# City of La Crosse, Wisconsin

City Hall
400 La Crosse Street
La Crosse, WI 54601

## Meeting Agenda

## Commercial/Multi-Family Design Review Committee

**Friday, November 7, 2025** | **9:00 AM** | **Virtual via Zoom**

The meeting is conducted through video conferencing.
Members of the public will be able to attend the meeting via video conferencing with the link below.

Join Zoom Meeting
Click this link (or typing the URL in your web browser address bar):
https://cityoflacrosse-org.zoom.us/j/82799188943?pwd=pAMS3MbJusyBqR9mjCiK3jH6cAP0rk.1

Meeting ID: 827 9918 8943
Passcode: 212646

Dial by your location
1 312 626 6799

If you wish to speak please provide written comments by emailing acklint@cityoflacrosse.org, using a drop box outside of City Hall or mailing the Department of Planning, Development, and Assessment at 400 La Crosse St, WI 54601

### Approval of Minutes

**1.** 25-0625 Review of plans for the new building located at 807 East Ave. (New Elementary School- Hogan Site)

***Attachments:*** Final Application 11-7-25
Final Renderings 11-7-25
Final Civil Plans 11-7-25
Final Stormwater Permit App 11-7-25
Final Stormwater Plan 11-7-25
Final Photometric 11-7-25
Revised Prelim Design Review Memo 9-26-2025
Revised Civil Plans 9-26-25
Revised Floor Plans/Elevations 9-26-25
Preliminary Design Review Memo 5-30-25
Preliminary Plans 5-30-25
Final Elevations 11-7-25

### Agenda Items:

### Adjournment

*Notice is further given that members of other governmental bodies may be present at the above scheduled meeting to gather information about a subject over which they have decision-making responsibility.*

*NOTICE TO PERSONS WITH A DISABILITY*
*Requests from persons with a disability who need assistance to participate in this meeting should call the City Clerk's office at (608) 789-7510 or send an email to ADAcityclerk@cityoflacrosse.org, with as much advance notice as possible.*

# City of La Crosse, Wisconsin

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400 La Crosse Street
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## Text File

### File Number: 25-0625

**Agenda Date:** 5/30/2025 **Version:** 1 **Status:** Agenda Ready

**In Control:** Commercial/Multi-Family Design Review Committee **File Type:** Review of Plans

**Agenda Number:** 1.

# COMMERCIAL DEVELOPMENT DESIGN STANDARDS APPLICATION

Planning Department · Phone 608.789.7512 · Fax 608 789.7318
http://www.cityoflacrossse.org Planning@cityoflacrosse.org

| | |
|---|---|
| Permit No.: | |
| Date: | |
| STATUS: | Parcel No.: |

**OWNER**

| | | | |
|---|---|---|---|
| Name: *SCHOOL DISTRICT OF LA CROSSE - JOE LEDVINA, DIRECTOR OF FACILITIES* | | | |
| Address: *807 EAST AVENUE SOUTH* | | | |
| City: *LA CROSSE* | | | |
| Phone: *608-789-7627* | Cell: | Fax: *608-789-8479* | E-mail: *jledvina@lacrosse.org* |

**ARCHITECT CONTRACTOR**

| | | | |
|---|---|---|---|
| Name: *POINT OF BEGINNING, INC.* | | | |
| Address: *4941 KIRSCHLING COURT* | | | |
| City: *STEVENS POINT, WI 54481* | | | |
| Phone: *715-344-9999* | Cell: | Fax: *715-344-9922* | E-mail: *melissak@pobinc.com* |

**PROJECT**

Check One: ☑ Building ☐ Addition ☐ Alteration/Remodel

Description of Work:

*The proposed project consists of demolition of the existing Hogan Administration Center building including all paved surfacing, infield baseball mix, gravel areas, woodchip playground areas, and landscape areas. A new elementary school building is to be constructed including paved parking lot, concrete sidewalks, hard surface playground areas, rubber surface play areas, woodchip play areas, and landscape/turf grass areas. New sanitary, storm sewer, and sanitary sewer laterals will be installed to service the new building. Additionally, the site will be graded to ensure proper drainage, and one new underground stormtech chamber system will be installed to control and treat storm water runoff.*

Pre-application Meeting Date: *MAY 30, 2025 & SEPTEMBER 26,2025*

Applying for Exception: ☑ No ☐ Yes (Include $300 Check for Public Notification)

**PROPERTY**

| | | |
|---|---|---|
| Project Address: *807 EAST AVENUE SOUTH* | | |
| Zoning District: *PS- PUBLIC & SEMI-PUBLIC* | Parcel Number: *17-30206-10, 17-50250-60, 17-50250-61, 17-30230-120, 17-50250-60, & 17-50241-70* | |
| Address: | Address same as property owner's address: ☑ | |
| City: | State: | Zip Code: |

**OFFICIAL USE ONLY**

Date Received: Review Date:

Exception Check: ☐ Yes ☐ No

Required Information: ☐ Site Plan ☐ Architecture Plan ☐ Landscape Plan ☐ Building Elevations & Materials ☐ Exterior Light Diagram ☐ LEED Checklist ☐ Photos

The applicant agrees that all design aspects and maintenance plans are in accordance with the requirements of Section 15.47 of the Code of Ordinances for the City of La Crosse. Application, the checklist, and seven (7) sets of required information must be submitted to the City Inspection Department prior to review and acceptance.

*Melissa Kluck*

(PRINT) Architect/Engineer Name

Melissa Kluck Digitally signed by Melissa Kluck DN: C=US, E=melissak@pobinc.com, CN=Melissa Kluck Date: 2025.10.21 13:51:21 -05'00'

Signature (Architect/Engineer) Date

*Joe Ledvina*

(Print) Owner Name

*10-22-25*

Signature (Owner) Date

# DESIGN REVIEW CHECKLIST

*The checklist must be completed in full by the applicant prior to submission. Completed elements should be checked. Any elements that do not apply to your site or you are requesting an exception on, check the corresponding column and include notes. Items in italics are recommended actions but not required.*

| | | YES | NO | N/A | NOTES |
|---|---|---|---|---|---|
| PARKING LOT DESIGN AND PARKING STANDARDS | | | | | |
| C.2 | No parking stall may be closer to the street than the building setback line or the building on the same parcel, whichever is farther from the street unless the applicant can demonstrate that there are no practical alternatives related specifically to the site. | ☑ | ☐ | ☐ | |
| C.3 | All points of ingress and egress will be evaluated by the City Traffic Engineer to determine if ingress and egress should be allowed directly to the street or via an alley. | ☑ | ☐ | ☐ | |
| C.4 | Parking areas shall be separated from primary buildings by a landscaped buffer. | ☑ | ☐ | ☐ | |
| C.5 | Minimum setback for parking stalls and drives is five (5) feet from all property lines with the exception of the alley (in order to accommodate landscaping or drainage swales). Parking for adjacent properties may be combined into continuous paved lots, eliminating the required setback at the shared property line, provided that 100% of the lost green space is replaced elsewhere on the parcel (e.g. with a 10' setback along the opposite lot line). | ☑ | ☐ | ☐ | |
| C.6 | A parking lot for more than 12 vehicles shall incorporate at least 288 square feet of planting islands at least 8 feet in width (face of curb to face of curb). Planting islands may be either parallel to parking spaces or perpendicular to the parking spaces. As parking lot size increase, and additional planting island is required at the ratio of one planting island for every 20 automobile parking spaces. No less that 5 percent of the islands shall be interior to the parking lot. | ☑ | ☐ | ☐ | |
| C.7 | Landscaping buffers, green space, and planting islands must total a minimum of 10 percent of the lot. | ☑ | ☐ | ☐ | |
| C.8 | Buffers, setbacks, and planting islands are encouraged to be used for stormwater infiltration. | ☑ | ☐ | ☐ | |
| C.9 | All approaches, parking and vehicular circulation areas shall be paved and graded for proper stormwater management. The use of pervious pavement for stormwater infiltration is highly encouraged. | ☑ | ☐ | ☐ | |
| C.10 | For structures not needing approval by the Wisconsin Department of Commerce, parking spaces shall not be less than 8.5 feet in width and 17 feet in length. The full dimensions of this rectangle must be maintained in angled parking designs. Drive aisle widths vary depending upon the angle of parking space. The following minimum standards apply and shall be consistent with requirements of the City Engineering Department adopted standards:<br>45 degrees – 12'10" aisle<br>55 degrees – 13'7" aisle<br>65 degrees – 15'4" aisle<br>75 degrees – 17'10" aisle<br>90 degrees – 22' aisle | ☑ | ☐ | ☐ | |
| C.11 | Where maximums on parking ratios exist, parking surfaces and drive aisles shall be permitted to be increased in size by no more than five percent (5%), provided at least twenty-five percent (25%) of the parking lot and pedestrian sidewalks consist of paving blocks (plastic or concrete honeycomb grid) planted with grass. | ☑ | ☐ | ☐ | |
| C.12 | Parking lots shall be located on the same lot as the principal structure (unless it can be demonstrated that shared parking will be beneficial to multiple property owners and does not result in a "gap tooth" effect on a block face). | ☑ | ☐ | ☐ | |

# DESIGN REVIEW CHECKLIST

| | | YES | NO | N/A | NOTES |
|---|---|---|---|---|---|
| C.13 | Raised curbs, parking blocks or stops, decorative bollards and/or fences, trees and/or shrubs shall be utilized along the edge(s) of parking lots to prevent motor vehicles from parking on green space buffers, outdoor recreation space, bike parking areas, sidewalks and side and front yards. In the event the original protective measures are inadequate to preventing inappropriate parking, additional measures shall be taken. | ☑ | ☐ | ☐ | |
| C.14a | Parking lot snow storage area(s) shall be designated in the parking lot and/or green space buffers. | ☑ | ☐ | ☐ | |
| C.14b | Snow storage areas shall not be located near parking lot entrances and impede driver vision. | ☑ | ☐ | ☐ | |
| C.14c | If these green space buffer(s) are no longer capable of storing snow, the property owner shall arrange for the excess snow to be removed. | ☑ | ☐ | ☐ | |
| C.14.d | To the greatest extent possible, melting snow or ice should not drain over sidewalks or across neighboring properties. | ☑ | ☐ | ☐ | |
| C.15 | Light-colored and/or reflective surface coating should be considered to reduce the "heat island" effect of traditional asphalt parking lots. | ☐ | ☑ | ☐ | |
| C.16 | Environmentally-friendly paving materials and methods are encouraged, including but not limited to using recycled asphalt tires and roofing shingles as part of the mix or base. | ☐ | ☑ | ☐ | |
| C.17 | Porous paving materials such as paving blocks with decorative gravel, or properly spaced cobbles, brick, and natural stone with grass planted in between in small clusters and methods that reduce stormwater runoff are encouraged. | ☐ | ☑ | ☐ | |
| C.18 | The off-street parking provisions for all commercial development shall be in conformance with 15.04(G). Required off-street parking space, including access drives and aisles, shall not cover more than seventy-five percent (75%) of the lot area in which such off-street parking space is permitted. | ☑ | ☐ | ☐ | |
| PEDESTRIAN CIRCULATION | | | | | |
| D.2 | There shall be a paved pedestrian route from the sidewalk or street to the main building entrance, and from the parking area to the nearest building entrance. | ☑ | ☐ | ☐ | |
| D.3 | Pedestrian routes shall be paved with concrete. Bituminous material shall not be allowed for pedestrian routes. | ☑ | ☐ | ☐ | |
| D.4 | Porous paving materials and methods that reduce stormwater runoff.is encouraged. | ☐ | ☑ | ☐ | |
| BUILDING MECHANICAL SERVICE ELEMENTS | | | | | |
| E.2 | The design and location of the following items shall be indicated on building and/or site plans, illustrated with spec sheets as appropriate, and submitted with the Design Standards Checklist: | X | ☐ | ☐ | |
| E.2a | utility meters | X | ☐ | ☐ | |
| E.2b | building mechanicals | X | ☐ | ☐ | |
| E.2c | trash and recycling containers | ☑ | ☐ | ☐ | |
| E.2d | bicycle parking | ☑ | ☐ | ☐ | |
| E.2e | outdoor seating areas | ☑ | ☐ | ☐ | |
| E.2f | solar and wind facilities | X | ☐ | ☐ | |
| E.2g | dish antennas (not permitted to hang off the side of buildings) | ☐ | ☐ | X | |
| E.2h | transformers | ☑ | ☐ | ☐ | |
| E.2i | back-up generators | ☑ | ☐ | ☐ | |

# DESIGN REVIEW CHECKLIST

| | | YES | NO | N/A | NOTES |
|---|---|---|---|---|---|
| E.3 | Service areas, utility meters, and building mechanicals shall not be located on the street side of the building, nor on the side wall closer than 10 feet to the street side of the building. The location of emergency back-up generators and transformers shall be coordinated between the City, developer and the utility company. Screening of meters, generators, transformers, and mechanicals is required when visible from the street with an approved screen device. Screening materials shall match building materials. Cable, conduit and phone line shall not be visible on the exterior with the exception of conduit running directly to the meter/utility boxes at the time of initial occupancy. Mailboxes are permitted within 10 feet of the front of the building if not visible from the street. | ☒ | ☐ | ☐ | |
| E.4 | Trash and recycling containers, including cans and dumpsters, shall have covers and be screened so as not to be visible from the street or from neighboring properties. Screening shall be one foot higher than the container but no higher than six feet; however, roofed enclosures may exceed this limit. | ☑ | ☐ | ☐ | |
| E.5 | If a building owner chooses to provide a trash receptacle and/or a smoking materials receptacle, the receptacle(s) shall be decorative if located at the entrance that faces a public street. These receptacles shall be screened from view and/or designed to fit with the architecture and materials of the building. | ☑ | ☐ | ☐ | |
| E.6a | High energy gas appliances shall have the air intakes and exhaust vents located on the sides or rear of the building where they do not interfere with any sidewalks, are not likely to be blocked or damaged by pedestrian traffic, snow or the removal of snow, and away from any trees or shrubs that would be harmed by the exhaust heat and gases. | ☒ | ☐ | ☐ | |
| E.6b | Window-mounted air conditioners shall not be permitted. | ☐ | ☐ | ☒ | |
| E.6c | PTAC air conditioner/heat pump units must be designed into the architecture of the building. | ☒ | ☐ | ☐ | |
| E.6d | If heat pumps or air conditioners are located on the ground, they shall be on one side or the rear of the building and screened with evergreens or decorative screening that matches or complements the exterior siding of the building, such that proper clearances are maintained for the manufacturer's warranty. | ☐ | ☐ | ☒ | |
| E.6e | If heat pumps or air conditioners are located on the roof, they shall be placed, painted and/or screened so as to minimize the visual impact to the street. | ☐ | ☐ | ☒ | |
| E.7a | Bicycle parking using bike racks specifically designed for bike parking shall be provided at one (1) space per 10 automobile parking spaces or one (1) space per 20 employees, whichever is greater, and should be located near building entries, shall not interfere with pedestrian circulation, and shall be well-lit. Bikes are not permitted to be stored, locked or chained on decks, patios, fences or any other exterior location other than a bike rack specifically designed for bike parking. | ☑ | ☐ | ☐ | |
| E.7b | Bicycle parking (to accommodate four bicycles) shall be nominally at least nine (9) by six (6) feet or fifty-four (54) square feet and increase by the same ratio to accommodate the number of bike spaces. | ☑ | ☐ | ☐ | |
| E.7c | The base for bike racks should be concrete to ensure their stability; however, the remaining bicycle parking area shall be porous paving materials (paving blocks with decorative gravel or wood mulch, or properly spaced cobbles, brick, and natural stone with grass planted in between in small clusters) to reduce stormwater runoff but shall not result in standing water. If an area for bike parking is designed using these standards, then up to 100 percent of the space taken for the bike parking shall count as green space. | ☐ | ☑ | ☐ | |

# DESIGN REVIEW CHECKLIST

| LANDSCAPING OPEN SPACE & PLANTINGS | | YES | NO | N/A | NOTES |
|---|---|---|---|---|---|
| F.2 | A landscape design and planting plan shall be prepared and submitted for all buildings. Landscape plans for developments shall be prepared and signed by a Landscape Architect, nurseryman, or professional site planner with educational training or work experience in land analysis and site plan preparation prior to submittal to the City. | ☑ | ☐ | ☐ | |
| F.2a | No building permit shall be issued until the required landscaping plan has been submitted and approved, and no certificate of occupancy shall be issued until the landscaping is completed as certified by an on-site inspection by the Building Inspector, Planning Staff, or other designated official, unless a financial guarantee acceptable to the City has been submitted. | ☑ | ☐ | ☐ | |
| F.2b | Landscape surety. The owner shall provide the City with a cash deposit, bond, or approved letter of credit to guarantee the proper installation and growth of all landscape improvements proposed in the approved landscape plan. Said surety may remain in effect for two full growing seasons. A growing season shall be considered a period from May 1 to September 30. The first year, the amount of the surety will be equal to 100% of the estimated cost of plant material, installation and tree preservation. Once installation has been completed per the approved landscape plan and verified by the City, 75% of the surety will be reimbursed back to the owner. The remaining 25% will be kept by the City for a period of twelve (12) months to cover any maintenance cost that may be needed. Such surety shall be filed with the City Finance Officer. | ☑ | ☐ | ☐ | |
| F.2c | The City may allow an extended period of time for completion of all landscaping if the delay is due to conditions which are reasonably beyond the control of the developer. Extensions may not exceed nine months, and extensions may be granted due to seasonal weather conditions. When an extension is granted, the City may require such additional security and conditions as it deems necessary. | ☑ | ☐ | ☐ | |
| F.3a | The plan shall address all parts of the parcel and shall indicate: Details of all proposed vegetative landscaping materials, including placement, common and botanical names, caliper/height or container size and quantity and maintenance requirements. | ☑ | ☐ | ☐ | |
| F.3b | Details of proposed non-vegetative landscaping and screening materials. | ☑ | ☐ | ☐ | |
| F.3c | Planting and construction schedule for completion of landscaping and screening plans. | ☐ | ☑ | ☐ | |
| F.3d | Estimated cost from a landscaper on a bid or estimate form of the proposed landscaping. | ☐ | ☑ | ☐ | |
| F.4 | All portions of the site not covered by buildings, paving material, or other planned and approved surfaces shall be considered "landscaped area" and shall have a minimum of 4 inches of top soil and be planted with living plant materials and/or mulches. Overall site landscaping shall include not less than: | ☑ | ☐ | ☐ | |
| F.4a | One tree placed in the boulevard per 40 linear feet of lot frontage; | ☑ | ☐ | ☐ | |
| F.4b | Not less than two trees and eight shrubs per 600 square feet of landscaped area. These are minimum standards – more plantings are encouraged. | ☑ | ☐ | ☐ | |
| F.5 | All plant material used shall meet the minimum standards established by the American Association of Nurserymen as published in the American Standards for Nursery Stock and shall meet the following minimum requirements: | ☑ | ☐ | ☐ | |
| F.5a | Deciduous trees: 2" dbh (diameter at breast height) | ☑ | ☐ | ☐ | |
| F.5b | Ornamental trees: 2" dbh | ☑ | ☐ | ☐ | |
| F.5c | Evergreen trees: 5' height | ☑ | ☐ | ☐ | |
| F.5d | Shrubs: 5 gallon container | ☑ | ☐ | ☐ | |
| F.5e | Vines and Perennials: 1 gallon container | ☑ | ☐ | ☐ | |

# DESIGN REVIEW CHECKLIST

| | | YES | NO | N/A | NOTES |
|---|---|---|---|---|---|
| F.6 | Boulevard trees will be installed by the City Forester at City expense if the developer attends City tree school. If the developer installs boulevard trees they shall conform to City street standards. A complete list of trees and shrubs and other reliable plant material that has been approved by the City Forester is available in the City Planning and Development Department. | ☑ | ☐ | ☐ | |
| F.7 | Existing healthy trees should be preserved to the greatest extent practicable and shall be indicated on grading and landscape plans submitted for plan review; however, invasive trees shall be removed. Existing damaged, decayed, or diseased trees should be removed to protect remaining trees. Construction near existing trees should follow Best Management Practices to ensure their survival. | ☑ | ☐ | ☐ | |
| F.8 | Landscaping should reinforce pedestrian circulation routes and obstruct undesired routes of convenience. Bushes, trees, rocks, and other landscape features should be used to indicate where pedestrians should and should not travel. | ☑ | ☐ | ☐ | |
| F.9a | Provide a five (5) to six (6) foot high solid screen to separate parking lots from abutting residential uses or other non-compatible uses. A solid landscape screen is defined as an evergreen or nearly evergreen mixture (minimum of 65% evergreen) of shrubs, bushes, or trees that produce a dense, sight-obscuring screen at least five (5) to six (6) feet in height within three years of planting. Berms may be included in this definition as long as the maximum height of the berm is five feet; both sides of the berm are planted with evergreen or nearly evergreen shrubs or bushes so that the total height of landscaping and berm will be at least six feet within three years of planting; and top of the berm plantings form a dense, sight-obscuring screen within the same three-year period. | ☐ | ☐ | ☑ | |
| F.9b | Provide a minimum three (3) foot high visual relief screen when adjacent to a street in the form of a hedge, fence, planter, berm, dividers, shrubbery and trees or any combination. The visual relief screen shall extend the length of the parking lot. Three (3) feet in height shall be measured from surface of the parking lot and may be negotiable depending on the elevation of the parking lot in relation to the sidewalk and/or street. All landscaping to form such a visual relief shall be a minimum height of 2 feet at time of planting. Bark or other loose material shall not be placed on berms in these areas since it may be displaced on the street or sidewalk. | ☑ | ☐ | ☐ | |
| F.10a | The property owner shall be responsible for maintenance and replacement of trees, shrubs, grass, ground covers, loose bark or gravel, and sod which are part of the approved landscape plan. If any such plant materials are not maintained or replaced, the City may utilize the required surety to replace the newly planted or protected landscaping or to deem this to be a Municipal Code Violation and issue an Order to Correct. | ☑ | ☐ | ☐ | |
| F.10b | The owner is responsible for keeping trees in a plumb position. When staking or securing trees is done, it shall occur so as not to create any hazards or unsightly obstacles. | ☑ | ☐ | ☐ | |
| F.10c | Plants must be maintained to be kept in sound, healthy and vigorous growing conditions and free of disease, insect eggs and larvae. | ☑ | ☐ | ☐ | |
| F.10d | A sprinkler or lawn irrigation system shall be required in the front yard and boulevard of all developments if lawn or sod is proposed. This standard does not apply to boulevards if sprinkler or lawn irrigation systems are not needed for the front yard. | ☑ | ☐ | ☐ | |
| WALLS AND FENCES | | | | | |
| G.2 | Walls and fences located in the front yard setback shall not exceed six feet in height above the finished grade and shall be at least 50% transparent to retain the visual connection between street and building. | ☒ ?? | ☐ | ☐ | |

# DESIGN REVIEW CHECKLIST

| | | YES | NO | N/A | NOTES |
|---|---|---|---|---|---|
| G.3 | The design and materials for walls and fences shall be coordinated with the design and materials of the principal buildings and should have substantially the same detail. This is not intended to require identical materials and design. | X | | | |
| G.3a | Pressure treated lumber fences shall not be permitted unless stained or painted. | | | X | |
| G.3b | All chain link fences must be plastic coated and shall only be permitted in side yards and backyard, and shall not extend nearer to the street than the front of the building nor used in the side yard on a corner property. | ✓ | | | |
| G.3c | Smooth faced concrete (CMV) blocks or non-architectural poured walls used to construct a wall shall be covered with brick or some other decorative block or dimensional material such as a stained block product. Painted or colored smooth-faced concrete bricks or blocks shall not be considered decorative block. | X | | | |
| G.4 | Walls and fences shall provide variety and articulation at each end and at intervals not exceeding 25 feet through at least one of the following methods: Changes in plane of not less than one (1) foot; Expression of structure, such as post, column, or pilaster; Variation of material; or Landscaping | X | | | |
| STORMWATER INFILTRATION AND CONTROL | | | | | |
| H.2 | A stormwater management and erosion control plan shall be required for all new construction, shall be coordinated with the Landscaping and Open Space Plan, and shall be designed by either a Registered Architect, Landscape Architect or a Professional Civil Engineer in accordance with the City of La Crosse's Stormwater Management Ordinance and shall include a maintenance plan and agreement. | ✓ | | | |
| H.2a | Until such time as the City adopts a stormwater management ordinance, the City shall use the La Crosse County Stormwater Management Ordinance. | ✓ | | | |
| H.2b | For parcels less than ¼ acre in size, the City shall work with the property owner/developer/applicant to develop a practical site-specific stormwater management plan that allows for flexibility in the use of stormwater treatment devices including rain barrels, rain gardens, swales, cisterns, drain tiles, soil amendments, porous pavements, grass pavers for overflow parking areas, etc. | ✓ | | | |
| H.3 | The use of bio-cells, living roofs and rain gardens is encouraged due to their aesthetic as well as utilitarian benefits. | | | ✓ | |
| H.4 | Newly concentrated stormwater, such as that from rooftop, impervious surface, or swales, shall not be directed onto or across adjacent properties or across sidewalks. Rooftop stormwater shall not be discharged within 5 feet of a sidewalk unless an intervening landscape element is used to promote infiltration, such as a rain garden. | ✓ | | | |
| H.5 | Stormwater detention and infiltration facilities shall be designed as visual and open space amenities that enhance the overall appearance of the site. | ✓ | | | |
| EXTERIOR LIGHTING | | | | | |
| I.2 | All exterior lights shall be designed for commercial use. A lighting plan showing lighting levels on-site and at the property line as well as spec sheets with pictures must be submitted with the Design Standards Checklist for each exterior light to be used. | X | | | |
| I.3 | Pedestrian lighting shall clearly indicate the path of travel, shall minimize dark spots along that path, and shall utilize coordinated light fixtures. | X | | | |
| I.4 | The maximum height of wall-mounted parking lot light fixtures shall be 16 feet above the ground. Pole-mounted fixtures are acceptable but not required and will have a maximum height of 30 feet from the ground to the top of the fixture. Fixtures shall be of full-cut-off (FCO) design to minimize glare and spillover. | | | X | |

# DESIGN REVIEW CHECKLIST

| | | YES | NO | N/A | NOTES |
|---|---|---|---|---|---|
| I.5 | Ornamental lighting to light the building façade is permitted provided that the light source is not visible from the property line and is designed to minimize glare and spillover. | ☐ | ☐ | ☒ | |
| I.6 | No overhead light source (i.e., the lamp or reflector) shall be visible from the property line. Shields may be employed, if necessary, to meet this requirement. The maximum allowable luminance measured 25 feet beyond the property line shall be .05 horizontal foot-candles (HFC). | ☒ | ☐ | ☐ | |
| I.7 | Lighting levels for parking lots and pedestrian routes: (horizontal luminance measured in foot-candles): | ☒ | ☐ | ☐ | |
| I.7a | Average: 2.4 foot-candles | ☒ | ☐ | ☐ | |
| I.7b | Minimum: 1.0 foot-candles | ☒ | ☐ | ☐ | |
| I.7c | Uniformity Ratio (Bright spots to dark spots): 4:1 | ☒ | ☐ | ☐ | |
| I.7d | Maximum Average: .5 foot-candles | ☒ | ☐ | ☐ | |
| I.8 | Each exterior entry to structures on the property shall have an exterior light. | ☒ | ☐ | ☐ | |
| I.9 | For properties adjacent to residential uses, motion sensor flood or spot lights shall have shrouds, be limited to two (2) bulbs pointed at least thirty degrees downward and not directly into windows or doors of neighboring building and the light sources shall not be visible from the street. | ☒ | ☐ | ☐ | |
| | PATIOS, PORCHES, DECKS, AND ROOFTOP GARDENS/DECKS | | | | |
| J.2 | Every residential unit is encouraged to have its own patio or balcony and shall be incorporated into the architectural façade of the building and may encroach into the building setback area but not more than 25%. Commercial structures are also permitted to have exterior balconies. No patio or balcony can hang over a sidewalk. | ☐ | ☐ | ☒ | |
| J.3 | For commercial developments, ground level patios or decks for customer seating are permitted in the setback areas and should include some screening for noise. | ☐ | ☐ | ☒ | |
| J.4 | Exterior stairs leading to a deck or balcony are permitted provided that they are decorative and are architecturally compatible with the building and constructed of compatible materials. Exterior corridors visible from a street are not permitted. | ☐ | ☐ | ☒ | |
| J.5 | Rooftop green roofs or rooftop patios and decks are permitted and if intended for occupied use shall have a railing height or parapet of at least 42 inches. Only outdoor furniture is permitted. | ☒ | ☐ | ☐ | |
| | | ?? | | | |
| | BUILDING DESIGN: FORM, SCALE AND CONTEXT | | | | |
| K.2 | Photos of at least four (4) street views of nearby blocks shall be submitted with the Design Standards checklist. | ☒ | ☐ | ☐ | |
| K.3 | Buildings shall be designed to provide human scale, interest, and variety. The following techniques may be used to meet this objective: | ☒ | ☐ | ☐ | |
| K.3a | Variation in the building form such as recessed or projecting bays, shifts in massing, or distinct roof shapes. | ☒ | ☐ | ☐ | |
| K.3b | Emphasis of building entries through projecting or recessed forms, detail, color, or materials. | ☒ | ☐ | ☐ | |
| K.3c | Variation of material, modules, expressed joints and details, surface relief, color, and texture to break up large building forms and wall surfaces. Such detailing could include sills, headers, belt courses, reveals, pilasters, window bays, and similar features. | ☒ | ☐ | ☐ | |
| K.4 | For all non-manufacturing or retail buildings, where the allowable building is more than 50% wider than adjacent buildings, one of the following techniques shall be employed to minimize the apparent width of the primary façade: | ☐ | ☐ | ☒ | |
| K.4a | Articulate the façade with projections or bays. | ☐ | ☐ | ☐ | |
| K.4b | Use architectural elements such as column, canopies, glass, changes in materials, and covered entries to interrupt the façade. | ☐ | ☐ | ☐ | |

# DESIGN REVIEW CHECKLIST

| | | YES | NO | N/A | NOTES |
|---|---|---|---|---|---|
| K.5 | The first floor façade shall include windows to provide visual interest and visual connection to the street. The total area of windows and doors on the street-facing façade, including trim, shall not be less than 20% of the total area of the façade, excluding gables. | X | | | |
| K.6 | Buildings shall be built to the front yard setback line. In highway commercial areas, the building setback shall not be greater than 25 feet and no parking is permitted in the front yard setback area. | X | | | |
| K.7 | Commercial buildings within Historic Districts or adjacent to any designated historic building must first receive DRC review and approval prior to submittal to the Heritage Preservation Commission for their review. Approval by the Heritage Preservation Commission is necessary prior to the issuance of any building permit. The developer can appeal to the City Plan commission if denied by the Heritage Preservation Commission. | | | X | |
| | BUILDING ENTRANCES, DETAILS, TRIM, DOORS AND WINDOWS | | | | |
| L.2 | The primary entrance to the building shall be covered at least three (3) feet from the door. Entrance features may encroach into the front yard setback a maximum of three (3) feet. Building entrances shall be emphasized through projecting or recessing forms, detail, color or materials. Buildings shall be oriented toward the street with pedestrian access. | X | | | |
| L.3 | All openings shall be articulated or appropriately trimmed through the use of materials such as flat or arched lintels, projecting sills, or surrounds. | X | | | |
| L.4a | All windows shall be in keeping with the architectural character of the building. | X | | | |
| L.4b | All windows shall have an interior locking or securing mechanism. | X | | | |
| L.4c | For mixed used developments that include residential units, exterior entry doors for individual units shall be residential in style (real or decorative styles, rails or panels) solid or insulated or multiple units may be commercial in style (glass). If the door does not have a translucent window lower than five (5) feet, it shall have a security peephole. | | | X | |
| | ROOFS AND ROOF LINES | | | | |
| M.2 | Any roof style such as hip, gambrel, mansard, colonial, flat or another roof style is permitted so long as the roof pitch is appropriate to the architectural style of the building (e.g. prairie school) and the roof element contains additional architectural elements such as dormers, long overhangs, windows or other feature. | X | | | |
| M.3 | Flat roofs are permitted, and must incorporate a parapet wall on all sides, unless the rear side of the building is sloped for drainage. The parapet should include architectural details appropriate to the building design that create a positive visual termination for the building (a "top"). | X | | | |
| M.4 | A minimum of 50% of a building's linear roof drip edge should fall to ground surfaces that do not contain impervious surface. If gutters or other stormwater drains toward neighboring properties, then water shall be directed to an onsite rain garden(s) designed to retain a 0.5 inch-1hr rainfall. For information regarding directing clean roof water to rain gardens, the Wisconsin DNR and UW-Extension have extensive publications on the proper calculation for the size and planting materials for rain gardens in Wisconsin. | X | | | |
| | EXTERIOR MATERIALS | | | | |
| N.2 | The use of identical materials on all sides of the building is encouraged; however; higher-quality materials on street-facing façade and complementary materials on other façade is acceptable. | X | | | |
| N.3 | Use of decorative accessories and trim is highly encouraged. | X | | | |

# DESIGN REVIEW CHECKLIST

| | | YES | NO | N/A | NOTES |
|---|---|---|---|---|---|
| N.4 | Vinyl, plywood, chipboard, T1-11, asphalt siding, non-architectural metal siding and smooth-faced concrete block are prohibited as exterior finish materials unless the architect can demonstrate that the materials are appropriate to the design of the building. Treated wood shall be painted or stained. | X | | | |
| N.5 | Natural wood shall be painted or stained, unless it is cedar, redwood or some other naturally weather resistant species and is intended to be exposed. | | | X | |
| N.6a | Since the selection of building colors has a significant aesthetic and visual impact upon the public and neighboring properties, as well as an impact on the energy use and comfort of customers and tenants, designs and color shall be selected in general harmony with the overall existing neighborhood. | X | | | |
| N.6b | Neutral or natural colors for the primary siding material with brighter or darker colors for accent and trim that provide for a more interesting building and are cooler in the summer are preferred. | X | | | |
| N.6c | Complementary multi-color and textured roofing materials that provide for a more interesting building and are cooler in the summer are preferred. | | | X | |
| | GARAGES AND ACCESSORY BUILDINGS | | | | |
| O.2 | Street-facing overhead doors on garages are not permitted on lots served by an alley. | | | X | |
| O.3 | The cumulative length of all garage doors facing the street shall not exceed 50% of the total length of the street-facing elevation unless architecturally justified. | | | X | |
| O.4 | Accessory buildings shall be architecturally compatible and be constructed of the same materials as the primary building(s). All changes to the approved plans such as the addition of an accessory structure shall be approved by the Design Review Committee if not submitted at the time of initial review. | X | | | |
| | BUILDING CONSTRUCTION | | | | |
| P.2 | A completed LEED checklist must be submitted with the Design Standards checklist to demonstrate compliance with the standard. | | | X | |
| | | | | ?? | |
| | BUILDING, PROPERTY AND LANDSCAPING MAINTENANCE | | | | |
| Q.2 | All commercial structures and buildings that are developed and constructed under this ordinance shall maintain the property through an ongoing maintenance program. The maintenance program is to include all exterior aspects of the development and include but is not limited to parking lots, building mechanicals, service elements, customer and tenant amenities, landscaping open space and plantings, wall and fences, signage, stormwater facilities, exterior lighting, patios and decks, exterior finishes, windows, architectural detail, and accessory structures. | ☑ | | | |
| Q.3 | The project shall be maintained over the life of the development in a like-new condition with an on-going maintenance program that adheres to the intent of the original building plans and is subject to inspection by the City at anytime. Failure to maintain the project may subject the property to fines as permitted under this Chapter and the City of La Crosse Stormwater Management Ordinance. (#4513-7/9/09) | ☑ | | | |




INTERSECTION OF MARKET ST & WINNEBAGO ST LOOKING EAST



MARKET ST & 19TH LOOKING EAST




INTERSECTION OF MARKET ST & EAST AVE S LOOKING EAST



INTERSECTION OF MARKET ST & 19TH ST S LOOKING WEST

NOTE: THESE IMAGES ARE AN IMPRESSION ONLY AND MAY NOT REFLECT THE ACTUAL CONSTRUCTION AS DELINEATED BY THE CONSTRUCTION DOCUMENTS
NOTE: THESE IMAGES ARE FOR REFERENCE ONLY. FURNITURE FOR REFERENCE ONLY, AND NOT PART OF SCOPE.

