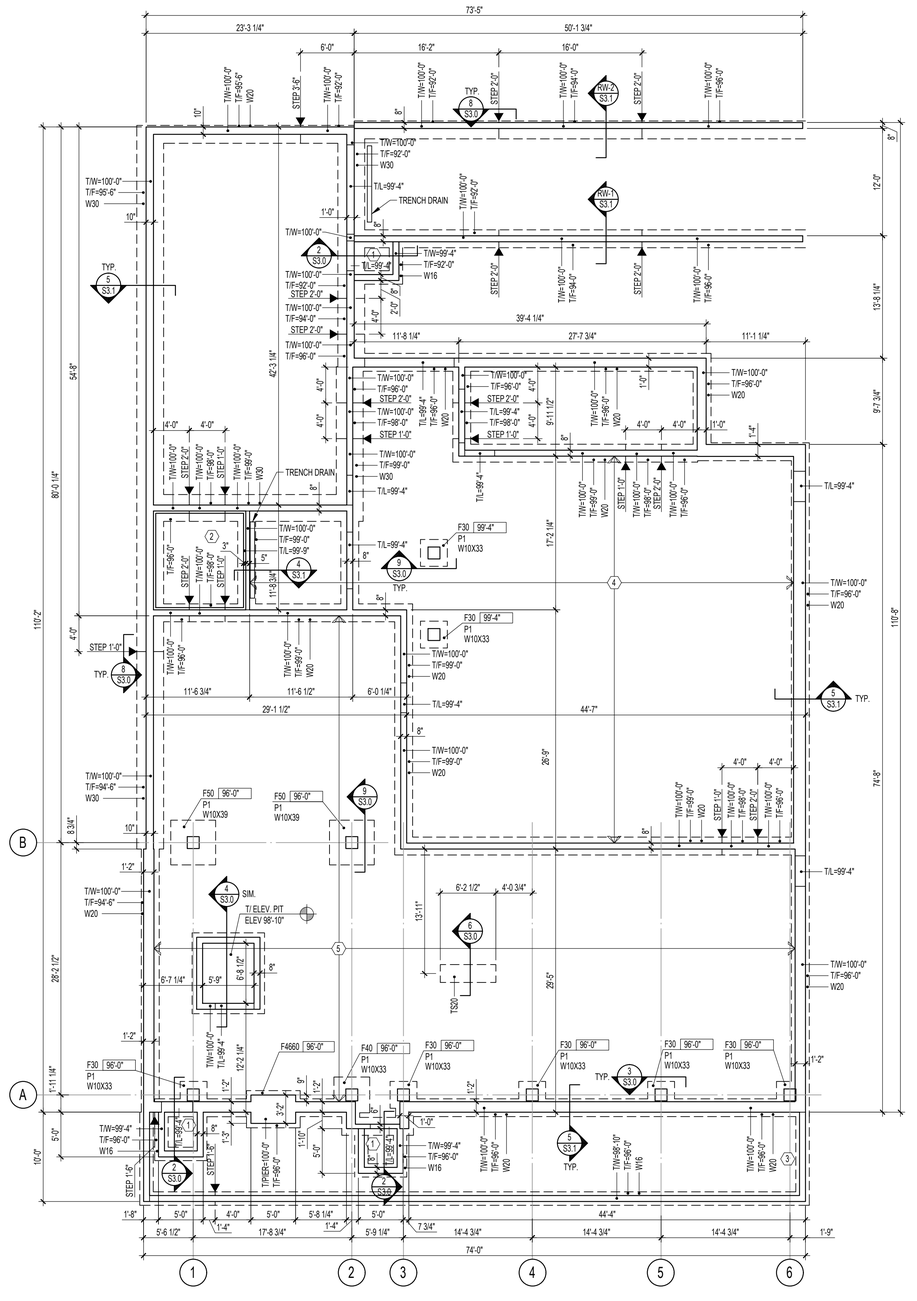

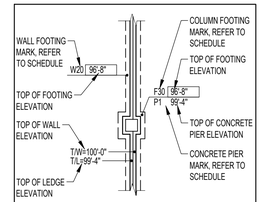

FOUNDATION LEGEND

FOUNDATION PLAN NOTES:

1. REFER TO GENERAL NOTES FOR ADDITIONAL STRUCTURAL NOTES AND FOUNDATION REQUIREMENTS.
2. ELEVATION 100'-0" ON STRUCTURAL DRAWINGS CORRESPONDS TO FF ELEVATION SHOWN ON SITE PLAN, TYPICAL.
3. SLAB DEPRESSIONS: VERIFY ALL SLAB DEPRESSIONS (SIZE, DEPTH, LOCATION) w/ ARCHITECTURAL DRAWINGS.

FOUNDATION PLAN KEYED NOTES:

1. 5" THICK REINFORCED STRUCTURAL CONCRETE STOOP SLAB w/ REINFORCING PER DETAILS. LOCATE REINFORCEMENT 1-1/2" FROM BOTTOM OF SLAB.
2. DEPRESSED SLAB, T/SLAB=98'-0". SLOPE SLAB TO DRAIN. GRATING SUPPORT TO BE COORD. w/ SUPPLIER.
3. FROST WALL AT PATIO SLAB OVER POUR FOR EDGE. STEPS TO SIDEWALK, COORD. w/ CIVIL. T/WALL AT 98'-0
4. 8" CONCRETE SLAB ON GRADE w/(4) #4T 12"o/c ON 6" FREELY DRAINING SUB-BASE w/ VAPOR BARRIER. REFER TO FOUNDATION PLAN NOTES FOR CONTROL JOINT REQUIREMENTS. FINISH FLOOR EL = 100'-0" SLOPE SLAB TO DRAIN PER EQUIPMENT LAYOUT REQUIREMENTS.
5. 4" CONCRETE SLAB ON GRADE w/ 6x6 - W1.4xW1.4 WWF ON 6" FREELY DRAINING SUB-BASE w/ VAPOR BARRIER. REFER TO FOUNDATION PLAN NOTES FOR CONTROL JOINT REQUIREMENTS. FINISH FLOOR EL = 100'-0"

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FOUNDATION FRAMING PLAN

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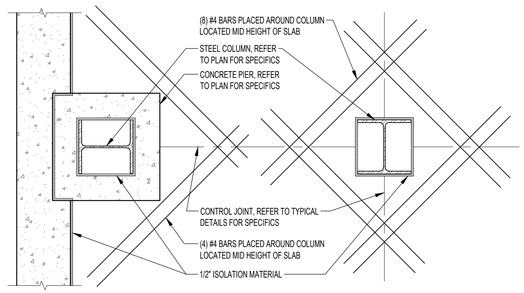
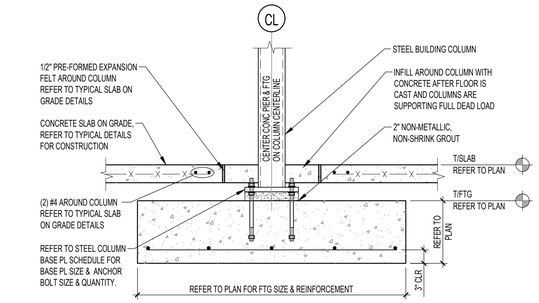
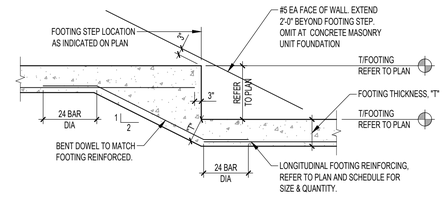


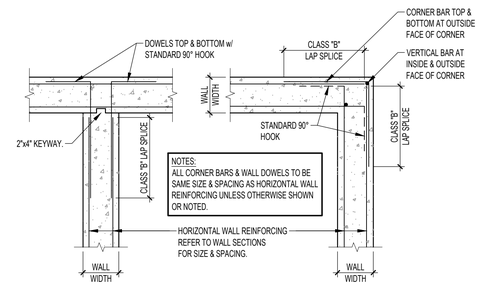
10 S3.0 PLAN AT EXTERIOR COLUMN
PLAN AT INTERIOR COLUMN



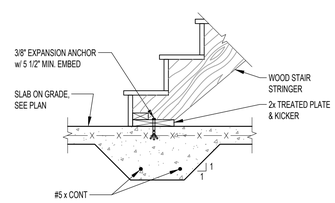
9 S3.0 STEEL COLUMN SETTING ON INTERIOR CONCRETE FOOTING WITH BOX-OUT ISOLATION JOINT