BRAY ARCHITECTS

Milwaukee

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Milwaukee, Wisconsin 53204
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Sheboygan • Davenport
Moline • Saint Paul

www.brayarch.com

Project Title:
New Building for
New Elementary School
La Crosse School District
807 East Ave S
La Crosse, WI 54601

REVISIONS:

| # | DATE | DESCRIPTION |
|---|---|---|

NOT FOR CONSTRUCTION

Project Number:
3752

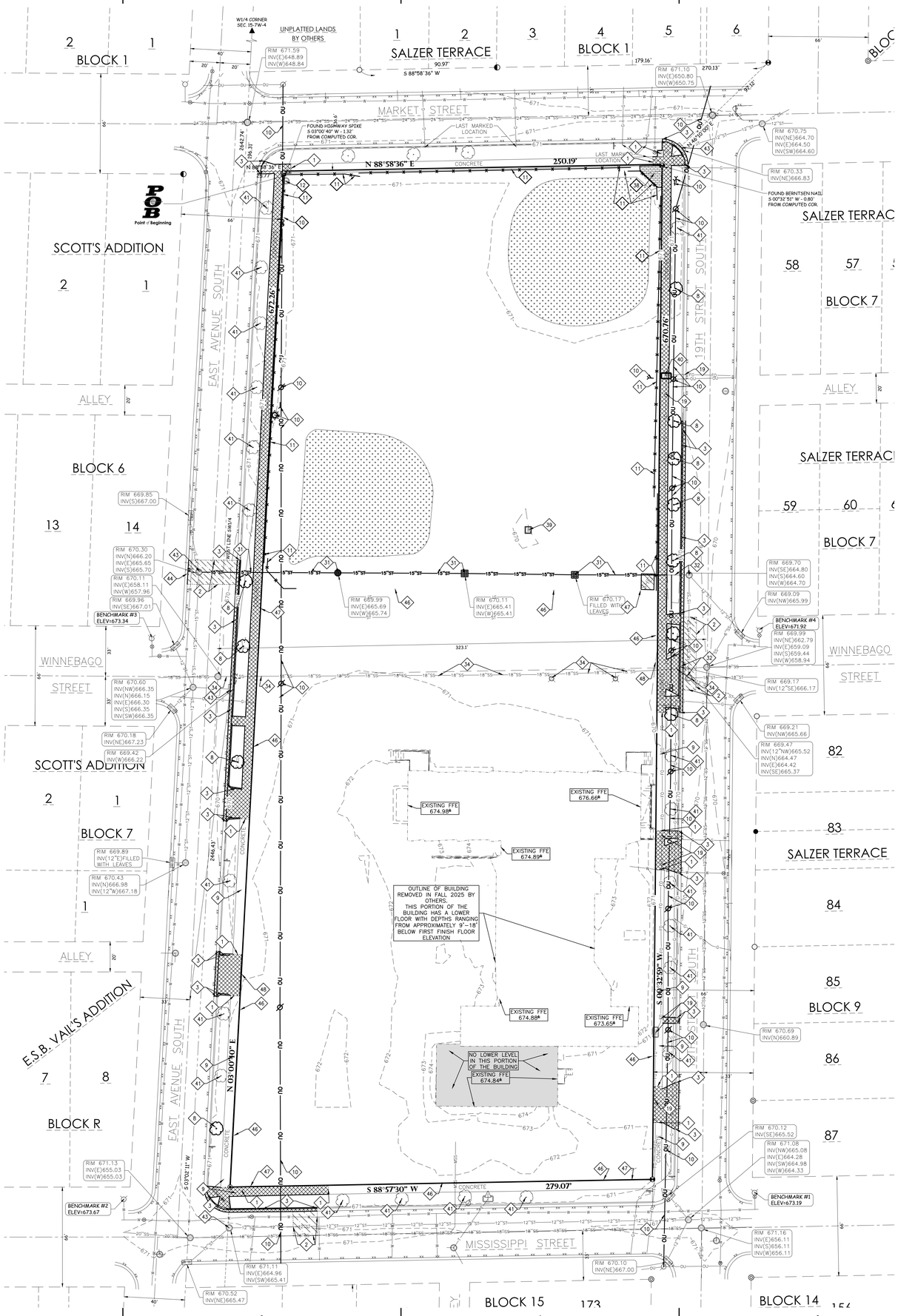
Issued For:
CONTRACT DOCUMENTS
10/28/2025

Sheet Title:
RENDERINGS - EXTERIOR

Sheet Number:
RA3.1

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## GENERAL NOTES:

1. CONTRACTOR SHALL LOCATE ALL PUBLIC AND PRIVATE UTILITIES PRIOR TO COMMENCEMENT OF WORK.
2. ALL DEMOLITION MATERIALS SHALL BE REMOVED FROM THE SITE AND DISPOSED OF IN A LEGAL MANNER EXCEPT FOR THOSE ITEMS NOTED TO BE SALVAGED, WHICH SHOULD BE TURNED OVER TO THE OWNER.
3. INSTALL AND MAINTAIN ALL REQUIRED EROSION CONTROL MEASURES FOR PERIMETER PROTECTION PRIOR TO THE START OF DEMOLITION/CONSTRUCTION, IN ACCORDANCE WITH THE LOCAL AND STATE GOVERNING AUTHORITIES.
4. BIDDERS SHALL VISIT THE SITE AND REVIEW EXISTING CONDITIONS PRIOR TO THE BID DATE.
5. ANY EXISTING PUBLIC & PRIVATE UTILITIES NOT SHOWN ON THIS DOCUMENT WHICH NEED TO BE REMOVED, RELOCATED, AND/OR ADJUSTED SHALL BE THE RESPONSIBILITY OF THE SITE GRADING CONTRACTOR.
6. COORDINATE ALL PRIVATE & PUBLIC UTILITY REMOVAL, DISCONNECTION, &/OR RECONNECTION WITH RESPECTIVE UTILITY COMPANIES PRIOR TO BEGINNING WORK.
7. COORDINATE UTILITY SERVICE DISCONNECTIONS/OUTAGES WITH OWNER AND ANY IMPACTED NEIGHBORS. MINIMIZE DISRUPTIONS TO THE MAXIMUM EXTENT PRACTICAL.
8. ANY EXISTING UTILITIES NOT EXPRESSLY LABELED FOR DEMOLITION/REMOVAL ON THIS DOCUMENT SHALL BE LEFT IN PLACE AND IN THEIR CURRENT STATE OF OPERATION. CONTACT ENGINEER WHEREVER CLARIFICATION IS NEEDED.
9. STRIP TOPSOIL WITHIN THE PROJECT LIMITS IN ACCORDANCE WITH THE PROJECT MANUAL SPECIFICATIONS.
10. IF STRIPPED TOPSOIL IS STOCKPILED ON SITE, SILT FENCE SHALL BE INSTALLED AROUND THE BASE OF THE STOCKPILE TO PREVENT SEDIMENT TRANSPORT.
11. PRIOR TO PERFORMING WORK WITHIN PUBLIC RIGHT OF WAYS, NOTIFY AND COORDINATE WORK WITH THE LOCAL MUNICIPALITY.
12. MAINTAIN TRAFFIC CIRCULATION TO ALL NEIGHBORING PROPERTIES SHOWN ON THIS DOCUMENT. COORDINATE ALL WORK WITH NEIGHBORING PROPERTY OWNERS.

## KEYNOTES:

1. SAWCUT EXISTING CONCRETE WALK
2. SAWCUT EXISTING ASPHALT PAVEMENT
3. SAWCUT & REMOVE EXISTING CONCRETE CURB & GUTTER
4. SAWCUT EXISTING CONCRETE CURB HEAD ONLY
5. MAINTAIN EXISTING SIGN
6. REMOVE EXISTING SIGN
7. MAINTAIN & PROTECT EXISTING SIGN
8. CLEAR & GRUB EXISTING TREE/VEGETATION
9. MAINTAIN EXISTING CONCRETE WALK
10. REMOVE EXISTING OVERHEAD POWER, POWER POLE, GUY WIRE. COORDINATE REMOVALS WITH UTILITY COMPANY.
11. REMOVE EXISTING FENCE & GATE
12. MAINTAIN EXISTING UTILITY MANHOLE. ADJUST RIM TO FINISH GRADE ELEVATION. SEE SHEET C3.0. COORDINATE ADJUSTMENT WITH UTILITY COMPANY.
13. REMOVE EXISTING CONCRETE STAIRS
14. REMOVE EXISTING BASKETBALL HOOP
15. REMOVE EXISTING PLAYGROUND EQUIMPMENT
16. REMOVE EXISTING BIKE RACK
17. REMOVE EXISTING GARDEN AREA
18. REMOVE EXISTING LANDSCAPE AREA, INCLUDING, NOT LIMITED TO PLANTINGS, MULCH, EDGING, FABRIC, ETC.
19. MAINTAIN EXISTING FIBER OPTIC SERVICE & BOX. ADJUST TOP OF BOX TO FINISH GRADE ELEVATION. SEE SHEET C3.0. COORDINATE ANY ADJUSTMENT WITH UTILITY COMPANY.
20. REMOVE EXISTING HANDRAIL
21. REMOVE EXISTING BOLLARD
22. REMOVE EXISTING FIBER OPTIC SERVICE. COORDINATE REMOVAL WITH UTILITY COMPANY.
23. REMOVE EXISTING GENERATOR
24. REMOVE EXISTING GAS SERVICE AND/OR METER. COORDINATE WITH UTILITY COMPANY.
25. REMOVE EXISTING HVAC UNIT
26. REMOVE EXISTING ELECTRICAL SERVICE. COORDINATE REMOVAL WITH UTILITY COMPANY.
27. REMOVE EXISTING ELECTRICAL TRANSFORMER. COORDINATE REMOVAL WITH UTILITY COMPANY.
28. REMOVE EXISTING TANK
29. REMOVE EXISTING PILLAR
30. REMOVE EXISTING RETAINING WALL
31. REMOVE EXISTING STORM SEWER MANHOLE AND/OR STORM PIPE
32. MAINTAIN EXISTING STORM SEWER MANHOLE AND/OR STORM PIPE
33. REMOVE AC UNIT
34. MAINTAIN EXISTING SANITARY SEWER PIPE AND/OR CLEANOUT
35. REMOVE EXISTING VENT PIPE
36. REMOVE EXISTING BIRD HOUSE
37. REMOVE EXISTING BENCH
38. REMOVE EXISTING BRICK AREA
39. REMOVE EXISTING CABLE BOX– COORDINATE REMOVAL WITH UTILITY COMPANY
40. REMOVE EXISTING FIBER OPTIC BOX– COORDINATE REMOVAL WITH UTILITY COMPANY
41. MAINTAIN & PROTECT EXISTING TREE
42. MAINTAIN EXISTING SANITARY MANHOLE– ADJUST RIM AS NEEDED TO FINISH GRADE ELEVATION
43. MAINTAIN EXISTING STORM MANHOLE– ADJUST RIM AS NEEDED TO FINISH GRADE ELEVATION
44. MAINTAIN EXISTING CONCRETE CURB & GUTTER
45. REMOVE & RELOCATE EXISTING SANITARY CLEANOUT
46. REMOVE AND REPLACE EXISTING CONCRETE AT THE BASE OF THE WATER FOUNTAIN, WATER FOUNTAIN TO BE MAINTAINED

## DEMOLITION HATCH PATTERNS:

- BITUMINOUS REMOVAL
- CONCRETE REMOVAL
- GRAVEL REMOVAL
- BUILDING REMOVAL

## CIVIL SHEET INDEX:


C1.0 DEMOLITION PLAN
C2.0 LAYOUT PLAN
C3.0 GRADING PLAN
C4.0 EROSION CONTROL PLAN
C5.0 UTILITY PLAN
C6.0 DETAILS
C6.1 DETAILS
C6.2 DETAILS
C6.3 DETAILS
L1.0 LANDSCAPE PLAN


## BENCHMARK:

ELEVATIONS ARE REFERENCED TO NAVD 88 DATUM.

**BENCHMARK #1**
NORTHWEST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHEAST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND 19TH STREET SOUTH.
ELEVATION = 673.19

**BENCHMARK #2**
NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND EAST AVENUE SOUTH.
ELEVATION = 673.67

**BENCHMARK #3**
NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST CORNER OF THE INTERSECTION OF EAST AVENUE SOUTH AND WINNEBAGO STREET.
ELEVATION = 673.34

**BENCHMARK #4**
BURY BOLT ON HYDRANT, LOCATED AT THE NORTHEAST CORNER OF THE INTERSECTION OF 19TH STREET SOUTH AND WINNEBAGO STREET.
ELEVATION = 671.92

## UTILITY DISCLAIMER:

THE LOCATIONS, SIZES, AND TYPES OF UNDERGROUND PUBLIC AND PRIVATE UTILITIES OR SUBSTRUCTURES SHOW HEREON WERE OBTAINED FROM VISUAL INSPECTION, FIELD MEASUREMENTS, AND/OR AS–BUILT PLANS. SANITARY SEWER AND STORM SEWER PIPE SIZES, INVERTS, DIRECTION, AND LOCATIONS BETWEEN MANHOLES ARE SUPPLEMENTED BY AS–BUILT PLANS AND/OR ESTIMATED BASED ON FIELD OBSERVATIONS. PRIOR TO CONSTRUCTION IN THE VICINITY OF ANY UTILITIES SHOWN HEREON, IT IS RECOMMENDED THAT THE LOCATIONS, DEPTHS, AND SIZES BE FIELD VERIFIED. THE LOCATIONS SHOWN HEREON ARE, ONLY APPROXIMATE, WITH POSSIBILITY THAT ADDITIONAL UTILITY LINES NOT DISCOVERED, OR MARKED, DURING THE SEARCH OF RECORDS AND THE FIELD SURVEY MAY EXIST. ANY CONTRACTOR USING THE INFORMATION SHOWN HEREON IS HEREBY FOREWARNED THAT ANY EXCAVATION UPON THIS SITE MAY RESULT IN THE DISCOVERY OF ADDITIONAL UNDERGROUND UTILITIES NOT SHOWN HEREON. IN GENERAL, UNDERGROUND UTILITY LOCATIONS ARE SHOWN FROM UTILITY MARKINGS, BY OTHERS, AND/OR AS–BUILT PLANS, PROVIDED BY OTHERS. POINT OF BEGINNING MAKES NO WARRANTY OF ANY KIND, EXPRESS OR IMPLIED, WITH RESPECT TO THE EXISTING UTILITIES SHOWN HEREON, AND BELIEVES THAT THE INFORMATION CONTAINED HEREIN IS RELIABLE AND GENERALLY ACCURATE FOR THE PURPOSE INTENDED.

0' 30' 60'
1" = 30'


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Point of Beginning
Civil Engineering
Land Surveying
Landscape Architecture
4941 Kirschling Court
Stevens Point, WI 54481
715.344.9999(Ph)
715.344.9922(Fx)


Project Title:
NEW BUILDING FOR:
NEW ELEMENTARY SCHOOL
LA CROSSE SCHOOL DISTRICT
807 EAST AVE S.
LA CROSSE, WI 54601

REVISIONS:

| | DATE | DESCRIPTION |
|---|---|---|

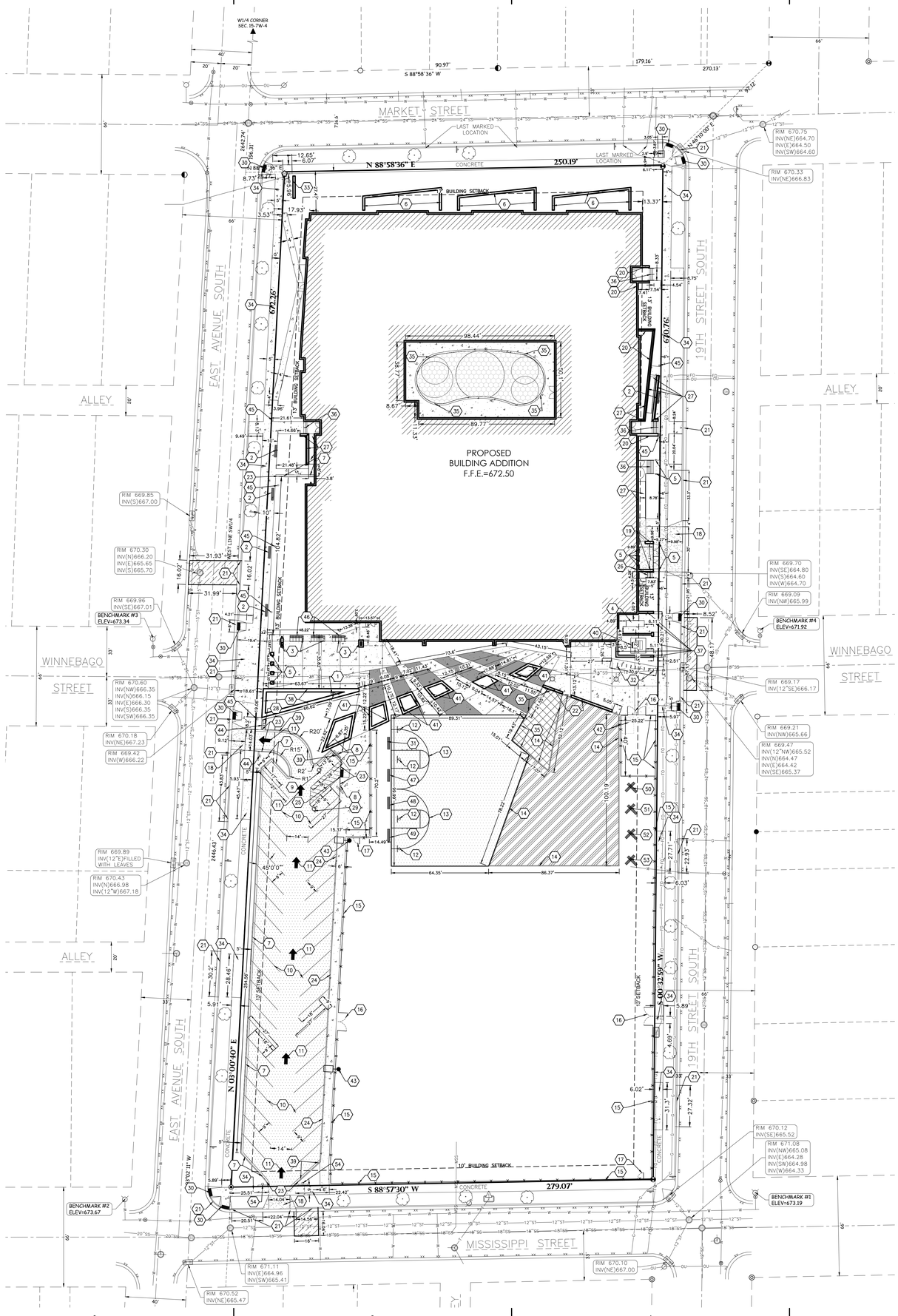
Project Number: 3752

Issued For: CONTRACT DOCUMENTS 10/28/2025

Sheet Title: Demolition Plan

Sheet Number: C1.0

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## GENERAL NOTES:

1. CONTRACTOR SHALL LOCATE ALL PUBLIC AND PRIVATE UTILITIES PRIOR TO COMMENCEMENT OF WORK.
2. GRADE, LINE, AND LEVEL TO BE REVIEWED IN THE FIELD BY THE CONSTRUCTION MANAGER.
3. ALL REQUIRED EROSION CONTROL MEASURES SHALL BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH LOCAL MUNICIPAL AND DEPARTMENT OF NATURAL RESOURCES REGULATIONS.
4. SEE SHEET C4.0 FOR ALL REQUIRED EROSION CONTROL ELEMENTS.
5. ANY EXISTING UTILITIES NOT SHOWN ON THIS DOCUMENT WHICH NEED TO BE REMOVED, RELOCATED AND OR ADJUSTED SHALL BE THE RESPONSIBILITY OF THE SITE GRADING CONTRACTOR.
6. VERIFY THE LOCATION OF ALL EXISTING UTILITIES PRIOR TO THE START OF DEMOLITION/CONSTRUCTION.
7. BIDDERS SHALL VISIT THE SITE AND REVIEW EXISTING CONDITIONS PRIOR TO THE BID DATE.
8. PRIOR TO STARTING WORK, VERIFY WITH THE LOCAL AUTHORITIES THAT ALL REQUIRED PERMITS HAVE BEEN ACQUIRED.
9. COORDINATE CONSTRUCTION IN THE RIGHT OF WAY WITH THE LOCAL AUTHORITIES.
10. PROVIDE PROPER BARRICADES, SIGNS, AND TRAFFIC CONTROL TO MAINTAIN THRU TRAFFIC ALONG ADJACENT STREETS IN ACCORDANCE WITH LOCAL MUNICIPAL REQUIREMENTS.
11. SIDEWALK JOINTS SHALL BE INSTALLED AS INDICATED OR AS APPROVED BY THE CONSTRUCTION MANAGER.
12. ALL GENERAL LANDSCAPE AREAS SHALL BE SEEDED, FERTILIZED, AND CRIMP HAY MULCHED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS.

## PAVEMENT HATCH PATTERNS:

- PROPOSED STANDARD ASPHALT PAVEMENT — 1 C6.0
- PROPOSED STANDARD CONCRETE PAVEMENT — 2 C6.0
- COLOR CONCRETE TYPE 1 COLOR: 'DARK GREY' BY DAVIS COLORS FINISH: LIGHT SAND BLAST — 2 C6.0
- PROPOSED REINFORCED CONCRETE PAVEMENT — 2 C6.0
- PROPOSED RUBBER SURFACE PLAY AREA (COURTYARD) — 11 C6.0, 9 C6.0
- PROPOSED WOOD CHIP PLAY AREA — 10 C6.0, 12 C6.0
- PROPOSED ASPHALT PAVEMENT– TO MEET CITY OF LA CROSSE STANDARDS SPECIFICATIONS AND DETAILS
- PROPOSED RUBBER SURFACE PLAY AREA (OUTSIDE OF SCHOOL) — 10 C6.0

## KEYNOTES:

1. FLAG POLE– AMERICAN BEACON EXTERNAL HALYARD SERIES 25' ANODIZED CLEAR FLAG POLE – (OR APPROVED EQUAL) INSTALL PER MANUFACTURER'S RECOMMENDATIONS — 7 C6.0
2. BENCH – TYPE ZB.LE.02 CONCRETE TECH BENCH BY WAUSAU TILE, COLOR: A20 WHITE FINISH: WEATHERSTONE(B) OPTION: CAST–IN SKATE STOPPERS
3. CANOPY PILLAR– SEE ARCHITECTURAL PLANS
4. GENERATOR PAD – SEE ELECTRICAL PLANS
5. REINFORCED CONCRETE BOLLARD– SEE ARCHITECTURAL PLANS
6. GARDEN WALL – SEE ARCHITECTURAL PLANS
7. 18" CONCRETE CURB & GUTTER–STANDARD — 3 C6.0
8. ADA PARKING SIGN — 8 C6.0
9. ADA PARKING STALL– STANDARD — 2 C6.1
10. PARKING LOT STRIPING — 2 C6.1
11. DIRECTIONAL ARROW — 1 C6.1
12. BASKETBALL HOOP– KEEPER GOALS PRODUCT ALL–AMER–IN–GL 60" IN–GROUND BASKETBALL HOOP (OR APPROVED EQUAL) INSTALL PER MANUFACTURER'S RECOMMENDATIONS. VERIFY LOCATION W/OWNER PRIOR TO INSTALLATION
13. PLAYGROUND STRIPING– 4" WIDE LINE, WHITE TRAFFIC PAINT (VERIFY LOCATION & DIMENSIONS W/OWNER PRIOR TO CONSTRUCTION)
14. PLAY AREA CURBING — 10 C6.0
15. 6' TALL CHAIN LINK FENCING, BLACK VINYL COATED — 5 C6.1, 6 C6.1
16. 12' SERVICE GATE, BLACK VINYL COATED — 4 C6.1
17. 4' PEDESTRIAN GATE, BLACK VINYL COATED — 3 C6.1
18. DRIVE APRON — 2 C6.3
19. DUMPSTER ENCLOSURE– SEE ARCHITECTURAL PLANS
20. ARCHITECTURAL WALL– SEE ARCHITECTURAL PLANS
21. CONCRETE CURB & GUTTER– TO MEET CITY OF LA CROSSE STANDARDS SPECIFICATIONS AND DETAILS — 4 C6.3
22. BENCH WALL – SEE ARCHITECTURAL PLANS
23. 6" CURB HEAD TAPER SECTION — 5 C6.0
24. THICKENED EDGE WALK — 4 C6.0
25. ADA CURB RAMP 'A' — 6 C6.0
26. CONCRETE STAIRS W/RAILING — 5 C6.2
27. RAMP & RAILING– SEE ARCHITECTURAL PLANS
28. STOP SIGN — 7 C6.1
29. ADA PARKING STALL– VAN ACCESSIBLE — 2 C6.1
30. ADA CURB RAMP 'B' — 1 C6.3
31. THEORY – THICK BEAM BENCH BY LANDSCAPE FORMS, LENGTH: 8 FOOT TYPE: THICK BENCH W/SOLID SEAT & 2 SKATE STOPS COLOR: APPLE RED FINISH: POWDERCOAT (ALTERNATE #4)
32. KEY BIKE RACK BY LANDSCAPE FORMS (9) COLOR: RED FINISH: POWDERCOAT, INSTALL PER MANUFACTURER'S RECOMMENDATIONS
33. MONUMENT SIGN– SEE ARCHITECTURAL PLANS
34. CONCRETE SIDEWALK– TO MEET CITY OF LA CROSSE STANDARD SPECIFICATIONS AND DETAILS — 2 C6.0
35. CONCRETE CURB ADJACENT TO POURED RUBBER SURFACE — 11 C6.0
36. CONCRETE STAIRS & RAILING– SEE ARCHITECUTAL PLANS
37. SCREEN WALL & GATES– SEE ARCHITECTURAL PLANS
38. PLANTER BENCH– SEE ARCHITECTURAL PLANS
39. 18" CONCRETE CURB & GUTTER –REJECT — 3 C6.0
40. TRANFORMER PAD– SEE ELECTRICAL PLANS
41. PLANTER BENCH– SEE ARCHITECTURAL PLANS. FILL INTERIOR OF PLANTER WITH 12" OF TOPSOIL. TOP OF TOPSOIL TO BE HELD 4" BELOW BOTTOM OF BENCH CAP.
42. UNDERGROUND GEOTHERMAL VAULT, SHOWN FOR REFERENCE ONLY. SEE GEOTHERMAL PLANS FOR DETAILS.
43. LIGHT POLE– SEE ELECTRICAL PLANS
44. 'DO NOT ENTER' SIGN — 7 C6.1
45. CONCRETE BOLLARD– SEE ARCHITECTURAL PLANS
46. BENCH– SEE ARCHITECTURAL PLANS
47. THEORY – THICK BEAM BENCH BY LANDSCAPE FORMS, LENGTH: 8 FOOT TYPE: THICK BENCH W/SOLID SEAT & 2 SKATE STOPS COLOR: NAVY BLUE FINISH: POWDERCOAT (ALTERNATE #4)
48. THEORY – THICK BEAM BENCH BY LANDSCAPE FORMS, LENGTH: 8 FOOT TYPE: THICK BENCH W/SOLID SEAT & 2 SKATE STOPS COLOR:LEAF GREEN FINISH: POWDERCOAT (ALTERNATE #4)
49. THEORY – THICK BEAM BENCH BY LANDSCAPE FORMS, LENGTH: 8 FOOT TYPE: THICK BENCH W/SOLID SEAT & 2 SKATE STOPS COLOR: LEMON FINISH: POWDERCOAT (ALTERNATE #4)
50. (2)THEORY – THICK BEAM BENCHES BY LANDSCAPE FORMS (LAYOUT 10) LENGTH: 8 FOOT TYPE: THICK BENCH W/SOLID SEAT & 2 SKATE STOPS COLOR: APPLE RED FINISH: POWDERCOAT (ALTERNATE #4)
51. (2) THEORY – THICK BEAM BENCHES BY LANDSCAPE FORMS (LAYOUT 10) LEGTH: 8 FOOT TYPE: THICK BENCH W/SOLID SEAT & 2 SKATE STOPS COLOR: NAVY BLUE FINISH: POWDERCOAT ALTERNATE #4
52. (2) THEORY – THICK BEAM BENCHES BY LANDSCAPE FORMS LENGTH: 8 FOOT TYPE: THICK BENCH W/SOLID SEAT & 2 SKATE STOPS COLOR: LEAF GREEN FINISH: POWDERCOAT (ALTERNATE #4)
53. (2) THEORY– THICK BEAMS BENCHES BY LANDSCAPE FORMS LENGTH: 8 FOOT TYPE: THICK BENCH W/SOLID SEAT & 2 SKATE STOPS COLOR: LEMON FINISH: POWDERCOAT (ALTERNATE #4)
54. 'ENTER ONLY' SIGN — 7 C6.1

## BENCHMARK:

ELEVATIONS ARE REFERENCED TO NAVD 88 DATUM.

**BENCHMARK #1**
NORTHWEST FLANGE BOLT ON HYDRANT,
LOCATED AT THE NORTHEAST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND 19TH STREET SOUTH.
ELEVATION = 673.19

**BENCHMARK #2**
NORTHEAST FLANGE BOLT ON HYDRANT,
LOCATED AT THE NORTHWEST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND EAST AVENUE SOUTH.
ELEVATION = 673.67

**BENCHMARK #3**
NORTHEAST FLANGE BOLT ON HYDRANT,
LOCATED AT THE NORTHWEST CORNER OF THE INTERSECTION OF EAST AVENUE SOUTH AND WINNEBAGO STREET.
ELEVATION = 673.34

**BENCHMARK #4**
BURY BOLT ON HYDRANT,
LOCATED AT THE NORTHEAST CORNER OF THE INTERSECTION OF 19TH STREET SOUTH AND WINNEBAGO STREET.

## UTILITY DISCLAIMER:

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0' 30' 60'
1" = 30'


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Project Title:
NEW BUILDING FOR:
NEW ELEMENTARY SCHOOL
LA CROSSE SCHOOL DISTRICT
807 EAST AVE S.
LA CROSSE, WI 54601

REVISIONS:

| | DATE | DESCRIPTION |
|---|---|---|

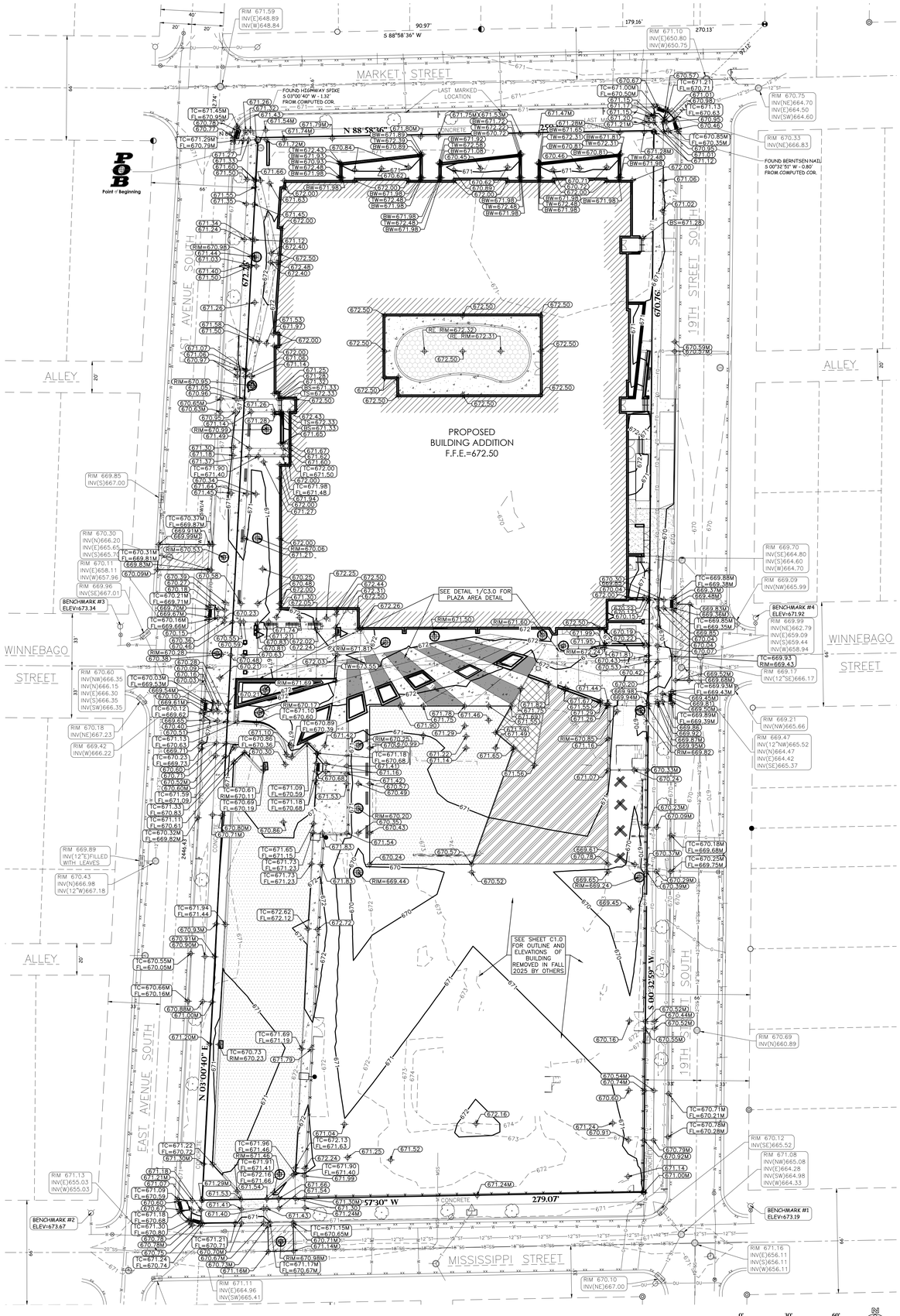
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Issued For: CONTRACT DOCUMENTS 10/28/2025

Sheet Title: Layout Plan

Sheet Number: C2.0

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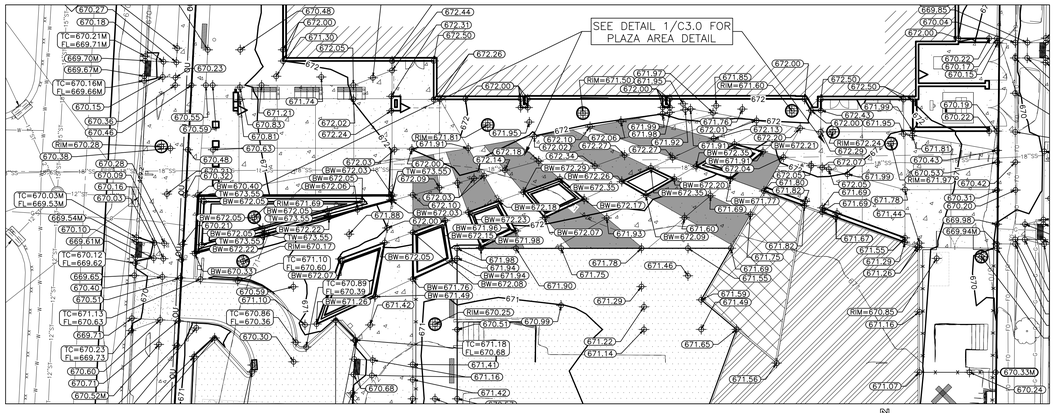



## GENERAL NOTES:

1. CONTRACTOR SHALL LOCATE ALL PUBLIC AND PRIVATE UTILITIES PRIOR TO COMMENCEMENT OF WORK.
2. THE PROPOSED SITE PLAN FINISH FLOOR ELEVATION OF 672.50 EQUALS THE PROPOSED BUILDING ARCHITECTURAL FINISH FLOOR ELEVATION OF 100.00'.
3. GRADE, LINE, AND LEVEL SHALL BE REVIEWED IN THE FIELD BY THE CONSTRUCTION MANAGER.
4. INSTALL AND MAINTAIN ALL REQUIRED EROSION CONTROL MEASURES IN ACCORDANCE WITH LOCAL MUNICIPAL AND DEPARTMENT OF NATURAL RESOURCES REGULATIONS.
5. 6" OF TOPSOIL SHALL BE PROVIDED IN ALL GENERAL LAWN AREAS AND 12" SHALL BE PROVIDED IN ALL PLANTING BED AREAS.
6. SEE SHEET C4.0 FOR ALL REQUIRED EROSION CONTROL ELEMENTS.
7. ANY EXISTING UTILITIES NOT SHOWN ON THIS DOCUMENT WHICH NEED TO BE REMOVED, RELOCATED, AND/OR ADJUSTED SHALL BE THE RESPONSIBILITY OF THE SITE GRADING CONTRACTOR.
8. COORDINATE ALL EARTHWORK ACTIVITIES WITH THE RESPECTIVE TRADES RESPONSIBLE FOR THE INSTALLATION OF GAS, CABLE, TELEPHONE AND ELECTRICAL (INCLUDING MAIN SERVICE, SITE LIGHTING, CONDUITS AND SIGNAGE).
9. INSTALL WisDOT TYPE HR FILTER FABRIC BENEATH ALL RIP RAP.
10. EXCESS TOPSOIL SHALL BE REMOVED FROM SITE, UNLESS OTHERWISE DIRECTED BY THE OWNER. COORDINATE WITH OWNER FOR LOCATION OF STOCKPILE IF THE OWNER CHOOSES TO SALVAGE EXCESS TOPSOIL FOR FUTURE USE. SILT FENCE SHALL BE PLACED AROUND STOCKPILE.
11. ALL TESTING AND INSPECTION SHALL BE DONE IN ACCORDANCE WITH SPS 382.21.
12. THE LOCAL MUNICIPALITY SHALL BE CONTACTED PRIOR TO ANY EXCAVATION IN THE PUBLIC RIGHT-OF-WAY.
13. THE CONTRACTOR SHALL HAVE A TRAFFIC CONTROL PLAN APPROVED PRIOR TO WORK COMMENCING.
14. THE LOCAL MUNICIPALITY SHALL OPERATE ALL EXISTING WATER VALVES IF NEEDED.
15. GRADES AT BUILDING EDGE SHALL BE 6" BELOW FINISHED FLOOR ELEVATION EXCEPT AT DOOR WAY ENTRANCES OR UNLESS OTHERWISE NOTED.

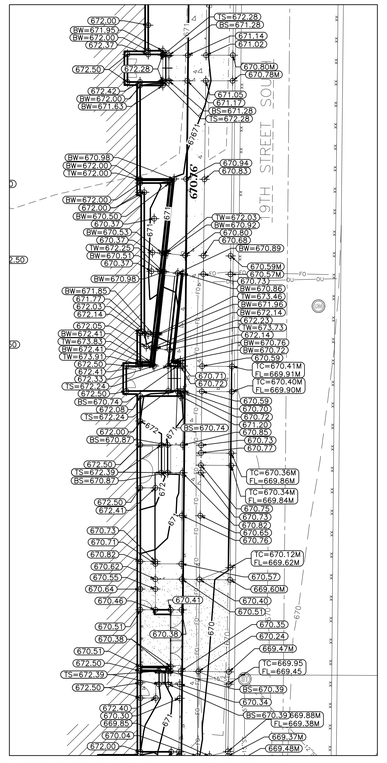
## GRADING LEGEND:

| | |
|---|---|
| EXISTING CONTOUR | 712 |
| PROPOSED CONTOUR | 712 |
| PROPOSED SPOT ELEVATION | 692.26 |
| PROPOSED RIM ELEVATION | RIM=893.56 |
| PROPOSED TOP OF CURB ELEVATION | TC=893.56 |
| PROPOSED FLOW LINE ELEVATION | FL=893.56 |
| PROPOSED TOP OF STAIR ELEVATION | TS=893.56 |
| PROPOSED BOTTOM OF STAIR ELEVATION | BS=893.56 |
| PROPOSED TOP OF WALL ELEVATION | TW=893.56 |
| PROPOSED BOTTOM OF WALL ELEVATION | BW=893.56 |
| PROPOSED MATCH ELEVATION (CONTRACTOR TO VERIFY) | 892.05M |
| PROPOSED RECESSED RIM ELEVATION | RE RIM=893.56 |
| PROPOSED STORM SEWER MANHOLE | ST (2 / C6.1) |
| PROPOSED STORM SEWER INLET | (7 / C6.1) |
| PROPOSED STORM SEWER CURB INLET | (1 / C6.1) |
| PROPOSED SANITARY MANHOLE | SS (3 / C6.2) |
| PROPOSED STORM SEWER INLET (SEE INTERIOR PLUMBING PLAN) | (9 / C6.0) |




## PLAZA AREA DETAILS

1 / C3.0 — 0' 20' 40' — 1" = 20'



## EAST SIDE BLOW UP DETAIL

2 / C3.0 — 0' 20' 40' — 1" = 20'

## BENCHMARKS:

ELEVATIONS ARE REFERENCED TO NAVD 88 DATUM.

**BENCHMARK #1**
NORTHWEST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHEAST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND 19TH STREET SOUTH.
ELEVATION = 673.19

**BENCHMARK #2**
NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND EAST AVENUE SOUTH.
ELEVATION = 673.67

**BENCHMARK #3**
NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST CORNER OF THE INTERSECTION OF EAST AVENUE SOUTH AND WINNEBAGO STREET.
ELEVATION = 673.34

**BENCHMARK #4**
BURY BOLT ON HYDRANT, LOCATED AT THE NORTHEAST CORNER OF THE INTERSECTION OF 19TH STREET SOUTH AND WINNEBAGO STREET.
ELEVATION = 671.92

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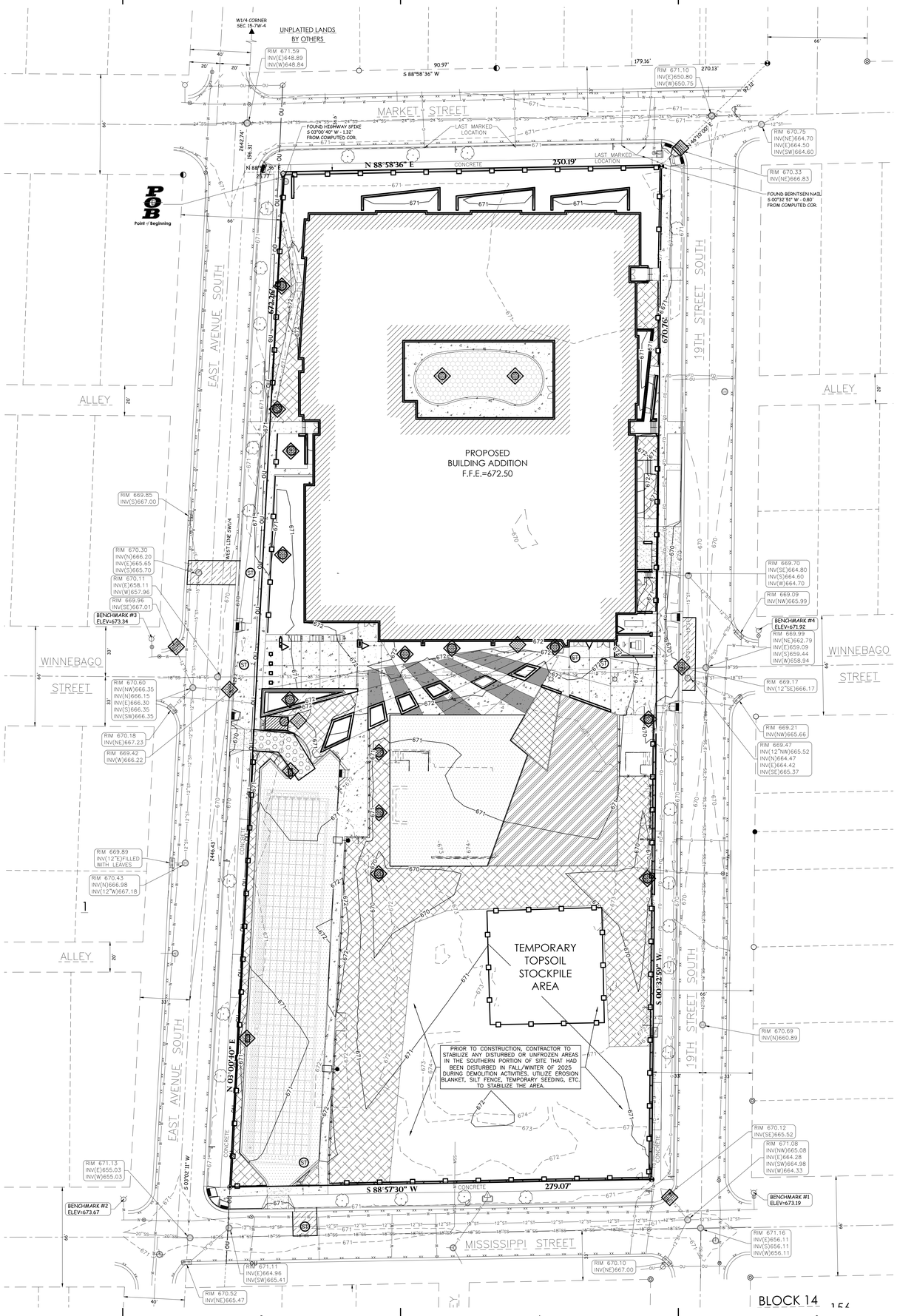
Project Number:
3752

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CONTRACT DOCUMENTS
10/28/2025

Sheet Title:
Grading Plan

Sheet Number:
C3.0

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## GENERAL NOTES:

1. CONTRACTOR SHALL LOCATE ALL PUBLIC AND PRIVATE UTILITIES PRIOR TO COMMENCEMENT OF WORK.
2. NOTIFY THE LOCAL MUNICIPALITY AT LEAST 2 WORKING DAYS PRIOR TO THE START OF SOIL DISTURBING ACTIVITIES.
3. INSTALL ALL TEMPORARY EROSION CONTROL ELEMENTS PRIOR TO THE START OF DEMOLITION/CONSTRUCTION.
4. ALL ACTIVITIES SHALL BE CONDUCTED IN A LOGICAL SEQUENCE TO MINIMIZE THE AMOUNT OF BARE SOIL EXPOSED AT ANY ONE TIME. MAINTAIN EXISTING VEGETATION AS LONG AS POSSIBLE.
5. CRUSHED ROCK DRIVES FOR SEDIMENT TRACKING UTILIZING 3" CRUSHED ROCK SHALL BE MAINTAINED AT ALL CONSTRUCTION ENTRANCES TO THE SITE. THE ROCK DRIVE SHALL BE A MINIMUM OF 12" THICK AND BE A MINIMUM OF 50 FEET IN LENGTH BY THE WIDTH OF THE DRIVEWAY.
6. OFFSITE SEDIMENT DEPOSITS RESULTING FROM STORMWATER RUNOFF SHALL BE CLEANED BY THE END OF THE NEXT WORKDAY. OFFSITE SEDIMENT DEPOSITS RESULTING FROM CONTRACTOR ACTIVITIES, INCLUDING SOIL TRACKING, SHALL BE CLEANED EACH WORKDAY. EXCESSIVE AMOUNTS OF SEDIMENT OR DEBRIS TRACKED ONTO ADJACENT STREETS SHALL BE CLEANED IMMEDIATELY. FINE SEDIMENT ACCUMULATIONS ON ADJACENT STREETS SHALL SWEPT MECHANICALLY OR MANUALLY AT LEAST WEEKLY AND BEFORE IMMINENT RAINFALL.
7. DISTURBED GROUND OUTSIDE OF THE EVERYDAY CONSTRUCTION AREAS, INCLUDING SOIL STOCKPILES, THAT ARE LEFT INACTIVE FOR MORE THAN 7 DAYS SHALL BE TEMPORARILY STABILIZED BY SEEDING/MULCHING OR OTHER APPROVED METHODS.
8. WASTE MATERIAL THAT IS GENERATED ON THE CONSTRUCTION SITE SHALL BE PROPERLY DISPOSED OF AND NOT ALLOWED TO RUN INTO RECEIVING WATERS.
9. EROSION CONTROL DEVICES DESTROYED AS A RESULT OF CONSTRUCTION ACTIVITIES SHALL BE REPAIRED BY THE END OF EACH WORK DAY.
10. INSPECT ALL EROSION CONTROL MEASURES AT LEAST ONCE A WEEK AND AFTER ANY RAINFALL OF 0.5" OR MORE. MAKE NEEDED REPAIRS AND DOCUMENT ALL ACTIVITIES AS PER THE REQUIREMENTS OF THE NOTICE OF INTENT SUBMITTED BY THE PROJECT CIVIL ENGINEER.
11. ALL TEMPORARY EROSION CONTROL ELEMENTS SHALL REMAIN IN PLACE UNTIL A SUFFICIENT GROWTH OF VEGETATION IS ESTABLISHED AND THEN BE REMOVED AS PART OF THE BASE BID.
12. IF SEDIMENT LADEN WATER NEEDS TO BE REMOVED FROM THE SITE, FILTER BAGS OR SCREENING SHALL BE USED IN ACCORDANCE WITH WI DNR TECHNICAL STANDARD 1061 TO PREVENT SEDIMENT DISCHARGE TO THE MAXIMUM EXTENT PRACTICABLE.
13. COORDINATE ALL EARTHWORK ACTIVITIES WITH THE RESPECTIVE TRADES RESPONSIBLE FOR THE INSTALLATION OF GAS, CABLE, TELEPHONE AND ELECTRICAL (INCLUDING MAIN SERVICE, SITE LIGHTING, CONDUITS AND SIGNAGE).
14. INSTALL WisDOT TYPE HR FILTER FABRIC BENEATH ALL RIP RAP.
15. IF BARE SOIL IS EXPOSED DURING THE WINTER MONTHS, STABILIZATION BY MULCHING OR ANIONIC POLYACRYLAMIDE SHALL OCCUR PRIOR TO SNOWFALL OR GROUND FREEZE.
16. SILT FENCE SHALL BE INSTALLED AROUND THE TOPSOIL STOCKPILE.
17. THE CONTRACTOR SHALL PERFORM INSPECTIONS AND MONITORING OF EROSION CONTROL PRACTICES IN ACCORDANCE WITH THE WI DNR "CONSTRUCTION SITE INSPECTION REPORT" FORM 3400–187. THIS FORM CAN BE FOUND IN THE CONSTRUCTION SPECIFICATIONS.
18. PRIOR TO CONSTRUCTION, CONTRACTOR TO STABILIZE ANY DISTURBED OR UNFROZEN AREAS IN THE SOUTHERN PORTION OF SITE THAT HAD BEEN DISTURBED IN FALL/WINTER OF 2025 DURING DEMOLITION ACTIVITIES.

## EROSION CONTROL LEGEND:

EXISTING CONTOUR — — — 888 — — —

PROPOSED CONTOUR 888

PROPOSED SILT FENCE — 9 / C6.1

PROPOSED INLET PROTECTION — 10 / C6.1

EROSION CONTROL BLANKET — 11 / C6.1

ROCK CONSTRUCTION ENTRANCE — 12 / C6.1

CONCRETE WASHOUT AREA — 8 / C6.1

STORM WATER OVERLAND FLOW DIRECTION →

## EROSION CONTROL SEQUENCING:

1. INSTALL PERIMETER EROSION CONTROL
2. BEGIN DEMOLITION
3. BEGIN ROUGH GRADING AND UTILITY INSTALLATION
4. DURING GRADING ACTIVITIES EXISTING GRASS AND VEGETATION, TO BE REMOVED, SHALL REMAIN IN PLACE FOR AS LONG AS POSSIBLE, TO AVOID SEDIMENT TRANSPORT.
5. TEMPORARY STABILIZATION ACTIVITY SHALL COMMENCE WHEN LAND DISTURBING CONSTRUCTION ACTIVITIES HAVE TEMPORARILY CEASED AND WILL NOT RESUME FOR A PERIOD EXCEEDING 14 CALENDAR DAYS.
6. FINAL STABILIZATION ACTIVITY SHALL COMMENCE WHEN LAND DISTURBING ACTIVITIES CEASE AND FINAL GRADE HAS BEEN REACHED ON ANY PORTION OF THE SITE.
7. IF DISTURBED AREAS MUST BE LEFT OVER WINTER, AN ANIONIC POLYACRYLAMIDE SHALL BE APPLIED TO ALL DISTURBED AREAS PRIOR TO GROUND FREEZE. SEE SPECIFICATIONS FOR DETAILS.

## BENCHMARK:

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**BENCHMARK #1**
NORTHWEST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHEAST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND 19TH STREET SOUTH.
ELEVATION = 673.19

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ELEVATION = 673.67

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NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST CORNER OF THE INTERSECTION OF EAST AVENUE SOUTH AND WINNEBAGO STREET.
ELEVATION = 673.34

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BURY BOLT ON HYDRANT, LOCATED AT THE NORTHEAST CORNER OF THE INTERSECTION OF 19TH STREET SOUTH AND WINNEBAGO STREET.
ELEVATION = 671.92

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0' 30' 60'
1" = 30'

N



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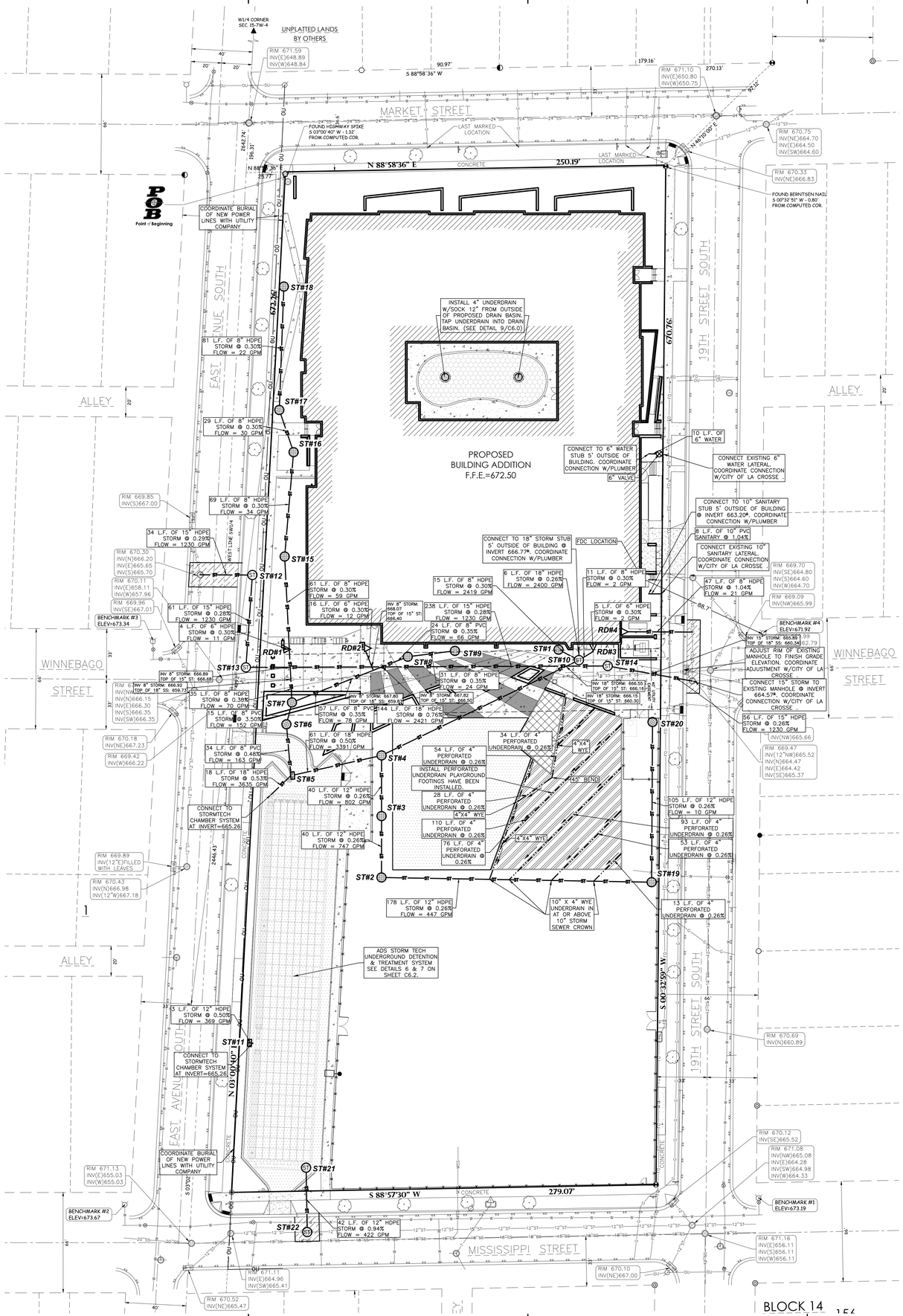
Project Number:
3752

Issued For:
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10/28/2025

Sheet Title:
Erosion Control Plan

Sheet Number:
C4.0

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## GENERAL NOTES:

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2. GRADE, LINE, AND LEVEL SHALL BE REVIEWED IN THE FIELD BY THE CONSTRUCTION MANAGER.
3. ANY EXISTING UTILITIES NOT SHOWN ON THIS DOCUMENT WHICH NEED TO BE REMOVED, RELOCATED AND OR ADJUSTED SHALL BE THE RESPONSIBILITY OF THE SITE GRADING CONTRACTOR.
4. REFER TO THE PROPOSED BUILDING MECHANICAL/PLUMBING PLANS TO VERIFY EXACT CONNECTION LOCATIONS AND SIZES OF PROPOSED SANITARY SEWER AND WATER LATERALS.
5. COORDINATE ALL UTILITY WORK WITH THE RESPECTIVE TRADES RESPONSIBLE FOR THE INSTALLATION OF GAS, CABLE, TELEPHONE AND ELECTRICAL (INCLUDING MAIN SERVICE, SITE LIGHTING, CONDUITS AND SIGNAGE).
6. COORDINATE UTILITY SERVICE DISCONNECTIONS/OUTAGES WITH OWNER AND ANY IMPACTED NEIGHBORS. MINIMIZE DISRUPTIONS TO THE MAXIMUM EXTENT PRACTICAL.
7. COORDINATE ALL WORK WITHIN THE PUBLIC RIGHT OF WAY WITH THE LOCAL MUNICIPALITY.
8. ALL TESTING AND INSPECTION SHALL BE DONE IN ACCORDANCE WITH SPS 382.21.
9. THE PROPOSED WATER MAIN SHALL HAVE A MINIMUM COVER OF 7'-6" TO THE TOP OF PIPE FROM PROPOSED FINISHED GRADE. SEE SHEET C3.0 FOR PROPOSED FINISHED GRADE.
10. THE MUNICIPALITY SHALL BE CONTACTED PRIOR TO ANY EXCAVATION IN THE PUBLIC RIGHT-OF-WAY, AND PRIOR TO CONNECTING SANITARY SEWER AND WATER LATERALS TO THE PUBLIC MAINS.
11. THE CONTRACTOR SHALL HAVE A TRAFFIC CONTROL PLAN APPROVED PRIOR TO WORK COMMENCING.
12. THE MUNICIPALITY SHALL OPERATE ALL EXISTING WATER VALVES, IF NEEDED.
13. FIELD VERIFY INVERT ELEVATION OF THE SANITARY SEWER AND WATER PUBLIC MAIN, AT THE LOCATION OF THE SERVICE LATERAL CONNECTIONS, PRIOR TO CONNECTING THE LATERALS TO THE PUBLIC MAIN.
14. INSTALL WisDOT TYPE HR FILTER FABRIC BENEATH PROPOSED RIP RAP.
15. CONTRACTOR TO COORDINATE REMOVAL OF EXISTING OVERHEAD POWER LINES SHOWN IN DEMOLITION PLAN AND BURIAL OF NEW POWER LINES.