8 S3.0 STRIP FOOTING STEP DETAIL

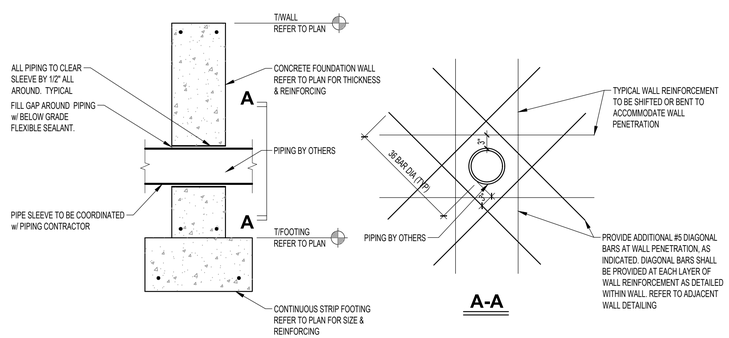


7 S3.0 TYPICAL CORNER AND WALL INTERSECTION REINFORCEMENT

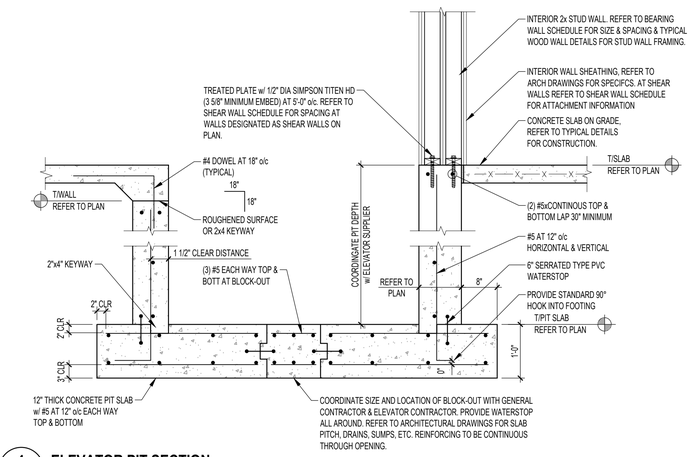


NOTES:
1. REFER TO ARCHITECTURAL DRAWINGS FOR STAIR LOCATIONS.

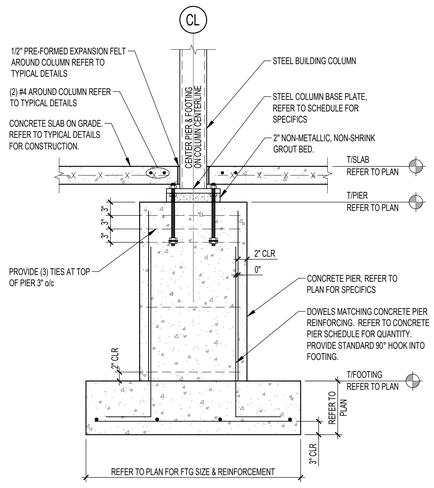
6 S3.0 THICKENED SLAB AT STAIR STRINGER



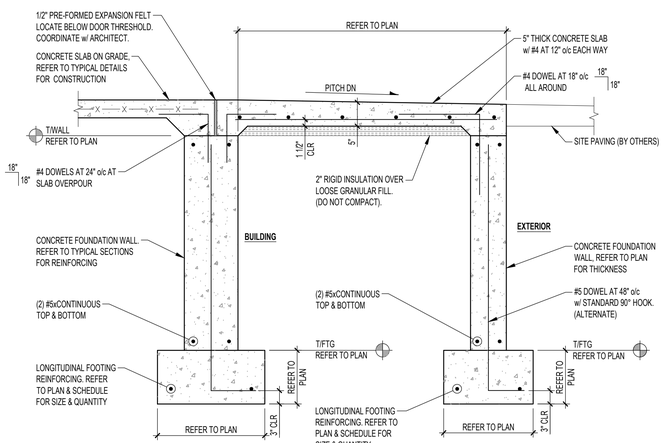
5 S3.0 CONCRETE FOUNDATION WALL REINFORCEMENT AT PIPE PENETRATION (24" MAX PIPE DIAMETER)