## UTILITY LEGEND:

- PROPOSED STORM SEWER
- PROPOSED SANITARY SEWER
- PROPOSED WATER MAIN
- PROPOSED STORM SEWER MANHOLE (ST) 2 C6.2
- PROPOSED STORM SEWER INLET 2 C6.2
- PROPOSED STORM SEWER CURB INLET 1 C6.2, 6 C6.3
- PROPOSED WATER VALVE 4 C6.2
- PROPOSED 4" UNDERDRAIN 9 C6.0, 12 C6.0
- PROPOSED SANITARY MANHOLE (SS) 3 C6.2
- PROPOSED STORM SEWER INLET (SEE INTERIOR PLUMBING PLAN) 9 C6.0
- PROPOSED OVERHEAD POWER-INSTALLED BY OTHERS, SHOWN FOR REFERENCE ONLY — OU —

## STORM MANHOLE SCHEDULE:

| STRUCTURE # | STRUCTURE DETAILS |
|---|---|
| ST#1 | RIM = 671.60<br>INV (SE) = 667.63<br>DEPTH = 3.97'<br><br>48" I.D. PRECAST MANHOLE W/NEENAH R-2560-EA CASTING W/BEEHIVE GRATE |
| ST#2 | RIM = 669.44<br>INV (N) = 666.37<br>INV (E) = 666.37<br>DEPTH = 3.07'<br><br>36" I.D. PRECAST MANHOLE W/NEENAH R-2560-EA CASTING W/BEEHIVE GRATE |
| ST#3 | RIM = 670.20<br>INV (N) = 666.27<br>INV (S) = 666.27<br>DEPTH = 3.93'<br><br>36" I.D. PRECAST MANHOLE W/NEENAH R-2560-EA CASTING W/BEEHIVE GRATE |
| ST#4 | RIM = 670.25<br>INV (S) = 666.16<br>INV (W) = 665.66<br>INV (NE) = 665.66<br>DEPTH = 4.59'<br><br>48" I.D. PRECAST MANHOLE W/NEENAH R-2560-EA CASTING W/BEEHIVE GRATE |
| ST#5 | RIM = 670.10<br>INV (N) = 666.19<br>INV (E) = 665.36<br>INV (SW) = 665.36<br>DEPTH = 4.75'<br><br>48" I.D. PRECAST MANHOLE W/NEENAH R- 3067 CASTING W/TYPE "R" GRATE<br>ADD UP-FLO FILTER TO MANHOLE<br>SEE DETAIL 6/C6.3 |
| ST#6 | RIM = 670.17<br>INV (N) = 666.35<br>INV (S) = 666.35<br>DEPTH = 3.81'<br><br>36" I.D. PRECAST MANHOLE W/NEENAH R-2560-EA CASTING W/BEEHIVE GRATE |
| ST#7 | RIM = 671.69<br>INV (E) = 667.65<br>INV (S) = 666.87<br>INV (N) = 666.87<br>DEPTH = 4.82'<br><br>36" I.D. PRECAST MANHOLE W/NEENAH R-2560-EA CASTING W/BEEHIVE GRATE |
| ST#8 | RIM = 671.81<br>INV (E) = 667.94<br>INV (W) = 667.94<br>DEPTH = 3.87'<br><br>48" I.D. PRECAST MANHOLE W/NEENAH R-2560-EA CASTING W/BEEHIVE GRATE |

| STRUCTURE # | STRUCTURE DETAILS |
|---|---|
| ST#9 | RIM = 671.50<br>INV (W) = 668.05<br>DEPTH = 3.45'<br><br>48" I.D. PRECAST MANHOLE W/NEENAH R-2560-EA CASTING W/BEEHIVE GRATE |
| ST#10 | RIM = 672.24<br>INV (SW) = 666.76<br>INV (NE) = 667.59<br>INV (NW) = 667.59<br>INV (N) = 666.76<br>DEPTH = 5.49'<br><br>60" I.D. PRECAST MANHOLE NEENAH CASTING R-1733-1 W/SOLID COVER |
| ST#11 | RIM = 670.23<br>INV (E) = 665.28<br>DEPTH = 4.95'<br><br>2'X3' BOX CATCH BASIN W/NEENAH R-3067 CASTING W/TYPE "R" GRATE<br>ADD UP-FLO FILTER TO MANHOLE<br>SEE DETAIL 6/C6.3 |
| ST#12 | RIM = 670.53<br>INV (S) = 665.55<br>INV (W) = 665.55<br>DEPTH = 4.98'<br><br>48" I.D. PRECAST MANHOLE NEENAH CASTING R-1733-1 W/SOLID COVER |
| ST#13 | RIM = 670.28<br>INV (E) = 665.38<br>INV (N) = 665.38<br>DEPTH = 4.90'<br><br>48" I.D. PRECAST MANHOLE NEENAH CASTING R-1733-1 W/SOLID COVER |
| ST#14 | RIM = 671.97<br>INV (E) = 664.71<br>INV (W) = 664.71<br>DEPTH = 7.26'<br><br>48" I.D. PRECAST MANHOLE NEENAH CASTING R-1733-1 W/SOLID COVER |
| ST#15 | RIM = 670.06<br>INV (S) = 667.16<br>INV (N) = 667.16<br>DEPTH = 2.90'<br><br>36" I.D. PRECAST MANHOLE W/NEENAH R-2560-EA CASTING W/BEEHIVE GRATE |
| ST#16 | RIM = 670.99<br>INV (S) = 667.37<br>INV (N) = 667.37<br>DEPTH = 3.62'<br><br>36" I.D. PRECAST MANHOLE W/NEENAH R-2560-EA CASTING W/BEEHIVE GRATE |

| STRUCTURE # | STRUCTURE DETAILS |
|---|---|
| ST#17 | RIM = 670.95<br>INV (S) = 667.46<br>INV (N) = 667.46<br>DEPTH = 3.50'<br><br>36" I.D. PRECAST MANHOLE W/NEENAH R-2560-EA CASTING W/BEEHIVE GRATE |
| ST#18 | RIM = 671.15<br>INV (S) = 667.70<br>DEPTH = 3.45'<br><br>36" I.D. PRECAST MANHOLE W/NEENAH R-2560-EA CASTING W/BEEHIVE GRATE |
| ST#19 | RIM = 669.24<br>INV (W) = 666.84<br>INV (N) = 666.84<br>DEPTH = 2.41'<br><br>36" I.D. PRECAST MANHOLE W/NEENAH R-2560-EA CASTING W/BEEHIVE GRATE |
| ST#20 | RIM = 669.82<br>INV (S) = 667.11<br>DEPTH = 2.71'<br><br>36" I.D. PRECAST MANHOLE W/NEENAH R-2560-EA CASTING W/BEEHIVE GRATE |
| ST#21 | RIM = 673.23<br>INV (S) = 665.26<br>INV (W) = 665.26<br>INV (N) = 665.26<br>DEPTH = 7.97'<br><br>60" I.D. PRECAST MANHOLE NEENAH CASTING R-1733-1 W/SOLID COVER |
| ST#22 | RIM = 670.98<br>INV (N) = 664.86<br>INV (W) = 664.86<br>INV (E) = 664.86<br>DEPTH = 6.12'<br><br>48" I.D. PRECAST MANHOLE NEENAH CASTING R-1733-1 W/SOLID COVER |

## DOWNSPOUT CONNECTION SCHEDULE:

| STRUCTURE # | STRUCTURE DETAILS |
|---|---|
| RD#1 | RIM = 670.80<br>INV (E) = 667.65 |
| RD#2 | RIM = 672.25<br>INV (S) = 668.07 |
| RD#3 | RIM = 672.50<br>INV (S) = 667.64 |
| RD#4 | RIM = 670.22<br>INV (E) = 665.60 |

## BENCHMARK:

ELEVATIONS ARE REFERENCED TO NAVD 88 DATUM.

**BENCHMARK #1**
NORTHWEST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHEAST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND 19TH STREET SOUTH.
ELEVATION = 673.19

**BENCHMARK #2**
NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND EAST AVENUE SOUTH.
ELEVATION = 673.67

**BENCHMARK #3**
NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST CORNER OF THE INTERSECTION OF EAST AVENUE SOUTH AND WINNEBAGO STREET.
ELEVATION = 673.34

**BENCHMARK #4**
BURY BOLT ON HYDRANT, LOCATED AT THE NORTHEAST CORNER OF THE INTERSECTION OF 19TH STREET SOUTH AND WINNEBAGO STREET.
ELEVATION = 671.92

## UTILITY DISCLAIMER:

THE LOCATIONS, SIZES, AND TYPES OF UNDERGROUND PUBLIC AND PRIVATE UTILITIES OR SUBSTRUCTURES SHOW HEREON WERE OBTAINED FROM VISUAL INSPECTION, FIELD MEASUREMENTS, AND/OR AS-BUILT PLANS. SANITARY SEWER AND STORM SEWER PIPE SIZES, INVERTS, DIRECTION, AND LOCATIONS BETWEEN MANHOLES ARE SUPPLEMENTED BY AS-BUILT PLANS AND/OR ESTIMATED BASED ON FIELD OBSERVATIONS. PRIOR TO CONSTRUCTION IN THE VICINITY OF ANY UTILITIES SHOWN HEREON, IT IS RECOMMENDED THAT THE LOCATIONS, DEPTHS, AND SIZES BE FIELD VERIFIED. THE LOCATIONS SHOWN HEREON ARE ONLY APPROXIMATE, WITH POSSIBILITY THAT ADDITIONAL UTILITY LINES NOT DISCOVERED, OR MARKED, DURING THE SEARCH OF RECORDS AND THE FIELD SURVEY MAY EXIST. ANY CONTRACTOR USING THE INFORMATION SHOWN HEREON IS HEREBY FOREWARNED THAT ANY EXCAVATION UPON THIS SITE MAY RESULT IN THE DISCOVERY OF ADDITIONAL UNDERGROUND UTILITIES NOT SHOWN HEREON. IN GENERAL, UNDERGROUND UTILITY LOCATIONS ARE SHOWN FROM UTILITY MARKINGS, BY OTHERS, AND/OR AS-BUILT PLANS, PROVIDED BY OTHERS. POINT OF BEGINNING MAKES NO WARRANTY OF ANY KIND, EXPRESS OR IMPLIED, WITH RESPECT TO THE EXISTING UTILITIES SHOWN HEREON, AND BELIEVES THAT THE INFORMATION CONTAINED HEREIN IS RELIABLE AND GENERALLY ACCURATE FOR THE PURPOSE INTENDED.

0' 30' 60'
1" = 30'

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Project Title:
NEW BUILDING FOR:
NEW ELEMENTARY SCHOOL
LA CROSSE SCHOOL DISTRICT
807 EAST AVE S.
LA CROSSE, WI 54601

REVISIONS:

| | DATE | DESCRIPTION |
|---|---|---|

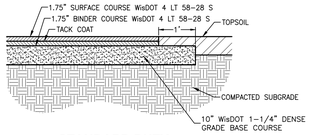
Project Number: 3752

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Sheet Title: Utility Plan

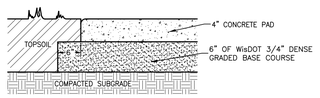
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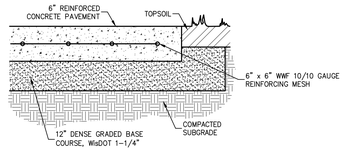



STANDARD DUTY

## ASPHALT PAVEMENT — 1 / C6.0

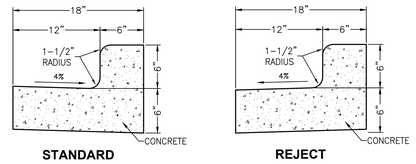



CONCRETE WALK



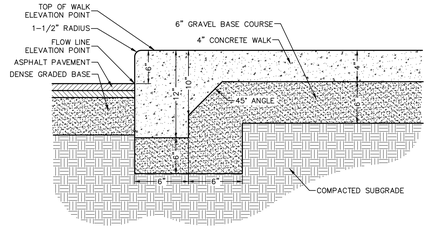

REINFORCED CONCRETE

## CONCRETE PAVEMENT — 2 / C6.0

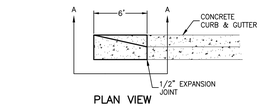



STANDARD REJECT

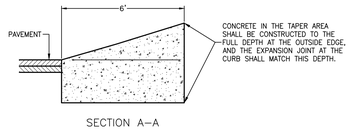
## CONCRETE CURB & GUTTER — 3 / C6.0




## THICKENED EDGE WALK — 4 / C6.0

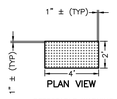



PLAN VIEW




SECTION A–A

## CURB HEAD TAPER SECTION — 5 / C6.0

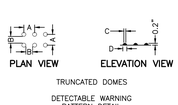



PLAN VIEW

DETECTABLE WARNING FIELD (TYPICAL)

| | MIN. | MAX. |
|---|---|---|
| A | 1.5" | 2.4" |
| B | 0.65" | 1.5" |
| C | * | * |
| D | 0.9" | 1.4" |

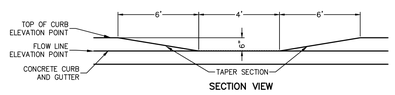
* THE C DIMENSION IS 50% TO 65% OF THE D DIMENSION




PLAN VIEW ELEVATION VIEW

TRUNCATED DOMES

DETECTABLE WARNING PATTERN DETAIL

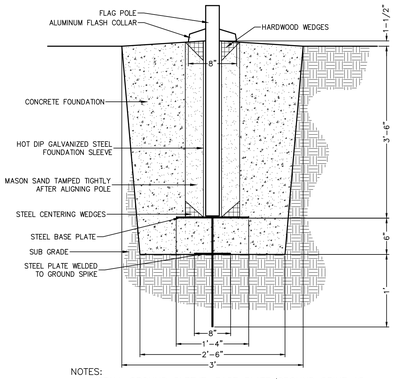



SECTION VIEW




PLAN VIEW

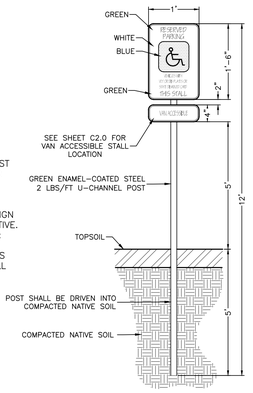
## ADA RAMP — 6 / C6.0



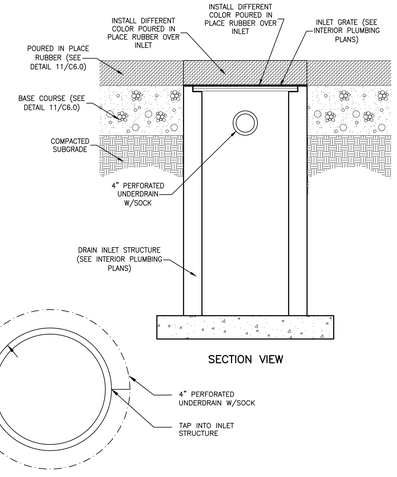

NOTES:
1. INSTALL GROUND SLEEVE, BASE PLATE/GROUND SPIKE AS OBTAINED FROM MANUFACTURER
2. VERIFY INSTALLATION PER MANUFACTURER'S SPECIFICATIONS

## FLAG POLE FOUNDATION — 7 / C6.0

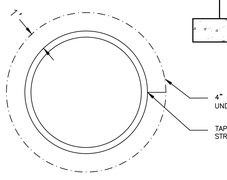
NOTES:
1. IN ACCORDANCE WITH WI TRANS 200.07(2)(a), THE SIGN SHALL CONSIST OF A WHITE RECTANGLE WITH LONGER DIMENSION VERTICAL, HAVING GREEN MESSAGE, AND A BLUE AND WHITE INTERNATIONAL SYMBOL FOR THE BARRIER–FREE ENVIRONMENTS. THE SIGN MAY BE REFLECTIVE OR NON–REFLECTIVE.
2. THE SIGN MUST INCLUDE THE WORDS:
   –RESERVED PARKING
   –VEHICLES WITH VET OR DIS PLATES OR STATE DISABLED CARD THIS STALL




## ADA PARKING SIGN — 8 / C6.0

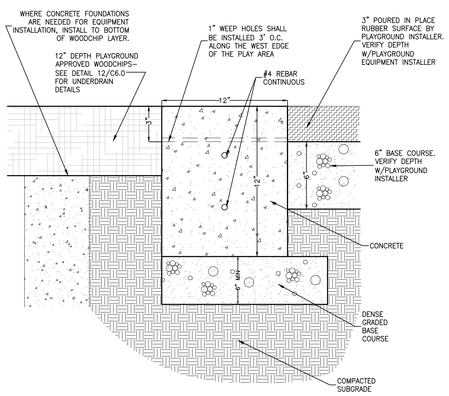



SECTION VIEW



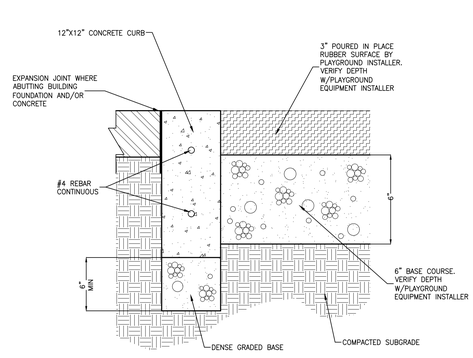

PLAN VIEW

## RUBBER SURFACE UNDERDRAIN — 9 / C6.0



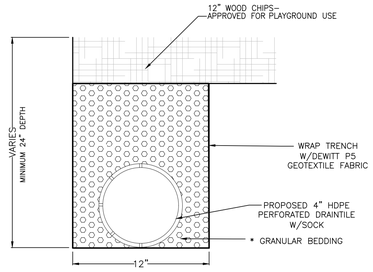

***NOTES:
- CONTRACTOR TO COORDINATE INSTALLATION OF CONCRETE CURB WITH INSTALLATION OF POURED RUBBER SURFACE.
- CONTRACTOR TO COORDINATE DEPTH OF CONCRETE CURB WITH CONTRACTOR INSTALLING POURED RUBBER SURFACE.

## 12"X12" CONCRETE CURB W/ WEEP HOLES — 10 / C6.0




***NOTES:
- CONTRACTOR TO COORDINATE INSTALLATION OF CONCRETE CURB WITH INSTALLATION OF POURED RUBBER SURFACE.
- CONTRACTOR TO COORDINATE DEPTH OF CONCRETE CURB WITH CONTRACTOR INSTALLING POURED RUBBER SURFACE.

## 12"X12" CONCRETE CURB ADJACENT TO RUBBER SURFACE — 11 / C6.0




NOTES:
* THE GRANULAR BEDDING 3/4" PEA GRAVEL OR GRANULAR FILL AS APPROVED BY ENGINEER

–SEE PLAN FOR LOCATIONS/SIZES/INVERT ELEVATIONS

## PLAY AREA UNDERDRAIN — 12 / C6.0

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Project Title:
NEW BUILDING FOR:
NEW ELEMENTARY SCHOOL
LA CROSSE SCHOOL DISTRICT
807 EAST AVE S.
LA CROSSE, WI 54601

REVISIONS:

| | DATE | DESCRIPTION |
|---|---|---|

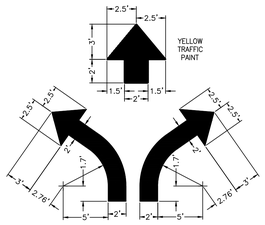
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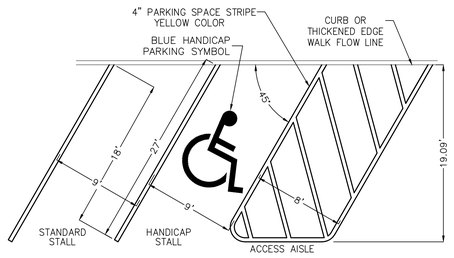
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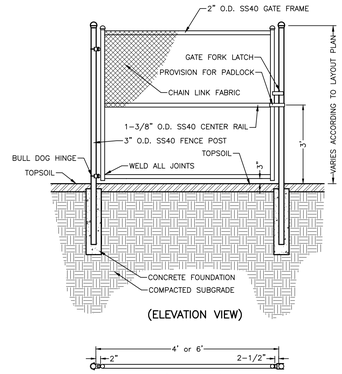
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## DIRECTIONAL ARROWS — 1 / C6.1

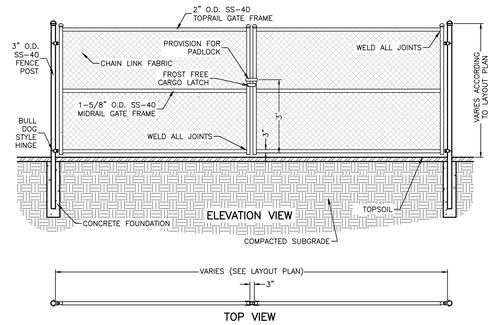



## PARKING LOT STRIPING — 2 / C6.1




NOTES:
1. SEE LAYOUT PLAN FOR GATE LOCATIONS

## PEDESTRIAN GATE — 3 / C6.1




NOTES:
1. SEE LAYOUT PLAN FOR GATE LOCATIONS

## SERVICE GATE — 4 / C6.1




NOTES:
1. ALL GATES WITH A SWING OF 6' OR LARGER SHALL HAVE A CONCRETE FOUNDATION AS SHOWN ABOVE.
2. SEE LAYOUT PLAN FOR GATE LOCATIONS

## CORNER FENCE POST — 5 / C6.1




NOTES:
1. SEE LAYOUT PLAN FOR FENCE LOCATIONS.
2. ALL CHAIN LINK FENCE POSTS SHALL BE DRIVEN.
3. DRIVEN FENCE POSTS INSTALLED WITHIN CONCRETE SHALL BE:
   3.1. CORED, DRIVEN, AND SEALED WITH CONCRETE JOINT SEALANT; OR
   3.2. SET PRIOR TO POURING CONCRETE, WRAPPED WITH EXPANSION FOAM, AND SEALED WITH CONCRETE JOINT SEALANT (SILICONE JOINT SEALANT MEETING ASTM C-920 TYPE S CLASS 25)
   3.3. BEFORE DRIVING A FENCE POST IN A CONCRETE AREA, IF THE FENCE POST IS LOCATED WITHIN 2 FEET OF THE EDGE OF CONCRETE, SAWCUT A CONTROL JOINT FROM THE FENCE POST TO THE EDGE OF CONCRETE.
4. DRIVEN FENCE POSTS INSTALLED WITHIN ASPHALT PAVEMENT SHALL BE CORED, DRIVEN, AND SEALED WITH CONCRETE JOINT SEALANT.

## DRIVEN FENCE POST — 6 / C6.1




| POST EMBEDMENT DEPTH | |
|---|---|
| AREA OF SIGN INSTALLATION (SQ. FT.) | D (MIN.) |
| 20 OR LESS | 4' |
| > THAN 20 | 5' |

NOTES:
1. IF APPLICABLE, SIGNS SHALL CONFORM TO MUTCD STANDARDS INCLUDING RETRO REFLECTIVITY STANDARDS.
2. SIGNS WIDER THAN 4 FEET OR LARGER THAN 20 SQ. FT. SHALL BE MOUNTED ON MULTIPLE POSTS.

## SIGN INSTALLATION — 7 / C6.1




NOTES:
1. DO NOT LOCATE AN UNLINED CONCRETE WASHOUT AREA (CWA) WITHIN 400' OF ANY NATURAL DRAINAGE PATHWAY OR WATERBODY. DO NOT LOCATE WITHIN 1,000 OF ANY WELLS OR DRINKING WATER SOURCES. IF SITE CONSTRAINTS MAKE THIS INFEASIBLE, OR IF HIGHLY PERMEABLE SOILS EXIST ON SITE, THE CWA MUST BE INSTALLED WITH AN IMPERMEABLE LINER (16 MIL MIN. THICKNESS) OR SURFACE STORAGE ALTERNATIVES USING PREFABRICATED CONCRETE WASHOUT DEVICES OR A LINED ABOVE GROUND STORAGE SHOULD BE USED.
2. THE CWA SHALL BE INSTALLED PRIOR TO CONCRETE PLACEMENT ON SITE.
3. CWA SHALL INCLUDE A FLAT SUBSURFACE PIT THAT IS AT LEAST 8' BY 8' WITH SLOPES LEADING OUT OF THE SUBSURFACE PIT SHALL BE 3:1. THE PIT SHALL BE AT LEAST 3' DEEP.
4. BERM SURROUNDING SIDES AND BACK OF CWA SHALL HAVE A MINIMUM HEIGHT OF 1'.
5. VEHICLE TRACKING PAD SHALL BE SLOPED 2% TOWARDS CWA PIT.
6. SIGNS SHALL BE PLACED AT THE CONSTUCTION ENTRACE, AT THE CWA, AND ELSEWHERE AS NECESSARY TO CLEARLY INDICATE THE LOCATION OF THE CWA TO OPERATORS OF CONCRETE TRUCKS AND PUMP RIGS.

## CONCRETE WASHOUT — 8 / C6.1




NOTES:
1. HORIZONTAL BRACE WITH 2"x4" WOODEN FRAME OR EQUIVALENT AT TOP OF POST AS DIRECTED BY THE ENGINEER.
2. TRENCH SHALL BE A MINIMUM OF 4" WIDE BY 6" DEEP TO BURY AND ANCHOR THE GEOTEXTILE FABRIC. FOLD MATERIAL TO FIT TRENCH AND BACKFILL. NEXT, COMPACT THE TRENCH WITH EXCAVATED SOIL.
3. WOOD POSTS SHALL BE A MINIMUM OF 1-1/8" x 1-1/8" IN SIZE AND BE MADE FROM OAK OR HICKORY.
4. ATTACH THE FABRIC TO THE POST WITH WIRE STAPLES OR WOODEN LATH AND NAILS.
5. WOOD POSTS SHALL MAINTAIN A 3' MAXIMUM SPACING WHEN USING STANDARD GEOTEXTILE FABRIC. WHEN WOVEN GEOTEXTILE FABRIC IS USED, THE WOOD POSTS MAY BE SPACED UP TO 8' APART.

## SILT FENCE — 9 / C6.1




NOTES:
1. USE TYPE D INLET PROTECTION FOR ALL NON-CURB INLETS
2. USE FLEXSTORM'S CATCH-IT PRODUCT OR APPROVED EQUAL WITH RIGID FRAME.
3. DO NOT BLOCK THE TOP HALF OF THE CURB FACE OPENING

## INLET PROTECTION — 10 / C6.1




NOTES:
1. PREPARE SOIL BEFORE INSTALLING MAT, INCLUDING APPLICATION OF LIME, FERTILIZER, AND SEED. NOTE: WHEN USING CELL-O-SEED DO NOT SEED PREPARED AREA. CELL-O-SEED MUST BE INSTALLED WITH PAPER SIDE DOWN.
2. BEGIN AT THE TOP OF THE SLOPE BY ANCHORING THE BLANKET IN A 6" DEEP BY 6" WIDE TRENCH. BACKFILL AND COMPACT THE TRENCH AFTER STAPLING.
3. ROLL THE BLANKETS DOWN THE SLOPE IN THE DIRECTION OF THE WATER FLOW.
4. THE EDGES OF PARALLEL BLANKETS MUST BE STAPLED WITH APPROXIMATELY 2" OVERLAP. WHEN BLANKETS MUST BE SPLICED DOWN THE SLOPE, PLACE BLANKETS END OVER END (SHINGLE STYLE) WITH APPROXIMATELY 6" OVERLAP. STAPLE THROUGH OVERLAPPED AREA APPROXIMATELY 12" APART.
5. EROSION CONTROL MAT SHALL BE LISTED ON THE CURRENT WisDOT PRODUCT ACCEPTABILITY LIST (PAL) AS A CLASS I TYPE A EROSION MAT.

## EROSION CONTROL BLANKET — 11 / C6.1




NOTES:
1. THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHT-OF-WAYS. THIS MAY REQUIRE TOP DRESSING, REPAIR AND/OR CLEANOUT ANY MEASURES USED TO TRAP SEDIMENT.
2. WHEN NECESSARY, WHEELS SHALL BE CLEANED PRIOR TO ENTRANCE ONTO PUBLIC RIGHT-OF-WAY.

## ROCK CONSTRUCTION ENTRANCE — 12 / C6.1

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Landscape Architecture

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Project Title:
NEW BUILDING FOR:
NEW ELEMENTARY SCHOOL
LA CROSSE SCHOOL DISTRICT
807 EAST AVE S.
LA CROSSE, WI 54601

REVISIONS:

| | DATE | DESCRIPTION |
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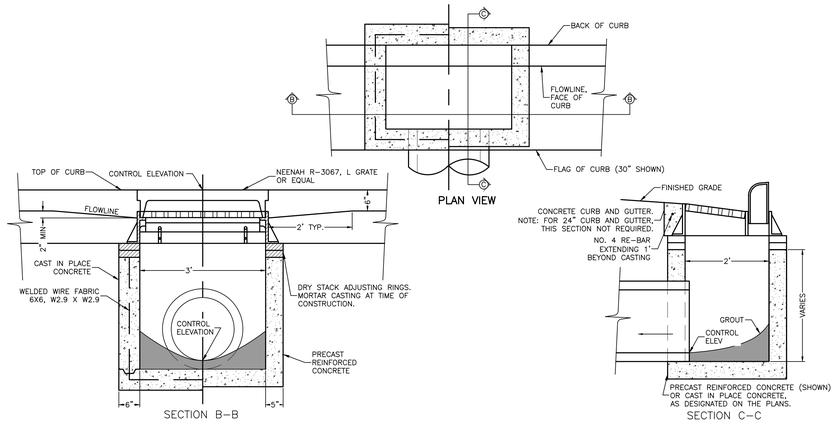
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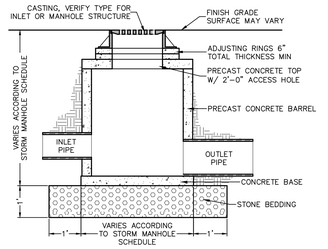
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## STORM CURB INLET

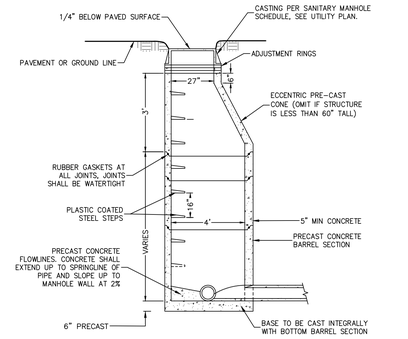
1 C6.2




NOTES:
1. MANHOLES STEPS SHALL BE INSTALLED IN ALL STORM STRUCTURES EXCEEDING A TOTAL HEIGHT OF 5.0'.

## STORM MANHOLE

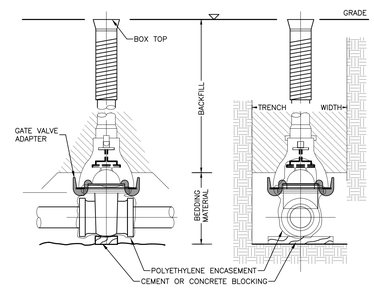
2 C6.2




NOTES:
1. MANHOLES SHALL BE PRECAST REINFORCED CONCRETE, MANUFACTURED TO MEET ASTM C-478.
2. CONNECT PIPES TO MANHOLE WITH WATER-TIGHT RUBBER BOOT CONNECTORS, DESIGNED FOR WASTEWATER APPLICATIONS AND APPLICABLE PIPE MATERIAL. INSTALL PER MANUFACTURER GUIDELINES.

## SANITARY MANHOLE

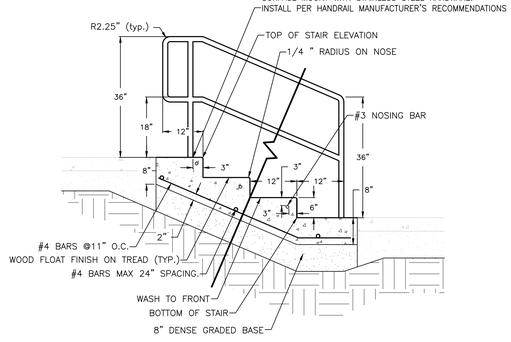
3 C6.2




NOTES:
1. VALVE SHALL BE WATEROUS 2500 SERIES, PER SPSS 601.1.4, OR CITY APPROVED EQUAL.

## WATER VALVE

4 C6.2




NOTES:
1. INSTALL POWDER COATED 1-5/8" DIAMETER STEEL HANDRAILS OR APPROVED EQUAL.
2. OWNER/ARCHITECT TO VERIFY POWDER COAT COLOR.
3. INSTALL HANDRAIL PER MANUFACTURE RECOMMENDATIONS..
4. INSTALL HANDRAILS ON BOTH SIDES OF STAIRCASE.
5. WELD JOINTS SHALL BE GROUND SMOOTH.
6. NUMBER OF TREADS VARIES, SEE LAYOUT PLAN

## CONCRETE STAIRS & RAILING

5 C6.2

| PROPOSED LAYOUT | | PROPOSED ELEVATIONS | | | ITEM ON LAYOUT | DESCRIPTION | INVERT* | MAX FLOW |
|---|---|---|---|---|---|---|---|---|
| 295 | STORMTECH SC-800 CHAMBERS | MAXIMUM ALLOWABLE GRADE (TOP OF PAVEMENT/UNPAVED) | 976.51 | PART TYPE | A | 12" TOP PRE-CORED END CAP, PART#: SC800EPE12TPC / TYP OF ALL 12" TOP CONNECTIONS | 14.40' | |
| 30 | STORMTECH SC-800 END CAPS | MINIMUM ALLOWABLE GRADE (UNPAVED WITH TRAFFIC) | 970.00 | 12" PRE-CORED END CAP | B | 24" BOTTOM PREFABRICATED EZ END CAP, PART#: SC800EZE24BPC / TYP OF ALL 24" BOTTOM CONNECTIONS AND ISOLATOR PLUS ROWS | 2.30' | |
| 5 | STONE ABOVE (in) | MINIMUM ALLOWABLE GRADE (UNPAVED FINISHED GRADE PAVEMENT) | 969.50 | 24" PREFABRICATED EZ END CAP | C | 6" SDR 35 PVC INSERTA TEE, PART#: SCI800J4SAMP (TYP 3 PLACES) | | |
| 9 | STONE BELOW (in) | MINIMUM ALLOWABLE GRADE (TOP OF RIGID CONCRETE PAVEMENT) | 969.00 | FLAMP | D | 12" x 12" TOP MANIFOLD, ADS N-12 | 14.40' | |
| 40 | STONE VOID | MINIMUM ALLOWABLE GRADE (BASE OF FLEXIBLE PAVEMENT) | 968.75 | | E | 24" x 24" BOTTOM MANIFOLD, ADS N-12 | 2.30' | |
| 24799 | INSTALLED SYSTEM VOLUME (CF) (PERIMETER STONE INCLUDED) | TOP OF STONE | 969.01 | MANIFOLD | F | 24" BOTTOM CONNECTION | 2.30' | |
| | EXCAVATION VOLUME (CF) (BASE STONE INCLUDED) | TOP OF SC-800 CHAMBER | 968.51 | MANIFOLD | G | OCS (DESIGN BY ENGINEER / PROVIDED BY OTHERS) | | 14.0 CFS OUT |
| 10632 | SYSTEM AREA (SF) | 24" BOTTOM MANIFOLD INVERT | 968.26 | MANIFOLD | H | 30" DIAMETER (24.00" SUMP MIN) | | 5.9 CFS IN |
| 570.7 | SYSTEM PERIMETER (ft) | 24" ISOLATOR ROW PLUS INVERT | 968.26 | CONNECTION | I | 30" DIAMETER (24.00" SUMP MIN) | | |
| | | 24" BOTTOM CONNECTION INVERT | 968.26 | NYLOPLAST | J | 6" ADS N-12 (DUAL WALL PERFORATED HDPE) UNDERDRAIN | | |
| | | BOTTOM OF 30" MH/CHAMBER | 968.26 | NYLOPLAST | K | 4" SEE DETAIL (TYP 2 PLACES) | | |
| | | CONNECTION INVERT | 968.26 | | | | | |
| | | BOTTOM OF STONE | 968.26 | | | | | |

*INVERT ABOVE BASE OF CHAMBER




NOTES
- SITE DESIGN ENGINEER MUST REVIEW ELEVATIONS AND IF NECESSARY ADJUST GRADING TO ENSURE THE CHAMBER COVER REQUIREMENTS ARE MET.
- **NOT FOR CONSTRUCTION**: THIS LAYOUT IS FOR DIMENSIONAL PURPOSES ONLY TO PROVE CONCEPT & THE REQUIRED STORAGE VOLUME CAN BE ACHIEVED ON SITE.

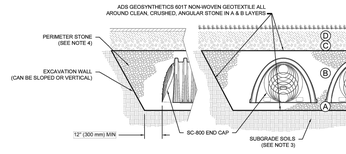
## STORMTECH CHAMBER SYSTEM

6 C6.2

### ACCEPTABLE FILL MATERIALS: STORMTECH SC-800 CHAMBER SYSTEMS

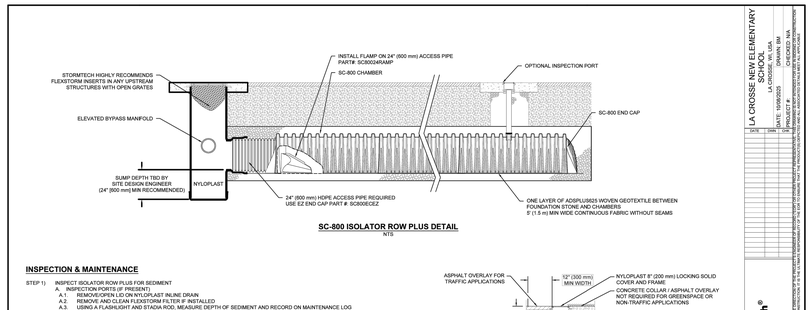
| | MATERIAL LOCATION | DESCRIPTION | AASHTO MATERIAL CLASSIFICATIONS | COMPACTION / DENSITY REQUIREMENT |
|---|---|---|---|---|
| D | **FINAL FILL:** FILL MATERIAL FOR LAYER 'D' STARTS FROM THE TOP OF THE 'C' LAYER TO THE BOTTOM OF FLEXIBLE PAVEMENT OR UNPAVED FINISHED GRADE ABOVE. NOTE THAT PAVEMENT SUBBASE MAY BE PART OF THE 'C' LAYER. | ANY SOIL/ROCK MATERIALS, NATIVE SOILS, OR PER ENGINEER'S PLANS. CHECK PLANS FOR PAVEMENT SUBGRADE REQUIREMENTS. | N/A | PREPARE PER SITE DESIGN ENGINEER'S PLANS. PAVED INSTALLATIONS MAY HAVE STRINGENT MATERIAL AND PREPARATION REQUIREMENTS. |
| C | **INITIAL FILL:** FILL MATERIAL FOR LAYER 'C' STARTS FROM THE TOP OF THE EMBEDMENT STONE (9" LAYER) TO 15" (375 mm) ABOVE THE TOP OF THE CHAMBER. NOTE THAT PAVEMENT SUBBASE MAY BE A PART OF THE 'C' LAYER. | GRANULAR WELL-GRADED SOIL/AGGREGATE MIXTURES, <35% FINES OR PROCESSED AGGREGATE.<br>OR<br>MOST PAVEMENT SUBBASE MATERIALS CAN BE USED IN LIEU OF THIS LAYER. | AASHTO M145[4]<br>A-1, A-2-4, A-3<br>OR<br>AASHTO M43[3]<br>3, 357, 4, 467, 5, 56, 57, 6, 67, 68, 7, 78, 8, 89, 9, 10 | BEGIN COMPACTIONS AFTER 12" (300 mm) OF MATERIAL OVER THE CHAMBERS IS REACHED. COMPACT ADDITIONAL LAYERS IN 6" (150 mm) MAX LIFTS TO A MIN. 95% PROCTOR DENSITY FOR WELL GRADED MATERIAL AND 95% RELATIVE DENSITY FOR PROCESSED AGGREGATE MATERIALS. ROLLER GROSS VEHICLE WEIGHT NOT TO EXCEED 12,000 lbs (53 kN). DYNAMIC FORCE NOT TO EXCEED 20,000 lbs (89 kN). |
| B | **EMBEDMENT STONE:** FILL SURROUNDING THE CHAMBERS FROM THE FOUNDATION STONE ('A' LAYER) TO THE 'C' LAYER ABOVE. | CLEAN, CRUSHED, ANGULAR STONE OR RECYCLED CONCRETE[1] | AASHTO M43[3]<br>3, 357, 4, 467, 5, 56, 57 | NO COMPACTION REQUIRED. |
| A | **FOUNDATION STONE:** FILL BELOW CHAMBERS FROM THE SUBGRADE UP TO THE FOOT (BOTTOM) OF THE CHAMBER. | CLEAN, CRUSHED, ANGULAR STONE OR RECYCLED CONCRETE[1] | AASHTO M43[3]<br>3, 357, 4, 467, 5, 56, 57 | PLATE COMPACT OR ROLL TO ACHIEVE A FLAT SURFACE.[2,3] |

PLEASE NOTE:
1. THE LISTED AASHTO DESIGNATIONS ARE FOR GRADATIONS ONLY. THE STONE MUST ALSO BE CLEAN, CRUSHED, ANGULAR. FOR EXAMPLE, A SPECIFICATION FOR #4 STONE WOULD STATE: "CLEAN, CRUSHED, ANGULAR NO. 4 (AASHTO M43) STONE".
2. STORMTECH COMPACTION REQUIREMENTS ARE MET FOR 'A' LOCATION MATERIALS WHEN PLACED AND COMPACTED IN 6" (150 mm) (MAX) LIFTS USING TWO FULL COVERAGES WITH A VIBRATORY COMPACTOR.
3. WHERE INFILTRATION SURFACES MAY BE COMPROMISED BY COMPACTION, FOR STANDARD DESIGN LOAD CONDITIONS, A FLAT SURFACE MAY BE ACHIEVED BY RAKING OR DRAGGING WITHOUT COMPACTION EQUIPMENT. FOR SPECIAL LOAD DESIGNS, CONTACT STORMTECH FOR COMPACTION REQUIREMENTS.
4. ONCE LAYER 'C' IS PLACED, ANY SOIL/MATERIAL CAN BE PLACED IN LAYER 'D' UP TO THE FINISHED GRADE. MOST PAVEMENT SUBBASE SOILS CAN BE USED TO REPLACE THE MATERIAL REQUIREMENTS OF LAYER 'C' OR 'D' AT THE SITE DESIGN ENGINEER'S DISCRETION.
5. WHERE RECYCLED CONCRETE AGGREGATE IS USED IN LAYERS 'A' OR 'B' THE MATERIAL SHOULD ALSO MEET THE ACCEPTABILITY CRITERIA OUTLINED IN TECHNICAL NOTE 6.20 "RECYCLED CONCRETE STRUCTURAL BACKFILL".




### NOTES:

1. CHAMBERS SHALL MEET THE REQUIREMENTS OF ASTM F2418, "STANDARD SPECIFICATION FOR POLYPROPYLENE (PP) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
2. SC-800 CHAMBERS SHALL BE DESIGNED IN ACCORDANCE WITH ASTM F2787 "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
3. THE SITE DESIGN ENGINEER IS RESPONSIBLE FOR ASSESSING THE BEARING RESISTANCE (ALLOWABLE BEARING CAPACITY) OF THE SUBGRADE SOILS AND THE DEPTH OF FOUNDATION STONE WITH CONSIDERATION FOR THE RANGE OF EXPECTED SOIL MOISTURE CONDITIONS. REFERENCE STORMTECH DESIGN MANUAL FOR BEARING CAPACITY GUIDANCE.
4. PERIMETER STONE MUST BE EXTENDED HORIZONTALLY TO THE EXCAVATION WALL FOR BOTH VERTICAL AND SLOPED EXCAVATION WALLS.
5. REQUIREMENTS FOR HANDLING AND INSTALLATION
   - TO MAINTAIN THE WIDTH OF CHAMBERS DURING SHIPPING AND HANDLING, CHAMBERS SHALL HAVE INTEGRAL, INTERLOCKING STACKING LUGS.
   - TO ENSURE A SECURE JOINT DURING INSTALLATION AND BACKFILL, THE HEIGHT OF THE CHAMBER JOINT SHALL NOT BE LESS THAN 2".
   - TO ENSURE THE INTEGRITY OF THE ARCH SHAPE DURING INSTALLATION, a) THE ARCH STIFFNESS CONSTANT AS DEFINED IN SECTION 6.2.8 OF ASTM F2418 SHALL BE GREATER THAN OR EQUAL TO 700 LBS/FT/%. AND b) TO RESIST CHAMBER DEFORMATION DURING INSTALLATION AT ELEVATED TEMPERATURES (ABOVE 73° F / 23° C), CHAMBERS SHALL BE PRODUCED FROM REFLECTIVE GOLD OR YELLOW COLORS.




**SC-800 ISOLATOR ROW PLUS DETAIL**
NTS

### INSPECTION & MAINTENANCE

STEP 1) INSPECT ISOLATOR ROW PLUS FOR SEDIMENT
- A. INSPECTION PORTS (IF PRESENT)
  - A.1 REMOVE/OPEN LID ON NYLOPLAST INLINE DRAIN
  - A.2 REMOVE AND CLEAN FLEXSTORM FILTER IF INSTALLED
  - A.3 USING A FLASHLIGHT AND STADIA ROD, MEASURE DEPTH OF SEDIMENT AND RECORD ON MAINTENANCE LOG
  - A.4 LOWER A CAMERA INTO ISOLATOR ROW PLUS FOR VISUAL INSPECTION OF SEDIMENT LEVELS (OPTIONAL)
  - A.5 IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.
- B. ALL ISOLATOR PLUS ROWS
  - B.1 REMOVE COVER FROM STRUCTURE AT UPSTREAM END OF ISOLATOR ROW PLUS
  - B.2 USING CAMERA OR MIRROR, SNAKE DOWN THE ISOLATOR ROW PLUS THROUGH OUTLET PIPE
    - i) MIRRORS ON POLES OR CAMERAS MAY BE USED TO AVOID A CONFINED SPACE ENTRY
    - ii) FOLLOW OSHA REGULATIONS FOR CONFINED SPACE ENTRY IF ENTERING MANHOLE
  - B.3 IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.

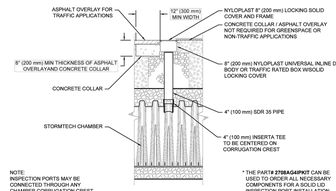
STEP 2) CLEAN OUT ISOLATOR ROW PLUS USING THE JETVAC PROCESS
- A. A FIXED CULVERT CLEANING NOZZLE WITH REAR FACING SPREAD OF 45° (1.1 m) OR MORE IS PREFERRED
- B. APPLY MULTIPLE PASSES OF JETVAC UNTIL BACKFLUSH WATER IS CLEAN
- C. VACUUM STRUCTURE SUMP AS REQUIRED

STEP 3) REPLACE ALL COVERS, GRATES, FILTERS, AND LIDS; RECORD OBSERVATIONS AND ACTIONS.

STEP 4) INSPECT AND CLEAN BASINS AND MANHOLES UPSTREAM OF THE STORMTECH SYSTEM.

### NOTES

1. INSPECT EVERY 6 MONTHS DURING THE FIRST YEAR OF OPERATION. ADJUST THE INSPECTION INTERVAL BASED ON PREVIOUS OBSERVATIONS OF SEDIMENT ACCUMULATION AND HIGH WATER ELEVATIONS.
2. CONDUCT JETTING AND VACTORING ANNUALLY OR WHEN INSPECTION SHOWS THAT MAINTENANCE IS NECESSARY.



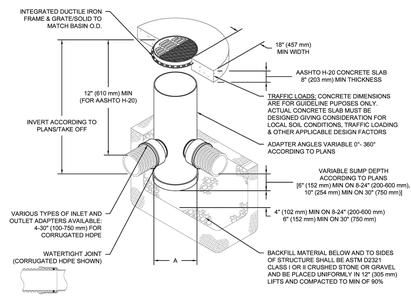

NOTE:
INSPECTION PORTS MAY BE CONNECTED THROUGH ANY CHAMBER CORRUGATION CREST.

* THE PART# **2708AG4PKIT** CAN BE USED TO ORDER ALL NECESSARY COMPONENTS FOR A SOLID LID INSPECTION PORT INSTALLATION

**4" (100 mm) PVC INSPECTION PORT DETAIL (SC SERIES CHAMBER)**
NTS

## STORMTECH CHAMBER SYSTEM

7 C6.2




### NOTES

1. 8-30" (200-750 mm) GRATES/SOLID COVERS SHALL BE DUCTILE IRON PER ASTM A536 GRADE 70-50-05
2. 12-30" (300-750 mm) FRAMES SHALL BE DUCTILE IRON PER ASTM A536 GRADE 70-50-05
3. DRAIN BASIN TO BE CUSTOM MANUFACTURED ACCORDING TO PLAN DETAILS
4. DRAINAGE CONNECTION STUB JOINT TIGHTNESS SHALL CONFORM TO ASTM D3212 FOR CORRUGATED HDPE (ADS & HANCOR DUAL WALL) & SDR 35 PVC
5. FOR COMPLETE DESIGN AND PRODUCT INFORMATION: **WWW.NYLOPLAST-US.COM**
6. TO ORDER CALL: **800-821-6710**

| A | PART # | GRATE/SOLID COVER OPTIONS | | |
|---|---|---|---|---|
| 8" (200 mm) | 2808AG | PEDESTRIAN LIGHT DUTY | STANDARD LIGHT DUTY | SOLID LIGHT DUTY |
| 10" (250 mm) | 2810AG | PEDESTRIAN LIGHT DUTY | STANDARD LIGHT DUTY | SOLID LIGHT DUTY |
| 12" (300 mm) | 2812AG | PEDESTRIAN AASHTO H-10 | STANDARD AASHTO H-20 | SOLID AASHTO H-20 |
| 15" (375 mm) | 2815AG | PEDESTRIAN AASHTO H-10 | STANDARD AASHTO H-20 | SOLID AASHTO H-20 |
| 18" (450 mm) | 2818AG | PEDESTRIAN AASHTO H-10 | STANDARD AASHTO H-20 | SOLID AASHTO H-20 |
| 24" (600 mm) | 2824AG | PEDESTRIAN AASHTO H-10 | STANDARD AASHTO H-20 | SOLID AASHTO H-20 |
| 30" (750 mm) | 2830AG | PEDESTRIAN AASHTO H-20 | STANDARD AASHTO H-20 | SOLID AASHTO H-20 |

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Project Title:
NEW BUILDING FOR:
NEW ELEMENTARY SCHOOL
LA CROSSE SCHOOL DISTRICT
807 EAST AVE S.
LA CROSSE, WI 54601

REVISIONS:

| | DATE | DESCRIPTION |
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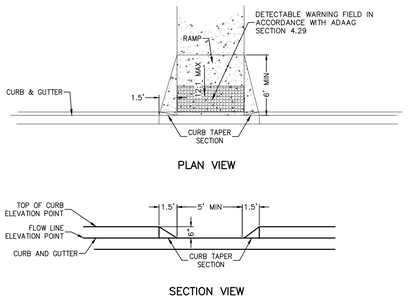
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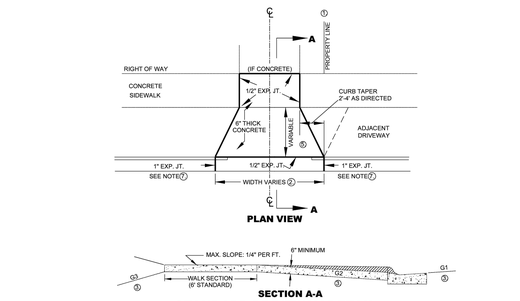
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## ADA CURB RAMP 'B'

1 C6.3

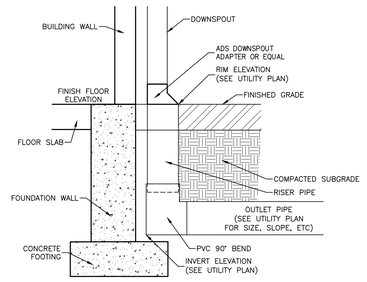



## STANDARD DRIVEWAY DETAIL

1. DRIVE SECTION SHALL NOT OVERLAP PROPERTY LINE EXTENDED, EXCEPT WHERE PERMITTED BY THE ENGINEER, OR WHEN A JOINT DRIVEWAY AGREEMENT IS EXECUTED BY OWNERS OF ADJACENT PROPERTIES.
2. MAXIMUM DRIVEWAY WIDTH AT THE CURB AND SIDEWALK IS SET FORTH IN CITY ORDINANCE SEC. 40-4. FINAL DIMENSIONS SHALL BE APPROVED BY THE ENGINEERING DEPARTMENT.
3. THE BREAKOVER ANGLE (CAUSE OF CARS BOTTOMING) BECOMES CRITICAL WHEN THE ALGEBRAIC DIFFERENCE OF GRADES (G1, G2, & G3) EXCEEDS 11%.
4. A REINFORCED DRIVE SECTION IS REQUIRED FOR CURB & GUTTER IN AREAS ZONED INDUSTRIAL OR COMMERCIAL.
5. BACK OF CURB TO FRONT OF CONCRETE SIDEWALK MUST BE CONCRETE, BRICK OR ASPHALT.
6. MECHANICAL COMPACTION OF SUBSOIL IN LAYERS LESS THAN 12" TO ACHIEVE MINIMUM COMPACTION OF 95% OF MAXIMUM DENSITY FROM MODIFIED PROCTOR IS REQUIRED. (INCLUDING STREET SIDE AFTER FORMS ARE REMOVED)
7. EXPANSION JOINT IS REQUIRED AT BOTH ENDS OF DRIVEWAY WHEN ONLY DRIVEWAY IS INSTALLED OR REPLACED. WHEN ENTIRE BLOCK OF CURB & GUTTER IS INSTALLED THE EXPANSION JOINT AT DRIVEWAY ENDS MAY BE OMITTED.
8. TURNING OF 2' DIAMETER DRIVEWAY RETURNS IN LIEU OF DIMINISHING CURB AS SHOWN IS PERMITTED IF DESIRED BY PROPERTY OWNER.
9. INSTALLATION OF A DRIVEWAY BY REMOVING EXISTING CURB ONLY IS NOT ALLOWED. ENTIRE EXISTING C&G MUST BE REMOVED FOR NEW DRIVEWAYS. SAWING & REMOVAL OF A MINIMUM 2' WIDTH OF ASPHALT STREET PAVEMENT TO INSTALL FRONT FORMS IS REQUIRED.

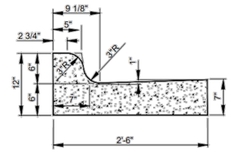
## DRIVE APRON

2 C6.3




## DOWNSPOUT CONNECTION

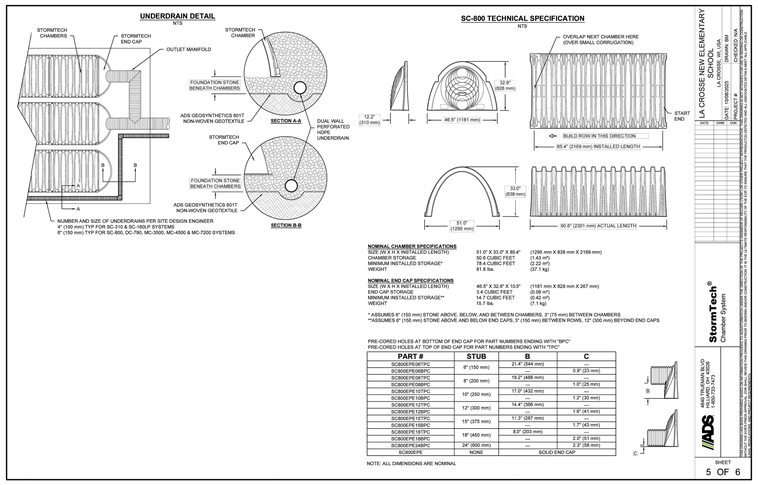
3 C6.3




## STANDARD CURB & GUTTER SECTION

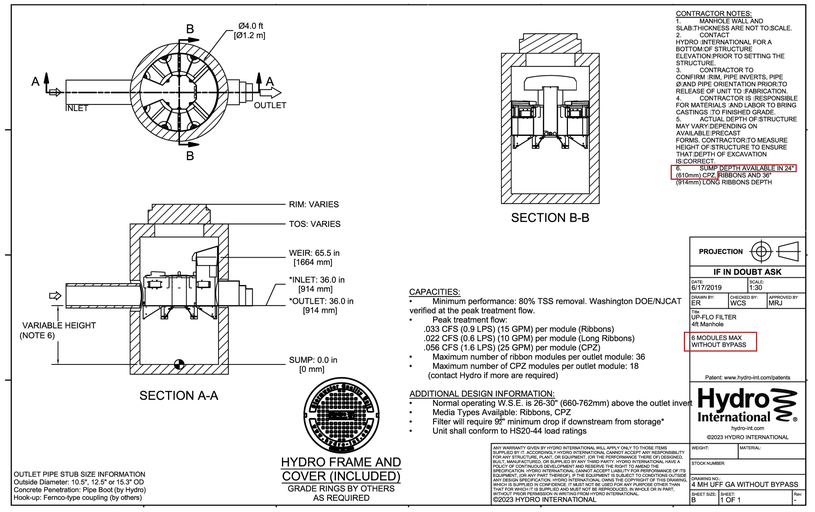
## CITY OF LA CROSSE STANDARD CURB

4 C6.3



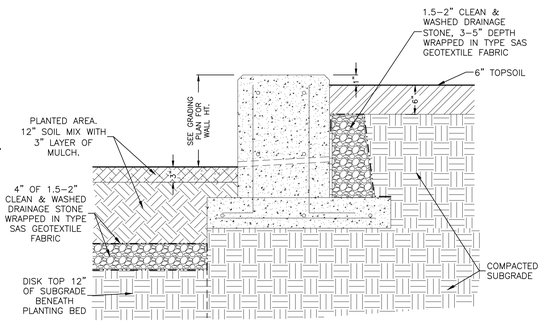

## STORMTECH CHAMBER SYSTEM

5 C6.3




## UP-FLO FILTER

6 C6.3




## PLANTING BED BEHIND WALLS

7 C6.3

BRAY ARCHITECTS
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T: 414.226.0200
Sheboygan • Davenport • Moline
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Point of Beginning
Civil Engineering
Land Surveying
Landscape Architecture
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715.344.9922(Fx)

Project Title:
NEW BUILDING FOR:
NEW ELEMENTARY SCHOOL
LA CROSSE SCHOOL DISTRICT
807 EAST AVE S.
LA CROSSE, WI 54601

REVISIONS:

| | DATE | DESCRIPTION |
|---|---|---|

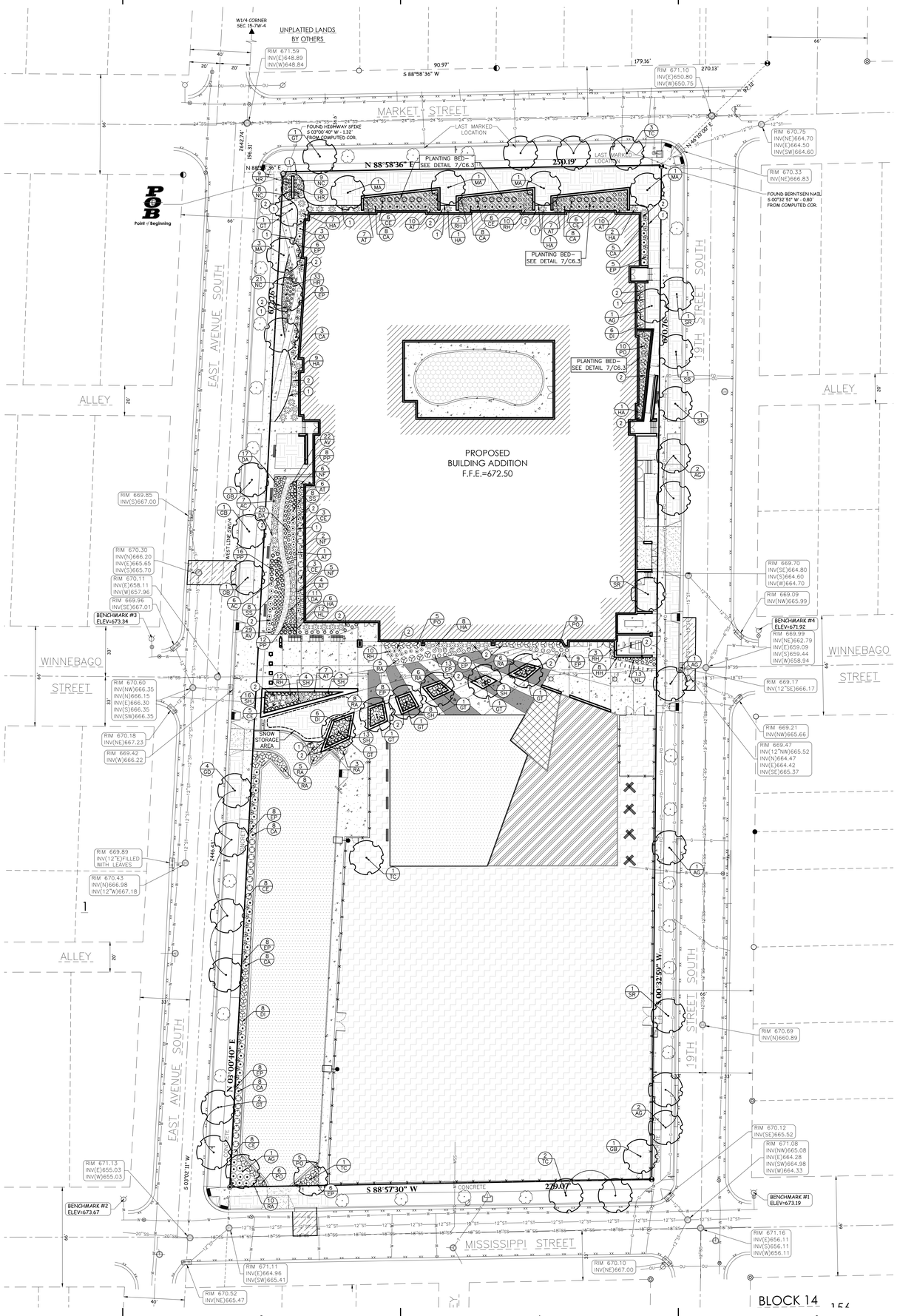
Project Number: 3752

Issued For: CONTRACT DOCUMENTS 10/28/2025

Sheet Title: Details

Sheet Number: C6.3

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## GENERAL NOTES:

1. CONTACT DIGGER'S HOTLINE 5 WORKING DAYS PRIOR TO THE START OF DEMOLITION/CONSTRUCTION.
2. 6" OF TOPSOIL SHALL BE PROVIDED IN ALL GENERAL LANDSCAPE AREAS. LANDSCAPE CONTRACTOR SHALL VERIFY THAT SPECIFIED PLANTING SOIL DEPTH IS PRESENT PRIOR TO PLANTING.
3. SEED/FERTILIZE/CRIMP HAY MULCH ALL GENERAL LANDSCAPE AREAS DISTURBED DURING CONSTRUCTION EXCEPT ARES LABELED AS SYNTHETIC TURF.
4. ALL PLANT MATERIALS LISTED SHALL MEET THE STANDARDS OF THE AMERICAN NURSERY & LANDSCAPE ASSOCIATION FOR THE SIZES GIVEN.
5. ALL TREES SHALL BE STAKED WITH A MINIMUM OF THREE STAKES.
6. ALL TREES IN THE TURF AREA SHALL HAVE A 5' DIAMETER CIRCLE OF 4" DEPTH SHREDDED HARDWOOD BARK MULCH.
7. CURV–RITE LANDSCAPING EDGING (SERIES 3000 3/16" X 4", WITH MILL FINISH) OR APPROVED EQUAL SHALL BE PLACED AROUND ALL LANDSCAPE BEDS.
8. 3" DEPTH OF SHREDDED HARDWOOD BARK MULCH SHALL BE PLACED IN ALL LANDSCAPE PLANTING BEDS AS NOTED. PROVIDE A SAMPLE TO THE OWNER FOR REVIEW AND APPROVAL PRIOR TO INSTALLATION.
9. 20 YEAR WEED BARRIER FILTER FABRIC SHALL BE PLACED BENEATH ALL LANDSCAPE HARDWOOD BARK MULCH.
10. COORDINATE ALL LANDSCAPE WORK WITH GAS, ELECTRIC, (INCLUDING MAIN SERVICE, SITE LIGHTING, CONDUITS AND SIGNAGE) CABLE AND TELEPHONE CONSTRUCTION AND RESPECTIVE TRADES FOR THE INSTALLATION OF SAID UTILITIES.