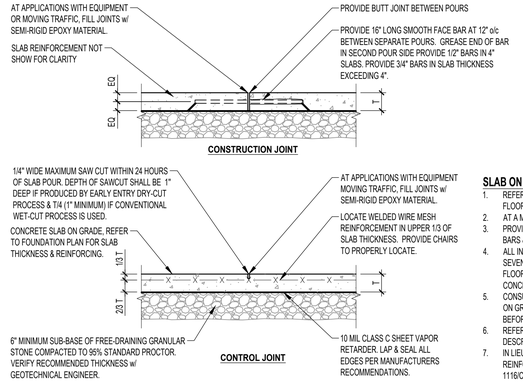
4 S3.0 ELEVATOR PIT SECTION



3 S3.0 STEEL COLUMN SETTING ON CONCRETE PIER



2 S3.0 REINFORCED CONCRETE STOOP SLAB



SLAB ON GRADE CONSTRUCTION NOTES:

1. REFER TO ARCHITECTURAL DRAWINGS FOR ALL FLOOR COATINGS & FINISHES ON SLAB ON GRADE. CONFIRM ALL FLOOR FINISHES WITH GENERAL CONTRACTOR & OWNER BEFORE APPLICATION.
2. AT A MINIMUM FINISH ALL FLOOR SLABS ON GRADE TO A FF25/FL20 CRITERIA (FLOOR CLASSIFICATION 2).
3. PROVIDE BOLSTERS, CHAIRS, SPACERS, & OTHER DEVICES FOR SPACING, SUPPORTING, & FASTENING REINFORCING BARS & WELDED WIRE MESH REINFORCEMENT IN PLACE AS REQUIRED.
4. ALL INTERIOR CONCRETE SLABS ON GRADE SHALL BE CURED PER ACI RECOMMENDATIONS FOR NO LESS THAN SEVEN DAYS. ALTERNATE CURING PROCESSES/METHODS MAY BE CONSIDERED DEPENDING DEPENDING UPON FLOOR FINISHES. CONSULT WITH ARCHITECT, ENGINEER, FLOOR COVERING MANUFACTURER, & OWNER BEFORE CONCRETE SLAB IS POURED.
5. CONSULT ARCHITECT, ENGINEER, & FLOOR COVERING MANUFACTURER AT ALL LOCATIONS WHERE CONCRETE SLABS ON GRADE WILL RECEIVE MOISTURE-SENSITIVE FLOORING MATERIALS (EPOXY FLOORS, POLISHED FLOORS, ETC.) BEFORE CONCRETE SLAB IS POURED.
6. REFER TO PROJECT SOILS REPORT FOR ALL SUB-BASE PREPARATION REQUIREMENTS & RECOMMENDATIONS & FOR DESCRIPTION OF EXISTING SOIL CONDITIONS.
7. IN LIEU OF STEEL WIRE MESH REINFORCEMENT, CONTRACTOR MAY PROVIDE MACROSYNTHETIC FIBER REINFORCEMENT ENGINEERED & DESIGNED FOR USE IN CONCRETE SLABS ON GRADE COMPLYING WITH ASTM C 1116/C 1116M TYPE III, 1 1/2" TO 2 1/2" LONG. STRUCTURAL ENGINEERS PRIOR APPROVAL IS REQUIRED.