## PLANTING SCHEDULE:

| TREES SYMBOLS | BOTANICAL NAME | COMMON NAME | INSTALLATION SIZE | SIZE AT MATURITY | QUANTITY |
|---|---|---|---|---|---|
| AG | AMELANCHIER X GRADIFLORA 'AUTUMN BRILLIANCE' | APPLE SERVICEBERRY | 2" CAL. | 25'TX20'W | 8 |
| GB | GINKGO BILOBA 'PRINCETON SENTRY' (MALE) | GINKGO | 2" CAL. | 50'TX25'W | 4 |
| GD | GYMNOCLADUS DIOICUS 'STATELY MANOR' | KENTUCKY COFFEE TREE | 2" CAL. | 60'TX40'W | 4 |
| GT | GLEDITSIA TRIACANTHOS 'SKYCOLE' | SKYLINE HONEYLOCUST | 2" CAL. | 45'TX35'W | 10 |
| MA | MAACKIA AMURENSIS | AMUR MAACKIA | 2" CAL. | 25'TX20'W | 7 |
| SR | SYRINGA RETICULATA 'IVORY SILK' | IVORY SILK JAPANESE TREE LILAC | 2" CAL. (SINGLE STEM) | 25'TX25'W | 5 |
| TC | TILIA CORDATA | LITTLE–LEAF LINDEN | 2" CAL. | 60'TX40'W | 7 |

| SHRUBS SYMBOLS | BOTANICAL NAME | COMMON NAME | INSTALLATION SIZE | SIZE AT MATURITY | QUANTITY |
|---|---|---|---|---|---|
| AT | ASCLEPIAS TUBEROSA | BUTTERFLY WEED | 5 GAL | 2'T–1'W | 52 |
| CE | CEANOTHUS AMERICANUS | NEW JERSEY TEA | 5 GAL | 4'T–4'W | 45 |
| DI | DIERVILLA IONICERA | DWARF BUSH HONEYSUCKLE | 5 GAL | 3'T–3'W | 20 |
| HA | HYDRANGEA ARBORESCENS 'ANNABELLE' | ANNABELLE HYDRANGEA | 5 GAL | 5'T–5'W | 35 |
| NF | NEPETA FAASSENII 'CAT'S MEOW' | CAT'S MEOW CATMINT | 5 GAL | 20"T–3'W | 13 |
| PO | PHYSOCARPUS OPULIFOLIUS 'SMPOTW' | TINY WINE NINEBARK | 5 GAL | 3'T–4'W | 35 |
| RA | RHUS AROMATICA 'GRO–LOW' | FRAGRANT SUMAC | 5 GAL | 2'T–6'W | 51 |

| PERENNIALS AND ORNAMENTAL GRASSES SYMBOLS | BOTANICAL NAME | COMMON NAME | INSTALLATION SIZE | SIZE AT MATURITY | QUANTITY |
|---|---|---|---|---|---|
| CA | CALAMAGROSTIS X ACUTIFLORA 'KARL FOERSTER' | KARL FOERSTER FEATHER REED GRASS | 24" | 5'–7'T | 59 |
| EP | ECHINACEA PURPUREA | PURPLE CONEFLOWER | 12" | 36"T–18"W | 85 |
| HR | HEMEROCALLIS X 'ROSY RETURNS' | ROSY RETURNS DAYLILY | 12" | 18"T–24"W | 50 |
| HL | HEMEROCALLIS X 'LITTLE GRAPETTE' | LITTLE GRAPETTE DWARF DAYLILY | 12" | 2'T–2'W | 25 |
| NC | NEPETA FAASSENII 'CAT'S PAJAMAS' | CAT'S PAJAMAS CATMINT | 12" | 14"T–20"W | 38 |
| RH | RUDBECKIA HIRTA | BLACK–EYED SUSAN | 18" | 24"T–18"W | 44 |
| SH | SPOROBOLUS HETEROLEPIS | PRAIRIE DROPSEED | 12" | 24"T–24"W | 47 |

| DRY BASIN GARDEN PLANTINGS SYMBOLS | BOTANICAL NAME | COMMON NAME | INSTALLATION SIZE | SIZE AT MATURITY | QUANTITY |
|---|---|---|---|---|---|
| AC | AQUILEGIA CANADENSIS | COLUMBINE | 1 GAL | 3'T–2'W | 13 |
| AV | ANEMONE VIRGINIANA | THIMBLEWOOD | 1 GAL | 2'T–1'W | 51 |
| DA | DRYMOCALLIS ARGUTA | PRAIRIE CINQUEFOIL | 1 GAL | 3'T–2'W | 48 |
| PP | PHLOX PILOSA | PRAIRIE PHLOX | 1 GAL | 1'T–1'W | 36 |
| SS | SCHIZACHYRIUM SCOPARIUM | LITTLE BLUESTEM | 1 GAL | 3'T–2'W | 16 |

| VINE * SYMBOLS | BOTANICAL NAME | COMMON NAME | INSTALLATION SIZE | SIZE AT MATURITY | QUANTITY |
|---|---|---|---|---|---|
| HH | HEDERA HELIX | ENGLISH IVY | 1 GAL | 4'–10' SPREAD | 8 |

* WALL ATTACHMENT SYSTEM – ESPALAR SURFACE MOUNTED HEAVY DUTY UNIVERSAL ESPALIER/TRELLIS KIT FOR CLIMBING PLANTS OR APPROVED EQUAL

## KEYNOTES:

1. LANDSCAPE EDGING – SEE GENERAL NOTES
2. LANDSCAPE HARDWOOD MULCH – SEE GENERAL NOTES

## HATCH LEGEND:

- 3'–0" WIDE DRY RIVER ROCK BED 1.5" – 2" MISSISSIPPI RIVER STONE
- INSTALL SOD WITHIN THIS LOCATION– SEE SPECIFICATIONS
- INSTALL TURF SEED WITHIN THIS LOCATION AS WELL AS ANY OTHER GREEN SPACE AREAS DISTURBED DURING CONSTRUCTION – SEE SPECIFICATIONS
- LANDSCAPE PLANTING BED SECTION DETAILS — 7 / C6.3

## LANDSCAPE REQUIREMENTS:

REQUIREMENT: ONE TREE PLACED IN THE BOULEVARD PER 40 LINEAR FEET OF LOT FRONTAGE.

TOTAL LOT FRONTAGE: 1871 LF = 46.7 | 47 TOTAL TREES REQUIRED

PROPOSED: 47 (23 EXISTING TRESS AND 24 PROPOSED)

REQUIREMENT: NOT LESS THAN TWO TREES AND EIGHT SHRUBS PER 600 SQUARE FEET OF LANDSCAPED AREA.

TOTAL LANDSCAPE AREA: 18,545 SF / 600 = 31 TOTAL TREES AND 248 SHRUBS REQUIRED

PROPOSED: 251 SHRUBS AND 21 TREES

## BENCHMARK:

ELEVATIONS ARE REFERENCED TO NAVD 88 DATUM.

**BENCHMARK #1**
NORTHWEST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHEAST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND 19TH STREET SOUTH. ELEVATION = 673.19

**BENCHMARK #2**
NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND EAST AVENUE SOUTH. ELEVATION = 673.67

**BENCHMARK #3**
NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST CORNER OF THE INTERSECTION OF EAST AVENUE SOUTH AND WINNEBAGO STREET. ELEVATION = 673.34

**BENCHMARK #4**
BURY BOLT ON HYDRANT, LOCATED AT THE NORTHEAST CORNER OF THE INTERSECTION OF 19TH STREET SOUTH AND WINNEBAGO STREET. ELEVATION = 671.92

## UTILITY DISCLAIMER:

THE LOCATIONS, SIZES, AND TYPES OF UNDERGROUND PUBLIC AND PRIVATE UTILITIES OR SUBSTRUCTURES SHOW HEREON WERE OBTAINED FROM VISUAL INSPECTION, FIELD MEASUREMENTS, AND/OR AS–BUILT PLANS. SANITARY SEWER AND STORM SEWER PIPE SIZES, INVERTS, DIRECTION, AND LOCATIONS BETWEEN MANHOLES ARE SUPPLEMENTED BY AS–BUILT PLANS AND/OR ESTIMATED BASED ON FIELD OBSERVATIONS. PRIOR TO CONSTRUCTION IN THE VICINITY OF ANY UTILITIES SHOWN HEREON, IT IS RECOMMENDED THAT THE LOCATIONS, DEPTHS, AND SIZES BE FIELD VERIFIED. THE LOCATIONS SHOWN HEREON ARE ONLY APPROXIMATE, WITH POSSIBILITY THAT ADDITIONAL UTILITY LINES NOT DISCOVERED, OR MARKED, DURING THE SEARCH OF RECORDS AND THE FIELD SURVEY MAY EXIST. ANY CONTRACTOR USING THE INFORMATION SHOWN HEREON IS HEREBY FOREWARNED THAT ANY EXCAVATION UPON THIS SITE MAY RESULT IN THE DISCOVERY OF ADDITIONAL UNDERGROUND UTILITIES NOT SHOWN HEREON. IN GENERAL, UNDERGROUND UTILITY LOCATIONS ARE SHOWN FROM UTILITY MARKINGS, BY OTHERS, AND/OR AS–BUILT PLANS, PROVIDED BY OTHERS. POINT OF BEGINNING MAKES NO WARRANTY OF ANY KIND, EXPRESS OR IMPLIED, WITH RESPECT TO THE EXISTING UTILITIES SHOWN HEREON, AND BELIEVES THAT THE INFORMATION CONTAINED HEREIN IS RELIABLE AND GENERALLY ACCURATE FOR THE PURPOSE INTENDED.

0' 30' 60'
1" = 30'

N


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Point of Beginning
Civil Engineering
Land Surveying
Landscape Architecture

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Stevens Point, WI 54481
715.344.9999(Ph)
715.344.9922(Fx)


Project Title:
NEW BUILDING FOR:
NEW ELEMENTARY SCHOOL
LA CROSSE SCHOOL DISTRICT
807 EAST AVE S.
LA CROSSE, WI 54601

REVISIONS:

| | DATE | DESCRIPTION |
|---|---|---|

Project Number: 3752

Issued For: CONTRACT DOCUMENTS 10/28/2025

Sheet Title: Landscape Plan

Sheet Number: L1.0

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# Stormwater Management Permit Application

City of La Crosse Engineering Department

400 La Crosse Street ▫ Engineering Department ▫ La Crossse, WI 54601

## Section 1 ▫ Property Information

Project Name: NEW ELEMENTARY SCHOOL

Property Address: 807 EAST AVENUE SOUTH (*Street*) | 807 (*Lot Number(s)*) | 17-30206-10, 17-502 (*Parcel Number*)

LA CROSSE (*City*) | WI (*State*) | 54601 (*ZIP Code*)

(*Plat or CSM*)

## Section 2 ▫ Landowner Information

Full Name: SCHOOL DISTRICT OF LA CROSSE - JOE LEDVINA (*Last* | *First* | *M.I.*)

Mailing Address: 807 EAST AVENUE SOUTH (*Street* | *Apartment/Unit #*)

LA CROSSE (*City*) | WI (*State*) | 54601 (*ZIP Code*)

Contact Phone: 608-789-7627 E-Mail: jledvina@lacrosse.org

## Section 3 ▫ Applicant Information

☑ **Same as Landowner** *(Check if YES, and continue with Section 4)*

Full Name: (*Last* | *First* | *M.I.*)

Mailing Address: (*Street* | *Apartment/Unit #*)

(*City*) | (*State*) | (*ZIP Code*)

Contact Phone: E-Mail:

## Section 4 ▫ Site Information

| | | |
|---|---|---|
| **Total Site Area** | 187,985 | $ft^2$ |
| **Existing Impervious Area** *(Before Project)* | 88,891 | $ft^2$ |
| **New Impervious Area** *(Impervious area added outside any existing impervious area)* | 65,187 | $ft^2$ |
| **Redeveloped Impervious Area** *(Impervious area redeveloped inside original impervious area foot print)* | 38,078 | $ft^2$ |
| **Removed Impervious Area** *(From inside original impervious area footprint)* | 36,369 | $ft^2$ |
| **Net Impervious Area** *(After Project)* | 66,896 | $ft^2$ |

**Work to be performed by** *(if known)*: ☐ **Same as Applicant** *(Check if YES)* ☐ **Same as Landowner** *(Check if YES)*

Construction Contact: TBD

Contact Phone: _____ E-Mail: _____

**Stormwater Management Report/Plan to be attached.**
***Please note application cannot be processed without report/plan****

## Section 5 ▫ Fee

Permit Fees per Municipal Code of Ordinances Appendix C Fee Schedule

**FEES RECEIVED**
*Office Use Only*
Date _____
Amt _____
By _____

## Section 6 ▫ Stormwater Management Requirements

- ☐ TSS Reduction: ☐ New Development (80%) ☒ Redevelopment (40%)
- ☐ Oil & Grease Removal
- ☒ Runoff Rate Control/Detention
- ☒ Infiltration
- ☐ Groundwater Recharge
- ☐ Thermal Control
- ☒ Maintenance Agreement Executed

Construction Start Date 02/01/2026 Estimated Project Completion Date 09/01/2027

## Section 7 ▫ Applicant Signature

***I have reviewed and understand Chapter 105 of the La Crosse Ordinances regarding erosion control, and I shall implement the control plan for this project as approved by the city.***

***I further, in accordance with Chapter 105, grant the right-of-entry onto this property, as described above, to the designated personnel of the City of La Crosse for the purpose of inspecting and monitoring for compliance with the aforesaid ordinance.***

Applicant Signature _____ Date of Application 10/22/25

**Applicant other than landowner requires a notarized statement authorizing the applicant to act as the landowner's agent—must be attached*

# Erosion Control & Storm Water Management Maintenance/Operation Plan

**For:**

**La Crosse New Elementary School**

**PREPARED BY:**

Point of Beginning

4941 KIRSCHLING COURT
STEVENS POINT, WI 54481
(715) 344-9999 ■ (715) 344-9922

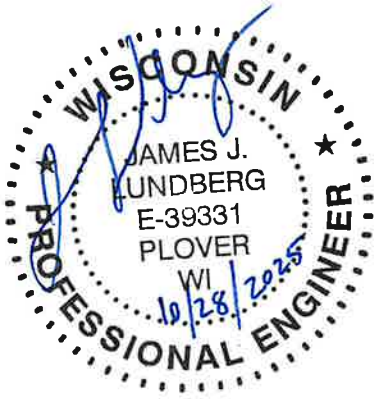
**Project Number: 25.0034**

**Located in:**

## City of La Crosse
## La Crosse County, Wisconsin

**Dated:**

**October 28, 2025**



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La Crosse New Elementary School
City of La Crosse, Wisconsin


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**Appendices**


- **A.** Proposed Layout Plan
- **B.** Geotechnical Data
- **C.** Existing Drainage Map and Calculations
- **D.** Proposed Drainage Map and Calculations
- **E.** Proposed Erosion Control Plan
- **F.** State of Wisconsin Construction Site Inspection Report and DNR Notice of Termination

# Erosion Control & Storm Water Management
# Maintenance/Operation Plan
# La Crosse New Elementary School
# La Crosse, WI

## 1.0 BACKGROUND & GENERAL INFORMATION

### 1.1 Introduction and Project Location

Point of Beginning, Inc. has been retained by the School District of La Crosse to perform storm water management calculations and prepare a storm water management plan per NR216.47 and NR151, for the proposed La Crosse New Elementary School project. The project site is located at 807 East Avenue South, La Crosse, WI 54601

### 1.2 Project Contacts

| Designation | Name | Address | Phone Number |
|---|---|---|---|
| Landowner & Developer<br>*School District of La Crosse* | Joe Ledvina | 807 East Avenue South<br>La Crosse, WI 54601 | (608) 789-7627 |
| Stormwater Engineer<br>*Point of Beginning, Inc.* | Jim Lundberg, P.E. | 4941 Kirschling Ct<br>Stevens Point, WI 54481 | (715) 344-9999 |
| Person Responsible for BMP Installation<br>TBD | TBD | TBD | TBD |

### 1.3 Project Description

The proposed project consists of demolition of the existing Hogan Administration Center building including all paved surfacing, infield baseball mix, gravel areas, woodchip playground areas, and landscape areas. A new elementary school building is to be constructed including paved parking lot, concrete sidewalks, hard surface playground areas, rubber surface play areas, woodchip play areas, and landscape/turf grass areas. Additionally, the site will be graded to ensure proper drainage, and one new underground stormwater wet pond system will be installed. (See Layout Plan in **Appendix A**).

### 1.4 Project Requirements

The project area includes approximately 4.5 acres that will be disturbed. Since the disturbed area exceeds one acre, a Wisconsin Department of Natural Resources Notice of Intent application/permit (NOI-WPDES per WDNR) is required.

The storm water management plan for this project has been developed in accordance with NOI-WPDES, NR216.47, NR151.121, and City of La Crosse Chapter 105 requirements for redevelopment sites.

### 1.5 General Project Data

<u>Soils</u>
The Natural Resources Conservation Service (NRCS) has no data for existing subgrade soils. A geotechnical exploration was performed to verify the soil conditions and explore the possibility of stormwater infiltration. The geotechnical data is attached in **Appendix B**.

<u>Groundwater</u>
Groundwater was estimated to be deeper than the termination depths (16-26') at the test boring locations at the time the borings were conducted.

Wetlands
Per the DNR Surface Water Data Viewer, there are no wetlands or wetland indicators located on the project site or within 75' of the project limits.

Precipitation
The following precipitation rates have been utilized for storm water calculations:

$P_{2,24}$ = 3.01"
$P_{10,24}$ = 4.46"

## 2.0 EXISTING DRAINAGE CONDITIONS

### 2.1 Existing Drainage Areas

The existing site consists of one sub-basin (E1). Sub-basin E1 consists of existing asphalt, concrete sidewalk, baseball infield mix, gravel, woodchip play area, and landscape/grass areas. Runoff from E1 drains offsite to via overland flow and into existing municipal storm sewer system. An existing drainage map can be found in **Appendix C**.

### 2.2 Existing Drainage Calculation Summary

Existing drainage calculations utilize TR-55 methodology and results for a 1, 2, 10, 25 and 100-year design storm are included. Existing drainage calculations are provided in **Appendix C**.

### 2.3 Existing Off-Site Drainage

Existing off-site storm water runoff draining onto the project site has been taken into consideration for the existing drainage evaluation.

## 3.0 PROPOSED DRAINAGE CONDITIONS

### 3.1 Proposed Drainage Areas

The proposed site is divided into seven sub-basins (D1-D7). Sub-basin D1 consists of building rooftop, concrete sidewalk, rubber play surface, hard surface play area, wood chip play area, and grass area. Runoff from D1 is collecting into storm pipe and conveyed to the proposed Storm Tech system. Sub-basin D2 consists of paved parking area, concrete sidewalk, and grass area. Runoff from D2 flows directly offsite to the west and into the existing municipal storm sewer system within the right of way of East Avenue. Sub-basin D3 consists of grass areas. Runoff from D3 flows directly offsite to the north and into the existing municipal storm sewer system within the right of way of 19th Street South. Sub-basin D4 consists of paved parking area, concrete sidewalk, soft surface play area, and grass area. Runoff from D4 flows directly offsite to the east and into the existing municipal storm sewer system within the right of way of 19th Street South. Sub-basin D5 consists of paved parking area, concrete sidewalk, and grass areas. Runoff from D5 flows directly offsite to the south and into the existing municipal storm sewer system within the right of way of Mississippi Street. Sub-basin D6 consists of concrete sidewalk, paved parking area, and grass area. Runoff from D6 is collected into storm pipe and conveyed to the proposed Storm Tech system. Sub-basin D7 consists of concrete sidewalk, paved parking area, and grass area. Runoff from D7 is collecting into storm pipe and conveyed to the proposed Storm Tech system. A proposed drainage area map is provided in **Appendix D.**

### 3.2 Post-Development Runoff Summary

Proposed drainage calculations utilize TR-55 methodology and results for a 2 and 10-year design storm are included. A proposed drainage area map and calculations are provided in **Appendix D.**

### 3.3 Proposed Detention Areas

One stormwater management practice is proposed.

The StormTech Chamber System (1P) collects storm water runoff from the developed areas of D1, D7, & D6 and is located under the proposed parking lot on the southwest side of the site. This system provides rate control and treatment for the site.

## 4.0 POST-DEVELOPMENT PERFORMANCE STANDARDS

### 4.1 Total Suspended Solids

According to NR151.122, BMPs shall be designed in accordance with Table 1, or to the maximum extent practicable. For redevelopment projects Table 1 indicates that the total suspended solids shall be reduced by 40 percent for parking and roads, based on an average annual rainfall, as compared to no runoff management controls.

Per City of La Crosse Section 105-61(b)(4)(a)(1), BMPs shall be designed in accordance with Table 1, or to the maximum extent practicable as provided in subsection (b)(4)a.2. The design shall be based on an average annual rainfall, as compared to no runoff management controls. For redevelopment projects Table 1 indicates that the total suspended solids shall be reduced by 40 percent for parking areas and roads.

The total suspended solids removal has been modeled in WinSLAMM version 10.5.0. According to the WinSLAMM modeling, the expected TSS removal is **71.18%**; therefore, the proposed design meets NR151.122 and City of La Crosse total suspended solids removal requirements. See **Appendix D** for the WinSLAMM modeling inputs and outputs.

### 4.2 Infiltration

According to NR151.124(1)(b), redevelopment projects are exempt from infiltration performance standards.

Per City of La Crosse Section 105-61(b)(4)(d)(3)(b)(iii), redevelopment post-construction sites are exempt from infiltration performance standards.

### 4.3 Peak Discharge

According to NR151.123(2)(b), redevelopment projects are exempt from peak discharge rate performance standards.

Per City of La Crosse Section 105-61(b)(4)(b), BMPs shall be employed to reduce the two-year, 24-hour; and the ten-year, 24-hour post construction peak runoff discharge rates to the two-year, 24-hour; and the ten-year, 24-hour pre-development peak runoff discharge rates respectively.

In accordance with good engineering practice and Point of Beginning policy, the project's pre-development and post-development peak runoff discharge rates have been calculated.
The pre-development and post-development peak rates of the discharge leaving the site are summarized in the table on the following page. See **Appendix D** for HydroCAD modeling routing diagrams, summaries, and node listings.

| | Pre-Development | Post-Development |
|---|---|---|
| | Total (1L) | Total (1L) |
| 2-year 24-hour Peak Flow | 1.53 cfs | 0.91 cfs |
| 10-year 24-hour Peak Flow | 3.65 cfs | 1.95 cfs |

### 4.4 Protective Area

No neighboring waterways or wetlands within 75' of the project site. Therefore, protective area requirements are not applicable for this project.

### 4.6 Summary

The modeling of this site shows that the performance standards set by the Department of Natural Resources and the City of La Crosse 105-61 can be met with the proposed design.

The Storm Water Management Plan shows basic compliance with accepted engineering practice in hydrologic planning and design. The resulting development will function as a positive addition to the community while sustaining environmental benefits in storm water management and quality.

## 5.0 CONSTRUCTION SITE PERFORMANCE STANDARDS

### 5.1 Erosion Control

The purpose of this control plan is to provide guidelines that comply with the state and local requirements, as well as to make recommendations regarding erosion control and storm water management. The construction of this development is a critical phase in terms of storm water management and runoff control. Construction site erosion control will help minimize the impact of development, enhance and protect the local environment, and protect the surrounding project area by applying best management practices for erosion control at construction sites. This work shall be planned and executed in accordance with the Wisconsin Department of Natural Resources Storm Water Management Technical Standards and/or accepted local engineering practice. The owner/developer will be responsible for erosion control during the process of construction. Silt fence, site vegetation, rock construction entrance, inlet protection, temporary ditch checks and erosion mat will be utilized to keep sediment from leaving the construction site. **See Appendix E.**

### 5.2 Construction Site Erosion Control Measures

The following erosion control devices may be used on the project site at any time during the construction phases to ensure compliance with NR 216 and local erosion control requirements, as applicable.

a) Site Vegetation

Existing site vegetation outside of project limits shall be protected and maintained to the maximum extent practicable. Existing site vegetation within the project limits shall remain undisturbed until construction schedule warrants disturbance. For disturbed areas vegetation that resists erosion, maintains slow storm water velocities, and retains sediment from runoff shall be provided by the contractor. Temporary seeding may be required for disturbed areas that are subject to long periods of construction inactivity. Temporary vegetation is used when areas are disturbed and may remain unfinished long enough to allow vegetation to grow and assist with erosion control. Permanent vegetation is encouraged as soon as possible in the construction process.

b) Inlet Protection (WDNR 1060)

Inlet protection will be placed at all inlets to minimize sediment from entering storm drainage systems in area where the contributing drainage area is temporarily disturbed. Runoff shall be routed through a properly designed sediment trapping or settling practice upstream of the inlet. The appropriate type of inlet protection shall be installed prior to drain, drop, or curb inlet receiving runoff and maintained until the contributing drainage area is stabilized with appropriate vegetation or impervious surface.

c) Stone Tracking Pad (WDNR 1057)

Stone tracking pads will be constructed at all entrances to the construction site to minimize sediment tracking onto existing streets. A minimum of one construction entrance is required for the project site. Tracking pads are temporary and will be removed or much of the aggregate will be removed before the site is completed.

d) Waste and Material Disposal

All waste and unused building materials (including garbage, debris, cleaning wastes, or other construction materials) shall be properly disposed of and not allowed to be carried by runoff into a receiving channel or inlet.

e) Silt Fence (WDNR 1056)

Continuous silt fencing will be required along all areas downstream of disturbed area, and around the base of all stockpiled material subject to sediment transportation during rain fall events (stockpiled topsoil, gravel base, etc.). The silt fencing will provide a siltation barrier between the disturbed area and any inlets and ultimately downstream water bodies. All silt fence shall be removed upon completion of the project or when disturbed areas have generated sufficient vegetation to prevent erosion and the threat of sediment reaching inlets and bodies of water.

f) Non-channel Erosion Mat (WDNR 1052)

The purpose of this practice is to protect the soil surface from the erosive effect of rainfall and prevent sheet erosion during the establishment of grass or other vegetation, and to reduce soil moisture loss due to evaporation. This practice applies to both Erosion Control Re-vegetative Mats (ECRM) and Turf-Reinforcement Mats (TRM).

1. CLASS I: A short-term duration (minimum of 6 months), light duty, organic mat with photodegradable plastic or biodegradable netting.
   a. Type A – Use on erodible slopes 2.5:1 or flatter.

   b. Type B – Double netted product for use on erodible slopes 2:1 or flatter.

## 5.3 Operation and Maintenance, Short-term

The owner of this project is responsible for implementation and maintenance of erosion control measures during construction.

The Contractor shall conduct the following inspections:

- Weekly inspections of implemented erosion and sediment controls.
- Inspections of erosion and sediment controls within 24 hours after precipitation event 0.5 inches or greater which results in runoff during active construction periods.

The Contractor shall maintain weekly written reports of all inspections that include:

- The date, time, and exact place of the inspection.
- The name of the individual who performed the inspection.
- An assessment of the condition of erosion and sediment controls.
- A description of any erosion and sediment control implementation and maintenance performed.
- A description of the present phase of construction at the site.

Repairs shall be made immediately, as required, to maintain effectiveness until permanent vegetation is established. All repairs to erosion control devices shall be documented on the

Wisconsin Department of Natural Resources Construction Site Inspection Report (Form 3400-187). A copy of Form 3400-187 can be found in **Appendix F**.

### 5.4 Operation and Maintenance, Long-term

The owner of this project is responsible for the post-construction inspection and maintenance of the proposed stormwater best management practices as described below:

- Catch Basins, Storm Sewer, and Outfall:
  - Inspection: Look for accumulation of sediment and/or debris within catch basin, storm sewer pipe, and/or outfall. Look for damage to pipe, catch basin structure, and outfall.
  - Maintenance: Remove accumulated sediment and/or debris within the pipe, sump below catch basin, and/or within or near outfall. Repair damage to pipe, catch basin, and/or outfall.

- StormTech Chamber System:
  - Inspection: Measure and document the depth of water in basin. Inspect storm sewer for erosion or damage. Inspect trash rack for debris.
  - Maintenance: Remove any accumulated sediment and/or debris within inlets. Repair storm sewer if erosion or damage is identified. Remove accumulated debris from trash rack and weir plate.

The aforementioned inspection and maintenance schedule shall be performed after any rainfall event exceeding one inch of rainfall, and at a minimum semi-annually in early spring and fall.

All inspections and maintenance shall be documented, and the owner shall keep all inspection and maintenance records onsite and available upon request of the local municipality or the Wisconsin Department of Natural Resources.