1 S3.0 SLAB ON GRADE CONSTRUCTION DETAILS

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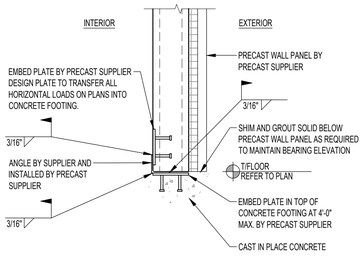
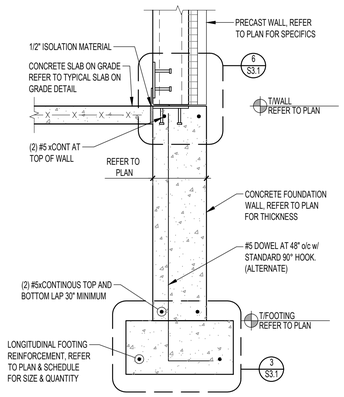
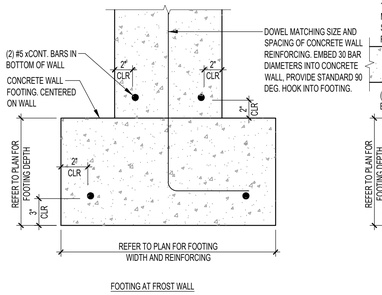


6 S3.1 PRECAST WALL PANEL CONNECTION DETAIL

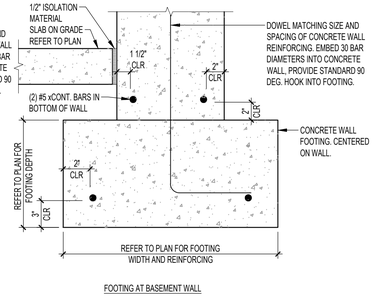


5 S3.1 EXTERIOR FOUNDATION WALL



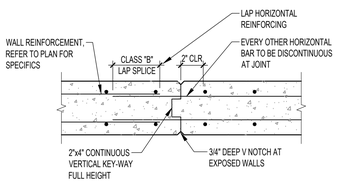
FOOTING AT FROST WALL

4 S3.1 EXTERIOR FOUNDATION WALL



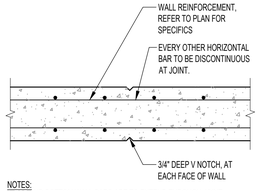
FOOTING AT BASEMENT WALL

3 S3.1 TYPICAL WALL FOOTINGS



NOTES:
1. TOP AND BOTTOM BARS ARE TO EXTEND THROUGH JOINT.
2. PROVIDE AT END OF EACH WALL POUR.

2 S3.1 WALL CONSTRUCTION JOINT



NOTES:
1. TOP AND BOTTOM WALL BARS ARE TO EXTEND THROUGH JOINT.
2. CONTROL JOINT SPACING SHALL BE 25'-0" MAXIMUM.