## 6.0 SUMMARY

### 6.1 General

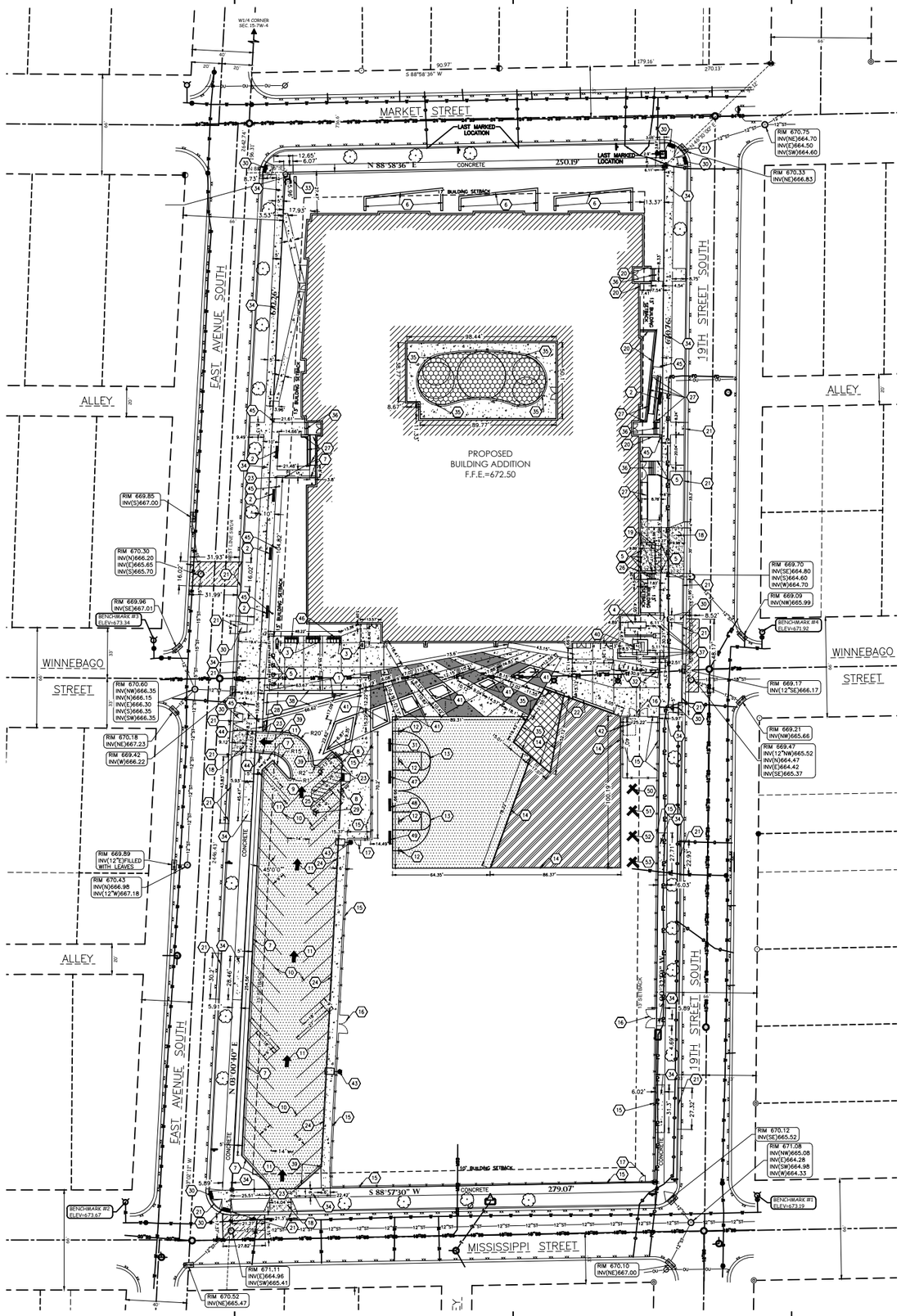
The proposed development as outlined above meets all applicable Wisconsin Department of Natural Resources storm water regulations.

For the temporary construction site scenario, sediment transport from this site to adjacent properties will be reduced by the erosion control devices and conservation practice standards.

This plan meets state storm water requirements and provides an environmentally sound and practical solution for the future storm water runoff generated from the development of this site.

# APPENDIX A

## Proposed Layout Plan




## GENERAL NOTES:

1. CONTRACTOR SHALL LOCATE ALL PUBLIC AND PRIVATE UTILITIES PRIOR TO COMMENCEMENT OF WORK.
2. GRADE, LINE, AND LEVEL TO BE REVIEWED IN THE FIELD BY THE CONSTRUCTION MANAGER.
3. ALL REQUIRED EROSION CONTROL MEASURES SHALL BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH LOCAL MUNICIPAL AND DEPARTMENT OF NATURAL RESOURCES REGULATIONS.
4. SEE SHEET C4.0 FOR ALL REQUIRED EROSION CONTROL ELEMENTS.
5. ANY EXISTING UTILITIES NOT SHOWN ON THIS DOCUMENT WHICH NEED TO BE REMOVED, RELOCATED AND OR ADJUSTED SHALL BE THE RESPONSIBILITY OF THE SITE GRADING CONTRACTOR.
6. VERIFY THE LOCATION OF ALL EXISTING UTILITIES PRIOR TO THE START OF DEMOLITION/CONSTRUCTION.
7. BIDDERS SHALL VISIT THE SITE AND REVIEW EXISTING CONDITIONS PRIOR TO THE BID DATE.
8. PRIOR TO STARTING WORK, VERIFY WITH THE LOCAL AUTHORITIES THAT ALL REQUIRED PERMITS HAVE BEEN ACQUIRED.
9. COORDINATE CONSTRUCTION IN THE RIGHT OF WAY WITH THE LOCAL AUTHORITIES.
10. PROVIDE PROPER BARRICADES, SIGNS, AND TRAFFIC CONTROL TO MAINTAIN THRU TRAFFIC ALONG ADJACENT STREETS IN ACCORDANCE WITH LOCAL MUNICIPAL REQUIREMENTS.
11. SIDEWALK JOINTS SHALL BE INSTALLED AS INDICATED OR AS APPROVED BY THE CONSTRUCTION MANAGER.
12. ALL GENERAL LANDSCAPE AREAS SHALL BE SEEDED, FERTILIZED, AND CRIMP HAY MULCHED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS.

## PAVEMENT HATCH PATTERNS:

- PROPOSED STANDARD ASPHALT PAVEMENT — 1 / C6.0
- PROPOSED STANDARD CONCRETE PAVEMENT — 2 / C6.0
- COLOR CONCRETE TYPE 1 COLOR: 'DARK GREY' BY DAVIS COLORS FINISH: LIGHT SAND BLAST — 2 / C6.0
- PROPOSED REINFORCED CONCRETE PAVEMENT — 2 / C6.0
- PROPOSED RUBBER SURFACE PLAY AREA (COURTYARD) — 11 / C6.0, 9 / C6.0
- PROPOSED WOOD CHIP PLAY AREA — 10 / C6.0, 12 / C6.0
- PROPOSED ASPHALT PAVEMENT– TO MEET CITY OF LACROSSE STANDARDS SPECIFICATIONS AND DETAILS
- PROPOSED RUBBER SURFACE PLAY AREA (OUTSIDE OF SCHOOL) — 10 / C6.0

## KEYNOTES:

1. FLAG POLE– AMERICAN BEACON EXTERNAL HALYARD SERIES 25' ANODIZED CLEAR FLAG POLE – (OR APPROVED EQUAL) INSTALL PER MANUFACTURER'S RECOMMENDATIONS — 7 / C6.0
2. BENCH – TYPE ZB.LE.02 CONCRETE TECH BENCH BY WAUSAU TILE, COLOR: A20 WHITE FINISH: WEATHERSTONE(B) OPTION: CAST–IN SKATE STOPPERS
3. CANOPY PILLAR– SEE ARCHITECTURAL PLANS
4. GENERATOR PAD – SEE ELECTRICAL PLANS
5. REINFORCED CONCRETE BOLLARD– SEE ARCHITECTURAL PLANS
6. GARDEN WALL – SEE ARCHITECTURAL PLANS
7. 18" CONCRETE CURB & GUTTER–STANDARD — 3 / C6.0
8. ADA PARKING SIGN — 8 / C6.0
9. ADA PARKING STALL– STANDARD — 2 / C6.1
10. PARKING LOT STRIPING — 2 / C6.1
11. DIRECTIONAL ARROW — 1 / C6.1
12. BASKETBALL HOOP– KEEPER GOALS PRODUCT ALL–AMER–IN–GL 60" IN–GROUND BASKETBALL HOOP (OR APPROVED EQUAL) INSTALL PER MANUFACTURER'S RECOMMENDATIONS
13. PLAYGROUND STRIPING– 4" WIDE LINE, WHITE TRAFFIC PAINT (VERIFY LOCATION & DIMENSIONS W/OWNER PRIOR TO CONSTRUCTION)
14. PLAY AREA CURBING — 10 / C6.0
15. 6' TALL CHAIN LINK FENCING — 5 / C6.1, 6 / C6.1
16. 12' SERVICE GATE — 4 / C6.1
17. 4' PEDESTRIAN GATE — 3 / C6.1
18. DRIVE APRON — 2 / C6.3
19. DUMPSTER ENCLOSURE– SEE ARCHITECTURAL PLANS
20. ARCHITECTURAL WALL– SEE ARCHITECTURAL PLANS
21. CONCRETE CURB & GUTTER– TO MEET CITY OF LACROSSE STANDARDS SPECIFICATIONS AND DETAILS — 4 / C6.3
22. BENCH WALL – SEE ARCHITECTURAL PLANS
23. 6" CURB HEAD TAPER SECTION — 5 / C6.0
24. THICKENED EDGE WALK — 4 / C6.0
25. ADA CURB RAMP 'A' — 6 / C6.0
26. CONCRETE STAIRS W/RAILING — 5 / C6.2
27. RAMP & RAILING– SEE ARCHITECTURAL PLANS
28. STOP SIGN — 7 / C6.1
29. ADA PARKING STALL–VAN ACCESSIBLE — 2 / C6.1
30. ADA CURB RAMP 'B' — 1 / C6.3
31. THEORY – THICK BEAM BENCH BY LANDSCAPE FORMS, LENGTH: 8 FOOT TYPE: THICK BENCH W/SOLID SEAT & 2 SKATE STOPS COLOR: APPLE RED FINISH: POWDERCOAT (ALTERNATE #4)
32. KEY BIKE RACK BY LANDSCAPE FORMS (9) COLOR: RED FINISH: POWDERCOAT
33. MONUMENT SIGN– SEE ARCHITECTURAL PLANS
34. CONCRETE SIDEWALK– TO MEET CITY OF LACROSSE STANDARD SPECIFICATIONS AND DETAILS — 2 / C6.0
35. CONCRETE CURB ADJACENT TO POURED RUBBER SURFACE — 11 / C6.0
36. CONCRETE STAIRS & RAILING– SEE ARCHITECUREAL PLANS
37. SCREEN WALL & GATES– SEE ARCHITECTURAL PLANS
38. PLANTER BENCH– SEE ARCHITECTURAL PLANS
39. 18" CONCRETE CURB & GUTTER –REJECT — 3 / C6.0
40. TRANFORMER PAD– SEE ELECTRICAL PLANS
41. PLANTER BENCH– SEE ARCHITECTURAL PLANS. FILL INTERIOR OF PLANTER WITH 12" OF TOPSOIL. TOP OF TOPSOIL TO BE HELD 4" BELOW BOTTOM OF BENCH CAP.
42. UNDERGROUND GEOTHERMAL VAULT, SHOWN FOR REFERENCE ONLY. SEE GEOTHERMAL PLANS FOR DETAILS.
43. LIGHT POLE– SEE ELECTRICAL PLANS
44. 'DO NOT ENTER' SIGN — 7 / C6.1
45. CONCRETE BOLLARD– SEE ARCHITECTURAL PLANS
46. BENCH– SEE ARCHITECTURAL PLANS
47. THEORY – THICK BEAM BENCH BY LANDSCAPE FORMS, LENGTH: 8 FOOT TYPE: THICK BENCH W/SOLID SEAT & 2 SKATE STOPS COLOR: NAVY BLUE FINISH: POWDERCOAT (ALTERNATE #4)
48. THEORY – THICK BEAM BENCH BY LANDSCAPE FORMS, LENGTH: 8 FOOT TYPE: THICK BENCH W/SOLID SEAT & 2 SKATE STOPS COLOR:LEAF GREEN FINISH: POWDERCOAT (ALTERNATE #4)
49. THEORY – THICK BEAM BENCH BY LANDSCAPE FORMS, LENGTH: 8 FOOT TYPE: THICK BENCH W/SOLID SEAT & 2 SKATE STOPS COLOR: LEMON FINISH: POWDERCOAT (ALTERNATE #4)
50. (2)THEORY – THICK BEAM BENCHES BY LANDSCAPE FORMS (LAYOUT 10) LENGTH: 8 FOOT TYPE: THICK BENCH W/SOLID SEAT & 2 SKATE STOPS COLOR: APPLE RED FINISH: POWDERCOAT (ALTERNATE #4)
51. (2) THEORY – THICK BEAM BENCHES BY LANDSCAPE FORMS (LAYOUT 10) LEGTH: 8 FOOT TYPE: THICK BENCH W/SOLID SEAT & 2 SKATE STOPS COLOR: NAVY BLUE FINISH: POWDERCOAT ALTERNATE #4
52. (2) THEORY – THICK BEAM BENCHES BY LANDSCAPE FORMS LENGTH: 8 FOOT TYPE: THICK BENCH W/SOLID SEAT & 2 SKATE STOPS COLOR: LEAF GREEN FINISH: POWDERCOAT (ALTERNATE #4)
53. (2) THEORY– THICK BEAMS BENCHES BY LANDSCAPE FORMS LENGTH: 8 FOOT TYPE: THICK BENCH W/SOLID SEAT & 2 SKATE STOPS COLOR: LEMON FINISH: POWDERCOAT (ALTERNATE #4)

## BENCHMARK:

ELEVATIONS ARE REFERENCED TO NAVD 88 DATUM.

BENCHMARK #1
NORTHWEST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHEAST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND 19TH STREET SOUTH. ELEVATION = 673.19

BENCHMARK #2
NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND EAST AVENUE SOUTH. ELEVATION = 673.67

BENCHMARK #3
NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST CORNER OF THE INTERSECTION OF EAST AVENUE SOUTH AND WINNEBAGO STREET. ELEVATION = 673.34

BENCHMARK #4
BURY BOLT ON HYDRANT, LOCATED AT THE NORTHEAST CORNER OF THE INTERSECTION OF 19TH STREET SOUTH AND WINNEBAGO STREET.

## UTILITY DISCLAIMER:

THE LOCATIONS, SIZES, AND TYPES OF UNDERGROUND PUBLIC AND PRIVATE UTILITIES OR SUBSTRUCTURES SHOW HEREON WERE OBTAINED FROM VISUAL INSPECTION, FIELD MEASUREMENTS, AND/OR AS–BUILT PLANS. SANITARY SEWER AND STORM SEWER PIPE SIZES, INVERTS, DIRECTION, AND LOCATIONS BETWEEN MANHOLES ARE SUPPLEMENTED BY AS–BUILT PLANS AND/OR ESTIMATED BASED ON FIELD OBSERVATIONS. PRIOR TO CONSTRUCTION IN THE VICINITY OF ANY UTILITIES SHOWN HEREON, IT IS RECOMMENDED THAT THE LOCATIONS, DEPTHS, AND SIZES BE FIELD VERIFIED. THE LOCATIONS SHOWN HEREON ARE ONLY APPROXIMATE, WITH POSSIBILITY THAT ADDITIONAL UTILITY LINES NOT DISCOVERED, OR MARKED, DURING THE SEARCH OF RECORDS AND THE FIELD SURVEY MAY EXIST. ANY CONTRACTOR USING THE INFORMATION SHOWN HEREON IS HEREBY FOREWARNED THAT ANY EXCAVATION UPON THIS SITE MAY RESULT IN THE DISCOVERY OF ADDITIONAL UNDERGROUND UTILITIES NOT SHOWN HEREON. IN GENERAL, UNDERGROUND UTILITY LOCATIONS ARE SHOWN FROM UTILITY MARKINGS, BY OTHERS, AND/OR AS–BUILT PLANS, PROVIDED BY OTHERS. POINT OF BEGINNING MAKES NO WARRANTY OF ANY KIND, EXPRESS OR IMPLIED, WITH RESPECT TO THE EXISTING UTILITIES SHOWN HEREON, AND BELIEVES THAT THE INFORMATION CONTAINED HEREIN IS RELIABLE AND GENERALLY ACCURATE FOR THE PURPOSE INTENDED.

0' 30' 60' — 1" = 30'

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Project Title:
ADDITIONS AND ALTERATIONS TO:
LA CROSSE NEW ELEMENTARY SCHOOL
LA CROSSE SCHOOL DISTRICT
807 EAST AVE S.
LA CROSSE, WI 54601

REVISIONS:

| | DATE | DESCRIPTION |
|---|---|---|

NOT FOR CONSTRUCTION

Project Number: 3752

Issued For: CONTRACT DOCUMENTS 09/30/2025

Sheet Title: Layout Plan

Sheet Number: C2.0

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# APPENDIX B

## Geotechnical Data




Map Scale: 1:1,220 if printed on A portrait (8.5" x 11") sheet.

N

Meters
0 15 30 60 90

Feet
0 50 100 200 300

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 15N WGS84

## MAP LEGEND

**Area of Interest (AOI)**

- Area of Interest (AOI)

**Soils**

- Soil Map Unit Polygons
- Soil Map Unit Lines
- Soil Map Unit Points

**Special Point Features**

- Blowout
- Borrow Pit
- Clay Spot
- Closed Depression
- Gravel Pit
- Gravelly Spot
- Landfill
- Lava Flow
- Marsh or swamp
- Mine or Quarry
- Miscellaneous Water
- Perennial Water
- Rock Outcrop
- Saline Spot
- Sandy Spot
- Severely Eroded Spot
- Sinkhole
- Slide or Slip
- Sodic Spot
- Spoil Area
- Stony Spot
- Very Stony Spot
- Wet Spot
- Other
- Special Line Features

**Water Features**

- Streams and Canals

**Transportation**

- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

**Background**

- Aerial Photography

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:12,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: La Crosse County, Wisconsin
Survey Area Data: Version 23, Sep 3, 2024

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jul 31, 2020—Sep 2, 2020

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

# Map Unit Legend

| Map Unit Symbol | Map Unit Name | Acres in AOI | Percent of AOI |
|---|---|---|---|
| 2020 | Urban land, valley trains | 4.2 | 100.0% |
| **Totals for Area of Interest** | | **4.2** | **100.0%** |



# Geotechnical Engineering Exploration and Analysis

## Proposed Elementary School
## 807 East Avenue South
## La Crosse, Wisconsin

**Prepared for:**

**School District of La Crosse**
**La Crosse, Wisconsin**

**May 9, 2025**
**Giles Project No. 1G-2502021-3**

GILES
ENGINEERING ASSOCIATES, INC.

GILES
ENGINEERING ASSOCIATES, INC.
GEOTECHNICAL, ENVIRONMENTAL & CONSTRUCTION MATERIALS CONSULTANTS

• Dallas, TX
• Los Angeles, CA
• Manassas, VA
• Milwaukee, WI

May 9, 2025

School District of La Crosse
Hogan Administrative Center
807 East Avenue South
La Crosse, WI 54601

Attention: Mr. Joe Ledvina
Director of Facilities

Proposal for: Geotechnical Engineering Exploration and Analysis
Proposed Elementary School
807 East Avenue South
La Crosse, Wisconsin
Giles Project No. 1G-2502021-3

Dear Mr. Ledvina:

As requested, Giles Engineering Associates, Inc. conducted a *Geotechnical Engineering Exploration and Analysis* for the proposed project. The accompanying report describes the services that were performed, and it provides geotechnical-related findings, conclusions, and recommendations that were derived from those services.

We sincerely appreciate the opportunity to provide geotechnical consulting services for the proposed project. Please contact the undersigned if there are questions about the report or if we may be of further service.

Very truly yours,

GILES ENGINEERING ASSOCIATES, INC.

Grace C. Hill
Staff Professional

Colleen M. Finley, P.E.
Geotechnical Department Manager

WISCONSIN
COLLEEN M. FINLEY
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PROFESSIONAL ENGINEER

Distribution: Bray Architects
Attn: Mr. Nathan Derks (nderks@brayarch.com)

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GEOTECHNICAL ENGINEERING EXPLORATION AND ANALYSIS

PROPOSED ELEMENTARY SCHOOL
807 EAST AVENUE SOUTH
LA CROSSE, WISCONSIN
GILES PROJECT NO. 1G-2502021-3


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Appendix B - Field Procedures
Appendix C - Laboratory Testing and Classification
Appendix D - General Information and Important Information about This Geotechnical Report


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GEOTECHNICAL ENGINEERING EXPLORATION AND ANALYSIS

PROPOSED ELEMENTARY SCHOOL
807 EAST AVENUE SOUTH
LA CROSSE, WISCONSIN
GILES PROJECT NO. 1G-2502021-3

## EXECUTIVE SUMMARY

This Executive Summary provides limited geotechnical information regarding the proposed project. Because this Executive Summary is exceedingly abbreviated, it must be read in complete context with the following report ("Report").

### Material Conditions

- Eighteen geotechnical test borings and three test pits were conducted at the site to explore subsurface conditions.
- Topsoil was at the surface of test borings 1, 2, 5, 6, 7, 8, 10, 11, 12, 13, 14, 15, and 16 and generally consisted of sandy silt or silty sand with estimated trace to little amounts of organic matter. The topsoil thickness varied between ±5 and ±10 inches. Asphalt-concrete that was ±6 and ±4 inches thick was at the surface of Test Boring 17 and 18, respectively. Base material that was ±10 and ±8.5 inches was below the asphalt-concrete, respectively.
- Fill material was at the surface of Test Borings 3, 4, and 9 and consisted of sandy silt and silty sand. Fill material was also below the surface materials at the remaining test borings. Fill material was encountered to depths between ±6½ to ±9 feet below-ground at Test Borings 1 through 16, to a depth of ±2 feet below ground at Test Borings 17 and 18, and to a depth of ±3 feet below ground at Test Pits TP-1 and TP-2. The fill material generally consisted of sand (variable gradations) with variable amounts of silt and gravel. Asphalt rubble was encountered within the fill material at Test Boring 6 and pulverized asphalt within the fill at Test Pit TP-1. Metal debris was encountered within the fill material at Test Pit TP-2. Based on corrected SPT N-values, the fill material exhibited loose to firm relative densities.
- Native sand with variable amounts of silt was beneath the material discussed above and was encountered to the ±16 and ±26-foot termination depth at each test boring. Based on SPT N-Values, the native sand exhibited loose to firm relative densities.
- It is estimated that the water table was deeper than the ±16- to ±26-foot termination depth at the test boring locations when the test borings were conducted. However, groundwater conditions will likely fluctuate, and the water table could be shallower at certain times.

### Building Foundation

- The proposed building can be supported by spread-footing foundations; however, because of the existing fill, a layer of compacted aggregate is recommended to be directly below each footing. The aggregate layer is recommended to be at least 12 inches thick and must extend horizontally at least 6 inches beyond the bottom edges of each footing pad.

**Building Foundation (Continued)**

- Assuming that all foundations will be supported by the recommended aggregate layer, a 2,000 pound per square foot (psf) maximum, net, allowable soil bearing capacity is recommended for foundation design. For geotechnical considerations and regardless of the calculated foundation-bearing stress, strip footings are recommended to be at least 16 inches wide and isolated footings are recommended to be at least 24 inches wide and long.

**At-Grade Floor**

- With proper subgrade preparation, it is expected that site soil will be suitable for support of at-grade floor slabs. Engineered fill that is selected, placed, and compacted according to the Report could also support concrete floor slabs. However, subgrade improvement might be necessary to develop uniform slab support, especially considering the low strength of some of the surface soils and existing fill material.
- Assuming proper site preparation, the floor slab may be designed using a *Modulus of Subgrade Reaction* ($K_{v1}$) value of 150 pounds per square inch per inch (psi/in). It is recommended and assumed that a structural engineer will specify the actual floor slab thickness, reinforcing, joint details, and other parameters.
- A minimum 4-inch-thick aggregate base course and a 10-mil vapor retarder are recommended to be below the floor slab. It is recommended that a structural engineer or architect specify the vapor retarder position (above or below the base course).

**Pavement Areas**

- Hot-mix asphalt (HMA) pavement is adequate for this site, except that Portland cement concrete (PCC) pavement is recommended in areas of higher traffic stress, such as at entrance and exit aprons, at trash enclosures, and in areas where buses and trucks will turn or will be parked.

**Preliminary Stormwater Infiltration Considerations**

- Based on the conditions encountered within stormwater management area, the underlying sand soils are generally suitable for stormwater infiltration. Based on the sandy soils and the correlations provided in WDNR Conservation Practice Standard 1002, a maximum infiltration rate of 1.63 inches per hour is considered appropriate for infiltration within the loamy sand site soils.

GEOTECHNICAL ENGINEERING EXPLORATION AND ANALYSIS

PROPOSED ELEMENTARY SCHOOL
807 EAST AVENUE SOUTH
LA CROSSE, WISCONSIN
GILES PROJECT NO. 1G-2502021-3

## 1.0 SCOPE OF SERVICES

This report provides the results of the *Geotechnical Engineering Exploration and Analysis* that Giles Engineering Associates, Inc. ("Giles") conducted for the proposed new school project. The *Geotechnical Engineering Exploration and Analysis* included a geotechnical subsurface exploration program, geotechnical laboratory services, and geotechnical engineering. The scope of each service area was narrow and limited, as directed by our client and based on our understanding and assumptions about the proposed project. Services are briefly described later. Environmental consulting was beyond Giles' authorized scope for this project.

Geotechnical-related recommendations are provided in this report for design and construction of the foundation and at-grade floor for the proposed school building. Recommendations are also provided for pavement parking lots and drives. Furthermore, information is provided regarding stormwater infiltration within the proposed stormwater management areas. Site preparation recommendations are also given but are only preliminary because the means and methods of site preparation will depend on factors that were unknown when this report was prepared. These factors include, but are not limited to, the weather before and during construction, the subsurface conditions that are exposed during construction, and the final details of the proposed project.

## 2.0 SITE DESCRIPTION

The subject site is located at 807 East Avenue South in La Crosse, Wisconsin. The site is bounded by Market Street, Mississippi Street, and 19th Street South to the north, south, and east, respectively. The site is shown on the *Test Boring and Test Pit Location Plan* , enclosed as Figure 1 in Appendix A. When the test borings (described later) were performed, the site was occupied by the existing Hogan Administrative Building, a paved parking lot, recreation fields, and grassy areas. Topographically, the site is relatively flat and level. Based on topographic contours shown on the *Layout Plan*, dated March 14, 2025, prepared by Bray Architects, the ground surface elevations at the site were between El. 670 and El. 674.

Based on historical aerial imagery, it appears the site has remained relatively unchanged. However, the 1906 Sanborn Fire Insurance Map shows that Winnebago Street previously ran through the site and a railroad line ran north-south on the east of East Avenue South Street.

## 3.0 PROJECT DESCRIPTION

Proposed School Building

A new school building will be constructed at the location shown in the *Test Boring and Test Pit Location Plan .* It is understood that the proposed school building will be an approximately 55,700

square foot, two-story, at-grade structure that will not have basements or other below-grade areas. It is assumed that the school will be a masonry structure with a bar-joist and metal-deck roof system. Interior and perimeter walls of the school building will be constructed of concrete masonry units (CMU). The at-grade floor is planned to be a ground-bearing concrete slab. It is assumed that bearing walls and columns will support the building. Maximum foundation loads were not provided to us and were, therefore, assumed to be 5,000 pounds per lineal foot (plf) from bearing walls and 200,000 pounds per column. This report assumes that the maximum floor load will be 100 pounds per square foot (psf).

The proposed at-grade floor elevation for the proposed school building is understood to be El. 671, based on the *Layout Plan*. Based on the test boring elevations and proposed floor elevation, only minor grade changes (two-foot maximum) are expected in the proposed building area.

### Proposed Pavement

It is understood that hard play surfaces and new parking areas and drives will be constructed at the site, as shown in the *Test Boring Location Plan*. It is assumed that new pavement will consist of asphalt-concrete, but Portland cement concrete pavement will be in areas of higher traffic stress. Because Giles was not provided with traffic information, the pavement recommendations provided in this report are based on arbitrarily assumed traffic conditions. Also, this report assumes that pavement surface grades will be within about two feet of the current ground grades.

### Proposed Stormwater Management

It is understood that stormwater management areas are planned on the southwest and southeast areas of the property, in the area of Test Pits TP-1 through TP-3. Additional details regarding the stormwater management areas were not provided. Therefore, this report assumes that stormwater management basins will be several feet deep, measured from existing ground grades.

## 4.0 GEOTECHNICAL SUBSURFACE EXPLORATION PROGRAM

### Test Borings

To explore subsurface conditions, eighteen geotechnical test borings were conducted at the site using a mechanical drill-rig. Test Borings 1 through 16 were in the proposed building area and were advanced to ±26 feet below-ground. Test Borings 17 and 18 were in proposed pavement areas and were advanced to ±16 feet below-ground. Test boring locations were positioned at the site based on measurement from existing site features and apparent property lines, and by estimating right angles. Approximate locations of the test borings are shown on the *Test Boring and Test Pit Location Plan*.

Samples were collected from each test boring, at certain depths, using the Standard Penetration Test (SPT), conducted with the drill rig. A brief description of the SPT is given in Appendix B along

with descriptions of other field procedures. Immediately after sampling, select portions of the SPT samples were placed in containers that were labeled at the site for identification. A Standard Penetration Resistance value (N-value) was determined from each SPT. N-values are reported on the *Test Boring Logs* (in Appendix A), which are records of the test borings. N-values are used to estimate the in-place density of granular soil, such as the granular soil that was encountered at the test borings, as described later.

The boreholes were backfilled upon completion; however, backfill material will likely settle or heave, creating a hazard that can injure people and animals. Borehole areas should, therefore, be carefully and routinely monitored by the School District of La Crosse or by others; settlement and heave of backfill materials should be repaired immediately. Giles will not monitor or repair boreholes.

### Test Pits

On April 30, 2025, three test pits were excavated in the proposed stormwater management areas to observe subsurface conditions with regard to stormwater infiltration. The test pit locations were positioned on-site based on field measurements from existing site features. Approximate locations of the test pits are shown on the *Test Boring and Test Pit Location Plan*.

The test pits were excavated using an excavator equipped with a toothed bucket. Each test pit was planned to be excavated to a depth of 15 feet; however the test pits were terminated at depths of ±5, ±11, and ±9 feet due to caving soils at Test Pits TP-1, TP-2, and TP-3, respectively. A Giles representative observed the excavation procedures and logged the subsurface conditions within the test pit. The test pits were backfilled after the subsurface conditions were logged. However, backfill material was loosely placed and is, therefore, unsuitable for structural or pavement support.

### Elevations

The ground elevations at the test boring and test pit locations were estimated from topographic contour lines shown on the *Layout Plan*. The test boring elevations are noted on the *Test Boring Logs* and the test pit elevations are noted on the Wisconsin DSPS *Soil and Site Evaluation – Stormwater Infiltration* logs. The reported elevations are assumed to be accurate within about one foot.

## 5.0 GEOTECHNICAL LABORATORY SERVICES

Samples that were retained from the test borings were transported to Giles' geotechnical laboratory where the samples were classified using the descriptive terms and particle-size criteria shown on the *General Notes* in Appendix D and by using the Unified Soil Classification System (ASTM D 2488) as a general guide. The classifications are shown on the *Test Boring Logs* along with horizontal lines that show estimated depths of material change. Field-related information

pertaining to the test borings is also shown on the *Test Boring Logs*. For simplicity and abbreviation, terms and symbols are used on the *Test Boring Logs;* the terms and symbols are defined on the *General Notes*.