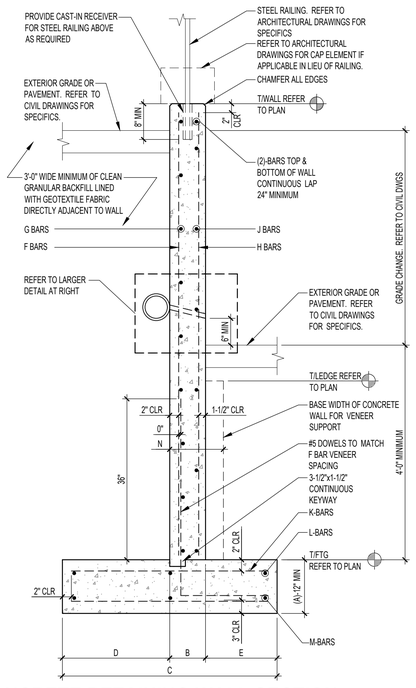
1 S3.1 WALL CONTROL JOINT

CONCRETE RETAINING WALL SCHEDULE

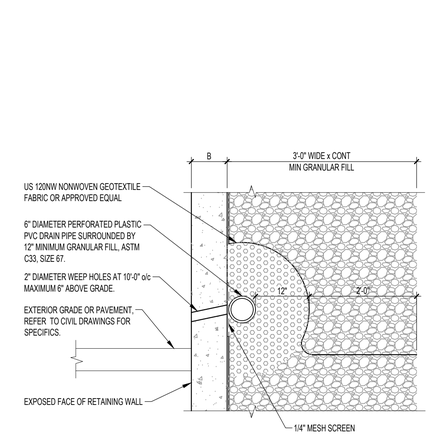
MARK	FOOTING DEPTH (A)	STEM THICKNESS (B)	FOOTING WIDTH (C)	HEEL WIDTH (D)	TOE WIDTH (E)	(F) BARS	(G) BARS	(H) BARS	(J) BARS	(K) BARS	(L) BARS	(M) BARS	THICKNESS AT STEM BASE (N)	REMARKS
RW-1	1'-2"	1'-0"	5'-0"	2'-0"	2'-0"	#5 AT 12" o/c	#5 AT 18" o/c	#5 AT 12" o/c	#5 AT 18" o/c	#5 AT 12" o/c	#5 AT 12" o/c	(4) #5	1'-0"	
RW-2	1'-6"	1'-0"	5'-0"	0'-2"	3'-10"	#5 AT 12" o/c	#5 AT 18" o/c	#5 AT 12" o/c	#5 AT 18" o/c	#6 AT 12" o/c	#6 AT 12" o/c	(4) #5	1'-0"	

SCHEDULE NOTES:

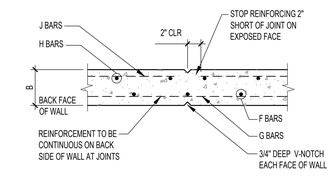
1. REFER TO GENERAL NOTES ON SHEET XXX FOR ADDITIONAL INFORMATION & CRITERIA.
2. REFER TO FOUNDATION PLAN FOR TOP OF FOOTING & TOP OF WALL ELEVATIONS.
3. NET ALLOWABLE SOIL BEARING CAPACITY = 2000 PSF. CONTRACTOR MUST HIRE A SOILS ENGINEER TO FIELD VERIFY AT TIME OF FOOTING EXCAVATION.
4. F'c (WALL) = 4000 PSI.
5. F'c (FOOTING) = 3000 PSI.
6. MAXIMUM WATER/CEMENT RATIO FOR EXPOSED CONCRETE = 0.42.
7. CONCRETE EXPOSED TO EXTERIOR CONDITIONS SHALL BE AIR-ENTRAINED 4%-6%.
8. ALL LAPS IN STEEL REINFORCING SHALL BE CLASS "B" LAP SPLICES UNLESS NOTED OTHERWISE.
9. CONTRACTOR SHALL USE SMOOTH FACE FORMS FOR ALL EXPOSED CONCRETE WALL SURFACES. ANY CONCRETE SURFACE REPAIRS SHALL BE PERFORMED BY THE CONTRACTOR AS REQUIRED. REPAIR AND PATCH DEFECTIVE AREAS WITH PROPRIETARY PATCHING COMPOUND IMMEDIATELY AFTER REMOVAL OF FORMS.
10. PROVIDE WALL CONTROL JOINTS AT 25'-0" ON CENTER MAXIMUM. REFER TO DETAIL BELOW.
11. PROVIDE WALL EXPANSION JOINTS AT 100'-0" ON CENTER MAXIMUM. REFER TO DETAIL BELOW.
12. PROVIDE WALL CONSTRUCTION JOINTS AT THE END OF EACH WALL POUR. REFER TO DETAIL BELOW.



CONCRETE RETAINING WALL SECTION

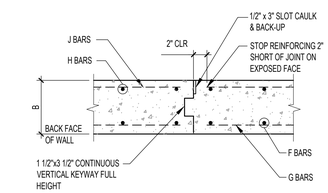


TYPICAL WEEP HOLE DETAIL



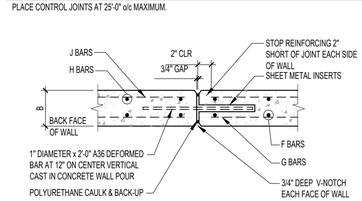
RETAINING WALL CONTROL JOINT

PLACE CONTROL JOINTS AT 25'-0" o/c MAXIMUM.



RETAINING WALL CONSTRUCTION JOINT

PROVIDE AT THE END OF EACH WALL POUR



RETAINING WALL EXPANSION JOINT

PLACE EXPANSION JOINTS AT 100'-0" o/c MAXIMUM.

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REVISIONS

DATE

October 30, 2017

PROJECT NUMBER

216216.00

SHEET TITLE

FOUNDATION DETAILS

SHEET NUMBER

S3.1

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