Soil conditions observed within Test Pits TP-1 through TP-3 (conducted in the proposed stormwater management areas) were visually classified using the USDA textural classification system in general accordance with the guidelines provided in the *Field Book for Describing and Sampling Soils* (USDA, Sept. 2012). USDA classifications of the retained samples are shown on the Wisconsin DSPS *Soil and Site Evaluation – Stormwater Infiltration* logs, enclosed in Appendix A. Supplemental information regarding soil classifications, including the USDA and USCS soil classification systems, is included in the *Soil Classification Notes* enclosure within Appendix D.

## 6.0 MATERIAL CONDITIONS

Because material sampling at the test borings was discontinuous, it was necessary to estimate conditions between sample intervals. Estimated conditions at the test borings are briefly discussed in this section and are described in more detail on the *Test Boring Logs*. The conclusions and recommendations in this report are based only on the estimated conditions.

### 6.1. Surface Materials

Topsoil was at the surface of test borings 1, 2, 5, 6, 7, 8, 10, 11, 12, 13, 14, 15, and 16 and generally consisted of sandy silt or silty sand with estimated trace to little amounts of organic matter. The topsoil thickness varied between ±5 and ±10 inches. Asphalt-concrete that was ±6 and ±4 inches thick was at the surface of Test Borings 17 and 18, respectively. Base material that was ±10 and ±8.5 inches was below the asphalt-concrete at Test Borings 17 and 18, respectively.

### 6.2. Fill Materials

Fill material was at the surface of Test Borings 3, 4, and 9 and consisted of sandy silt and silty sand. Fill material was also below the surface materials at the remaining test borings. Fill material was encountered to depths between ±6½ to ±9 feet below-ground at Test Borings 1 through 16, to a depth of ±2 feet below ground at Test Borings 17 and 18, and to a depth of ±3 feet below ground at Test Pits TP-1 and TP-2. The fill material generally consisted of sand (variable gradations) with variable amounts of silt and gravel. Asphalt rubble was encountered within the fill material at Test Boring 6 and pulverized asphalt within the fill at Test Pit TP-1. Metal debris was encountered within the fill material at Test Pit TP-2. Based on corrected SPT N-values, the fill material exhibited loose to firm relative densities.

### 6.3. Native Soil

Native sand with variable amounts of silt was beneath the fill materials and was encountered to the ±16- and ±26-foot termination depth at each test boring. Based on SPT N-Values, the native sand exhibited loose to firm relative densities.

## 7.0 GROUNDWATER CONDITIONS

It is estimated that the water table was deeper than the ±16- to ±26-foot termination depths at the test boring locations when the test borings were conducted. However, groundwater conditions will likely fluctuate, and the water table could be shallower at certain times. The estimated groundwater conditions is only an approximation based on the colors and relative moisture conditions of the retained soil samples, and the lack of groundwater within the test borings. The water table could be shallower than estimated. Groundwater observation wells can be installed and observed at the site to further evaluate the groundwater conditions. Giles can install and monitor observations wells.

## 8.0 CONCLUSIONS AND RECOMMENDATIONS

### 8.1. Seismic Design Considerations

A soil Site Class D is recommended for seismic design. Site Class is based on the average properties of subsurface materials to 100 feet below-ground. Because 100-foot test borings were not requested or authorized for the project, it was necessary to estimate the Site Class based on the test borings, presumed area geology, and the International Building Code.

### 8.2. School Building Foundation Recommendations

The proposed school building can be supported by spread-footing foundations; however, because of the existing fill, a layer of compacted aggregate is recommended to be directly below each footing. The aggregate layer is recommended to be at least 12 inches thick and must extend horizontally at least 6 inches beyond the bottom edges of each footing pad. Aggregate is recommended to consist of dense-graded crushed stone that meets the gradation requirements of *dense-graded base* (1¼-inch) in Section 305 of the Wisconsin Department of Transportation Standard Specifications (2019). Aggregate with other gradation characteristics could possibly also be used but must be approved by Giles before the material is placed.

Aggregate is recommended to be placed in uniform layers that are a maximum of 8 inches thick (measured loose), and each layer is recommended to be compacted to at least 95 percent of the material's maximum dry density determined from the Standard Proctor compaction test (ASTM D698). The water content of aggregate is recommended to be uniform and within a narrow range of the optimum moisture content, also determined by the Standard Proctor compaction test. A subsequent layer of aggregate should not be placed until a geotechnical engineer confirms (by testing and observation) that the previous layer was properly compacted to the required in-place density. Full-time observation and testing by a geotechnical engineer during placement and compaction of each aggregate layer is critical.

Existing granular soil that is within the foundation influence zone but does not meet the recommended strength criteria is recommended to be compacted to the required in-place density. Compaction must be done with care and in accordance with the recommendations of a geotechnical engineer. Soil might need to be scarified and moisture-conditioned (uniformly moistened or dried) before compaction.

Assuming that all foundations will be supported by the recommended aggregate layer, a 2,000 pound per square foot (psf) maximum, net, allowable soil bearing capacity is recommended for foundation design. For geotechnical considerations and regardless of the calculated foundation-bearing stress, strip footings are recommended to be at least 16 inches wide and isolated footings are recommended to be at least 24 inches wide and long. Also, due to the existing fill and possible fill, and from a geotechnical perspective, foundation walls are recommended to be constructed of reinforced cast-in-place concrete, rather than concrete masonry units. Specific foundation details including footing and wall dimensions, reinforcing, and other details are recommended to be specified by the project structural engineer.

A minimum 48-inch foundation-embedment depth is required by the building code. It is, therefore, recommended that footings for perimeter walls and other exterior elements of the building bear at least 48 inches below the finished ground-grade at the perimeter of the building. Because it is understood that the entire building will be heated, interior footings can bear above the 48-inch embedment depth.

A frictional coefficient of 0.40 is recommended to determine lateral resistance at the base of the foundation. The recommended frictional coefficient is only for concrete cast directly on suitable native soil, or on new engineered fill or lean-concrete backfill used to replace unsuitable materials. Lateral resistance due to friction should be determined based on dead load only. Also, the ultimate lateral resistance determined from the frictional coefficient is recommended to be factored to determine an allowable value. Passive resistance is recommended to be neglected to at least the recommended 48-inch foundation-embedment depth due to seasonal changes and due to the amount of lateral movement necessary to develop full passive pressure.

Foundation excavations are recommended to be dug with a smooth-edge bucket to develop a relatively undisturbed bearing grade. A toothed bucket will likely disturb foundation-bearing soil more than a smooth-edge bucket thereby making soil at the excavation base more susceptible to saturation and instability, especially during adverse weather. It is critical that contractors protect foundation-support soil and foundation construction materials (concrete and reinforcing). Furthermore, engineered fill is recommended to be placed and compacted in benched excavations along foundation walls immediately after the foundation walls can properly support lateral pressures from backfill, compaction, and compaction equipment. Earth-formed footing construction techniques are likely not feasible due to caving of granular soil.

Foundation Support-Soil Requirements

As previously described, each footing is recommended to be directly supported by a compacted-aggregate layer. Aggregate may be placed on suitable existing soil, including existing fill, possible fill, and native soil. Based on the recommended 2,000 psf maximum, net, allowable soil bearing capacity, the in-situ unconfined compressive strength of cohesive native soil (if any) within the foundation influence zone is recommended to be at least 1.0 tons per square foot (tsf). Granular soil, such as sand, within the foundation influence zone is recommended to have a corrected N-value (determined from SPTs and correlated from other in-situ tests) of at least 6, based on the recommended bearing capacity. It is further recommended that the strength characteristics of soil within all foundation influence zones (determined by a geotechnical engineer during construction) meet or exceed the recommended values unless Giles approves other values.

Because of the existing fill and existing development, full-time evaluation by a geotechnical engineer during excavation for the recommended aggregate layer for each building is critical. The purpose of the recommended evaluation is (1) to confirm that the aggregate layer will be placed on suitable existing soil, (2) to determine the required thickness and width of the aggregate layer, and (3) to confirm that the subsurface conditions are similar to those described on the *Test Boring Logs*. If a firm other than Giles performs the recommended evaluations, Giles must be notified if the composition or strength characteristics of existing soils differ from those shown on the *Test Boring Logs*; revision of this report might be necessary. Without evaluation and approval of foundation support materials by a geotechnical engineer, the proposed buildings could be improperly supported, which could lead to excessive settlement and structural distress.

Estimated Foundation Settlement

The post-construction total and differential settlements of a spread-footing foundation designed and constructed based on this report are estimated to be less than about 1 inch and ½ inch, respectively. These estimated settlements assume that the recommendations provided in this report will be followed and that foundation-support soil will be evaluated and approved by a geotechnical engineer during construction.

### 8.3. At-Grade Floor Slab Recommendations

With proper subgrade preparation, existing soil is expected to be suitable to support at-grade floor slabs for the proposed school building; new engineered fill that is placed on properly prepared existing soil is also expected to be suitable for floor slab support. However, subgrade improvement might be necessary to develop uniform slab support, especially considering the low strength of some of the surface soils and existing fill material. Consequently, the entire floor area is recommended to be thoroughly evaluated and approved by a geotechnical engineer immediately before fill placement and before floor construction.

Assuming proper site preparation, the floor slab may be designed using a *Modulus of Subgrade Reaction* ($K_{v1}$) value of 150 pounds per square inch per inch (psi/in). It is recommended and assumed that a structural engineer will specify the actual floor slab thickness, reinforcing, joint details, and other parameters.

A minimum 4-inch-thick base course is recommended to be below the floor slabs to serve as a capillary break. It is recommended that the base course consist of compacted free-draining crushed stone that meets the gradation requirements of ASTM No. 57 aggregate. Depending on the subgrade condition and materials, geotextile might need to be below the base course to serve as a separator, especially where the subgrade consists of cohesive soil. The need for geotextile should be determined during construction with the assistance of a geotechnical engineer.

A minimum 10-mil vapor retarder is recommended to be directly above or below the base course throughout all floor areas, including the garage area. The position (above or below the base course) of the vapor retarder should be specified by the project structural engineer or architect. Vapor retarder sheets are recommended to be overlapped at least 6 inches, and the overlaps are recommended to be continuously taped. Also, vapor retarder must extend to all foundation walls. Furthermore, vapor retarder is recommended to be in accordance with ASTM E 1745, entitled *Standard Specification for Plastic Water Vapor Retarders Used in Contact with Soil or Granular Fill under Concrete Slabs*, and other relevant documents. If the base course contains sharp aggregate, protecting the retarder with geotextile or by other means is recommended.

Certain areas of the floor slab (such as near exterior doors and entrance-exit vestibules) might be susceptible to freeze-thaw movement. Installation of insulation or other protective measures against freeze-thaw movement should be considered for floor areas that are susceptible to freeze-thaw. Pavement and ground grades are recommended to be sloped away from the building and sidewalks to reduce water infiltration and potential freeze-thaw problems.

Estimated Floor Slab Settlement

The post-construction total and differential settlements of an isolated floor slab constructed in accordance with this report are estimated to be less than about ⅝ inch and ⅜ inch, respectively, over about 20 feet. Estimated settlements assume that floor slab support materials will be thoroughly tested and approved by a geotechnical engineer.

### 8.4. Pavement Recommendations

Traffic-related information was not provided to us. Therefore, recommendations are provided herein for light-duty vehicle areas, medium-duty vehicle areas, and recreation areas. The light-duty pavement section is for passenger-vehicle parking lots and is based on an assumed traffic condition of five 18-kip Equivalent Single Axle Loads (ESALs) per day. The medium-duty pavement section is for drives that will be subject to buses and other heavy vehicles and is based on an assumed traffic condition consisting of fifteen 18-kip ESALs per day. The light-duty and

medium-duty pavement sections assume no increase in traffic volume and no changes in vehicle type or traffic pattern. It is assumed that the ESALs noted above will be in one direction for each lane. The recreation-area pavement section is for student recreation areas that will be subject to a very limited amount of vehicle traffic, such as due to snow removal or for infrequent maintenance.

It is important that the project owner, developer, civil engineer, and other design professionals involved with the project confirm that the ESALs noted above are appropriate for the expected traffic conditions, vehicle types, and axle loadings. If requested, Giles can provide supplemental pavement recommendations based on other traffic conditions, vehicle types, and axle loads. The recommended pavement sections could underperform or fail prematurely if the design ESALs are exceeded.

Based on the test borings and with proper subgrade preparation, it is expected that pavement support materials will consist of silty sand and sand fill. Therefore, the recommended pavement sections were developed based on an assumed field CBR value of 10 and a *Modulus of Subgrade Reaction* ($K_{V1}$) value of 150 psi/in. Engineered fill that is placed in proposed pavement areas is recommended to have a field CBR value and a *Modulus of Subgrade Reaction* ($K_{V1}$) value at least equal to these design values. Fill is recommended to be placed and compacted per this report.

Asphalt-Concrete Pavement

The following table shows the recommended thicknesses for hot-mix asphalt (HMA) pavement with an aggregate base course. State specifications are also included in the table. The recommended pavement sections are based on the traffic conditions described above.

**TABLE 1**
**RECOMMENDED HMA PAVEMENT SECTION**

| Materials | Recreation Areas | Light Duty | Medium Duty | Wisconsin DOT Standard Specifications |
|---|---|---|---|---|
| Hot-Mix Asphalt Surface Course | 1.5 inches | 1.5 inches | 1.5 inches | Section 460 |
| Hot Mix Asphalt Binder Course | 1.5 inches | 2.0 inches | 2.5 inches | Section 460 |
| Dense-Graded Aggregate Base Course | 6.0 inches | 6.0 inches | 6.0 inches | Section 305, 1¼-inch Crushed Stone |

Portland Cement Concrete Pavement

Portland cement concrete (PCC) pavement is recommended in areas of higher traffic stress, such as parking lot entrance and exit aprons, at refuse enclosures, and in areas where trucks and buses will turn or will be parked. Based on the assumed ESALs, discussed above, concrete

pavement is recommended to be at least 6 inches thick and is recommended to be underlain by a minimum 4-inch-thick aggregate base course. It is recommended that concrete pavement have load-transfer reinforcement, where appropriate. Control-joint spacing should be determined in accordance with the current ACI code. Expansion joints should be provided where pavement abuts fixed objects, such as the building and light poles. The recommended PCC pavement thickness assumes that the 28-day compressive strength of the concrete will be at least 4,000 psi. The concrete is recommended to be properly air-entrained for durability. It is recommended and assumed that a civil engineer will provide specific recommendations for concrete pavement, including reinforcing details and control-joint spacing. Materials and construction procedures for concrete pavement and the aggregate base are recommended to be in accordance with Wisconsin DOT specifications.

General Pavement Considerations

The pavement recommendations assume that the pavement subgrade will be prepared according to this report, the base course will be properly drained, and a geotechnical engineer will observe and test pavement construction. Pavement was designed based on AASHTO design parameters for a twenty-year design period, but the actual service could be less. Local codes may require specific testing to determine soil support characteristics, and a minimum pavement section might be required.

### 8.5. Preliminary Stormwater Infiltration Screening

It is understood that three stormwater management areas are planned to be constructed in the southwestern and southeastern portions of the site, in the areas of Test Pits TP1 through TP-3; the approximate locations of the test pits are shown on the *Test Boring and Test Pit Location Plan*. Details of the proposed stormwater management devices were not provided. This report assumes that the devices bottoms will be between 5 and 8 feet deep.

Based on the soil conditions at the test pits, infiltration of stormwater is feasible. Based on the textural correlation for design infiltration rates provided in Table 2 of Wisconsin DNR 1002 and the soil encountered at the test pits, a design infiltration rate 1.63 inch/hour may be achieved for the loamy sand site soils. The actual infiltration rate will depend on the in-place density, or compactness, of native soil and replacement media (if needed). Soil classifications at the test pit locations and the correlated infiltration rates are shown on the *Soil and Site Evaluation – Stormwater Infiltration* logs in Appendix A.

### 8.6. Generalized Site Preparation Recommendations

This section provides recommendations for the preparation of the proposed building, pavement, and engineered fill areas. The means and methods of site preparation will greatly depend on the weather conditions before and during construction, the subsurface conditions that are exposed during earthwork operations, and the finalized details of the proposed development. Therefore,

only general site preparation recommendations are given. In addition to being general, the following site preparation recommendations are abbreviated; the *Guide Specifications* in Appendix D gives further recommendations. The *Guide Specifications* should be read along with this section. Also, the *Guide Specifications* are recommended to be used as an aid to develop the project specifications.

Demolition and Removal

The existing building is recommended to be removed from the development area and to at least five feet beyond the proposed development area. Disposal of rubble and debris is recommended to be in accordance with local, state, and federal regulations for the material type. Outside the proposed building area, it might be feasible for existing foundations to remain, provided the foundations are stable, are cut off at least 3 feet below the planned subgrade, and hollow cores are grouted solid. Existing floor slabs that are sufficiently outside the proposed building area could possibly also stay in-place, provided that the slabs are at least 3 feet below the planned finished grade, are perforated (broken) on a maximum 2-foot grid, are “seated” into the subgrade for stability, and are covered with a minimum 12-inch-thick layer of well-graded free-draining granular material for drainage. It is important to note that building remnants that are left in-place could cause excavation difficulties for new utilities and landscape plantings and for future construction. Building remnants might also be susceptible to frost heave. Excavations created during removal of the existing building must be backfilled with engineered fill, which might need to be benched into the surrounding soil, as noted in Item No. 3 of the *Guide Specifications* enclosed in Appendix D.

Existing pavement, surface vegetation, trees and bushes (including root-balls), topsoil, and other unsuitable materials are recommended to be removed from the proposed building, pavement, and engineered fill areas. Where feasible, stripping should extend at least several feet beyond the development area. Existing pavement should remain in-place as long as practical to protect the underlying soil.

Proof-Rolling and Fill Placement

After the recommended demolition and removal operations, and once the site is cut (lowered) as needed, the subgrade within the proposed development area is recommended to be proof-rolled with a fully-loaded tandem-axle dump truck to help locate unstable soil based on subgrade deflection caused by the wheel loads of the proof-roll equipment. The proposed development area is recommended to be thoroughly proof-rolled except that, for safety, proof-roll equipment must be kept a sufficient distance from excavations. Where feasible, proof-rolling should extend at least several feet beyond the limits of the proposed development area. It is recommended that a geotechnical engineer observe the proof-roll operations and evaluate the stability of the subgrade based on these observations. Areas that are not accessible to proof-roll equipment are recommended to be evaluated and approved by a geotechnical engineer using appropriate means and methods.

Unsuitable granular soil that is identified during proof-rolling and testing can likely be improved by scarification and moisture-conditioning (uniformly moistening or drying) followed by compaction using appropriate compaction equipment. Also, unsuitable soil could be replaced with engineered fill; however, engineered fill material is recommended to be approved by a geotechnical engineer before it is placed. Also, recommendations for subgrade improvement should be made by a geotechnical engineer based on the site conditions during construction. Areas requiring subgrade improvement should be defined during construction with the assistance of a geotechnical engineer. Specific improvement methods should be determined during construction on an area-by-area basis.

The proposed development area is recommended to be raised, where necessary, to the planned finished grade with engineered fill immediately after the subgrade is confirmed to be stable and suitable to support the proposed site improvements. Engineered fill is recommended to be placed in relatively thin layers (lifts) that are uniform in elevation. Each layer of engineered fill is recommended to be compacted to at least 95 percent of the fill material's maximum dry density determined from the Standard Proctor compaction test (ASTM D698). As an exception, the in-place dry density of engineered fill within one foot of the pavement subgrade is recommended to be compacted to at least 100 percent of the fill material's maximum dry density. The water content of fill material is recommended to be uniform and within a narrow range of the optimum moisture content, also determined by the Standard Proctor compaction test. Item Nos. 4 and 5 of the *Guide Specifications* give more information pertaining to selection and compaction of engineered fill.

Engineered fill that does not meet the density and water content requirements is recommended to be replaced, or possibly it could be scarified to a sufficient depth (likely 6 to 12 inches, or more), moisture-conditioned, and compacted to the required density. A subsequent lift of fill should only be placed after a geotechnical engineer confirms that the previous lift was properly placed and compacted. Subgrade soil might need to be recompacted immediately before construction since equipment traffic and adverse weather may reduce soil stability.

Use of Site Soil as Engineered Fill

Site soil that does not contain adverse organic content or other deleterious materials, as noted in the *Guide Specifications,* could be used as engineered fill. Site soil will likely need to be moisture conditioned (uniformly moistened or dried) before it is used as engineered fill. If construction is during adverse weather (discussed in the following section), drying site soil will likely not be feasible. In that case, aggregate fill (or other fill material with a low water-sensitivity) will likely need to be imported to the site. Additional recommendations regarding fill selection, placement, and compaction are given in the *Guide Specifications*.

## 8.7. Generalized Construction Considerations

### Adverse Weather

Site soil is moisture sensitive and will likely become unstable when exposed to adverse weather, such as rain, snow, and freezing temperatures. Therefore, it might be necessary to remove or stabilize the upper 6 to 12 inches (or more) of soil due to adverse weather, which commonly occurs during late fall, winter, and early spring. At least some over-excavation or stabilization of unstable soil should be expected if construction is during or after adverse weather. Because site preparation is weather dependent, bids for site preparation and other earthwork activities should consider the time of year that construction will be conducted.

To protect soil from adverse weather, the site is recommended to be smoothly graded and contoured during construction to divert surface water from construction areas and excavations. Contoured subgrades are recommended to be rolled with a smooth-drum compactor before precipitation to “seal” the surface. Furthermore, construction traffic should be restricted to certain aggregate-covered areas to control traffic-related soil disturbance. Foundation, floor slab, and pavement construction should begin immediately after suitable support is confirmed.

### Dewatering

If water collects in shallow excavations due to precipitation, the water is expected to permeate into on-site sand relatively quick. If needed, filtered sump pumps, drawing water from sump pits excavated in the bottom of construction trenches, could be used to remove water that collects in excavations due to precipitation or runoff. Excavated sump pits should be fully-lined with geotextile and filled with open-graded, free-draining aggregate.

### Excavation Stability

Excavations are recommended to be made in accordance with current OSHA excavation and trench safety standards and other applicable requirements. Excavation walls are recommended to be sloped, benched, or braced to develop and maintain a safe work environment. Temporary shoring must be designed according to applicable regulatory requirements. Contractors are responsible for excavation safety.

### Existing Utilities

All existing utilities are recommended to be identified and located, and any planned to be maintained should be relocated outside the proposed addition area. Utilities that are not reused should be capped and removed in accordance with local codes and ordinances. Excavations for the removal of utilities are recommended to be backfilled with engineered fill placed under engineering-controlled conditions. Grading operations must be done carefully so that existing

utilities are not damaged or disturbed. Utility locations, elevations, and types should be checked relative to the planned construction to identify any concerns.

### 8.8. Recommended Construction Materials Testing Services

This report was prepared assuming that a geotechnical engineer will perform Construction Materials Testing ("CMT") services during construction of the proposed development. Supplemental geotechnical recommendations may be needed based on the results of CMT services and specific details of the project not known at this time.

## 9.0 BASIS OF REPORT

This report is strictly based on the project description given in Section 3.0. Giles must be notified if the project description or our assumptions about the project are not accurate so that this report can be amended, if needed. This report assumes that the proposed improvements will be designed and constructed according to the codes that govern construction at the site.

The conclusions and recommendations in this report are based on the estimated subsurface conditions shown on the *Test Boring Logs*. Giles must be notified if the subsurface conditions that are encountered during construction of the proposed development differ from those shown on the *Test Boring Logs*; revision of this report might be necessary. General comments and limitations of this report are given in the appendix.

The conclusions and recommendations in this report have been promulgated in accordance with generally accepted professional engineering practices in the field of geotechnical engineering. No other warranty is either expressed or implied.

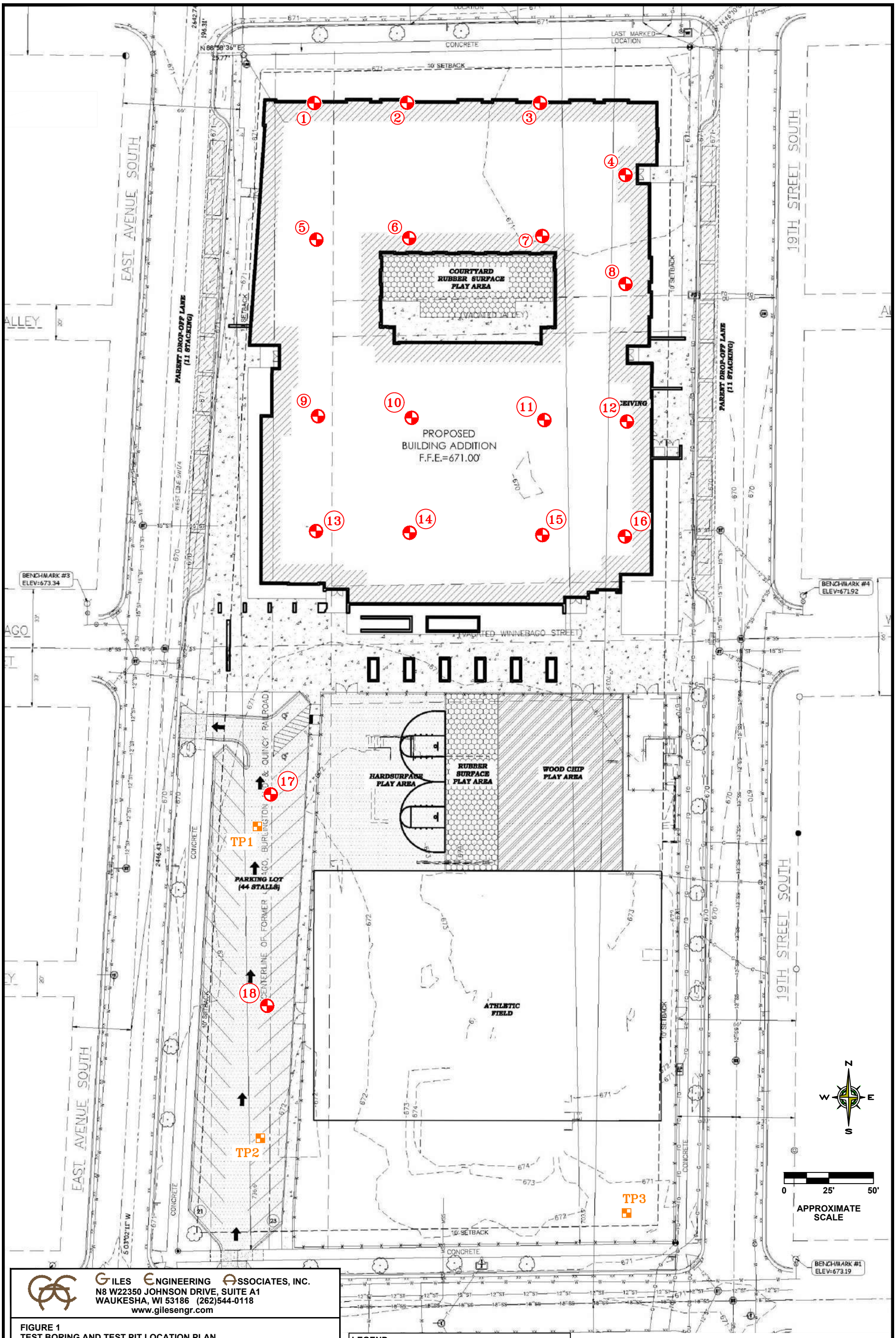
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# APPENDIX A

## FIGURES AND TEST BORING LOGS

The Test Boring Location Plan contained herein was prepared based upon information supplied by *Giles'* client, or others, along with *Giles'* field measurements and observations. The diagram is presented for conceptual purposes only and is intended to assist the reader in report interpretation.

The Test Boring Logs and related information enclosed herein depict the subsurface (soil and water) conditions encountered at the specific boring locations on the date that the exploration was performed. Subsurface conditions may differ between boring locations and within areas of the site that were not explored with test borings. The subsurface conditions may also change at the boring locations over the passage of time.




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**FIGURE 1**
**TEST BORING AND TEST PIT LOCATION PLAN**
**PROPOSED ELEMENTARY SCHOOL**
**807 EAST AVENUE SOUTH**
**La CROSSE, WISCONSIN**

| DESIGNED | DRAWN | SCALE | DATE | REVISED |
|---|---|---|---|---|
| GCH | GCH | approx. 1"=50' | 05-09-25 | -- |
| PROJECT NO.: 1G-2502021-3 | | | CAD No. 1g2502021-3blp2 | |

**LEGEND:**

TP1 — TEST PIT LOCATIONS

① — GEOTECHNICAL TEST BORING

**NOTES:**

1.) TEST BORING AND TEST PIT LOCATIONS ARE APPROXIMATE.

2.) BASE MAP DEVELOPED FROM THE "LAYOUT PLAN" (SHEET C2.0), DATED 3-14-2025, PREPARED BY BRAY ARCHITECTS.

| BORING NO. & LOCATION: 1 | TEST BORING LOG | GILES ENGINEERING ASSOCIATES, INC. |
|---|---|---|
| SURFACE ELEVATION: 670.6 feet | PROPOSED ELEMENTARY SCHOOL | |
| COMPLETION DATE: 04/24/25 | 807 EAST AVENUE SOUTH<br>LA CROSSE, WISCONSIN | |
| FIELD REP: DAVIS LUCKETT | PROJECT NO: 1G-2502021-3 | |

| MATERIAL DESCRIPTION | Depth (ft) | Elevation | Sample No. & Type | N | $Q_u$ (tsf) | $Q_p$ (tsf) | $Q_s$ (tsf) | W (%) | PID | NOTES |
|---|---|---|---|---|---|---|---|---|---|---|
| **±6" Topsoil:** Dark Brown Sandy Silt, little Organic Matter-Moist | | 670 | 1-SS | 5 | | | | | | |
| **Fill:** Dark Brown fine to medium Sand-Moist | | | 2-SS | 7 | | | | | | |
| | 5 | 665 | 3-SS | 6 | | | | | | |
| Brown fine to medium Sand-Moist | | | 4-SS | 7 | | | | | | |
| | 10 | 660 | 5-SS | 13 | | | | | | |
| | 15 | 655 | 6-SS | 14 | | | | | | |
| | 20 | 650 | 7-SS | 7 | | | | | | |
| | 25 | 645 | 8-SS | 25 | | | | | | |
| Boring Terminated at about 26 feet (EL. 644.6') | | | | | | | | | | |

| Water Observation Data | Remarks: |
|---|---|
| Water Encountered During Drilling: | |
| Water Level At End of Drilling: | |
| Cave Depth At End of Drilling: | |
| Water Level After Drilling: | |
| Cave Depth After Drilling: | |

GILES LOG REPORT 1G2502021-3.GPJ GILES.GDT 5/5/25

Changes in strata indicated by the lines are approximate boundary between soil types. The actual transition may be gradual and may vary considerably between test borings. Location of test boring is shown on the Boring Location Plan.

| BORING NO. & LOCATION: 2 | TEST BORING LOG | GILES ENGINEERING ASSOCIATES, INC. |
|---|---|---|
| SURFACE ELEVATION: 670.5 feet | PROPOSED ELEMENTARY SCHOOL | |
| COMPLETION DATE: 04/24/25 | 807 EAST AVENUE SOUTH<br>LA CROSSE, WISCONSIN | |
| FIELD REP: DAVIS LUCKETT | PROJECT NO: 1G-2502021-3 | |

| MATERIAL DESCRIPTION | Depth (ft) | Elevation | Sample No. & Type | N | $Q_u$ (tsf) | $Q_p$ (tsf) | $Q_s$ (tsf) | W (%) | PID | NOTES |
|---|---|---|---|---|---|---|---|---|---|---|
| **±6" Topsoil:** Dark Brown Silty fine Sand, trace Organic Matter-Moist | | 670 | 1-SS | 6 | | | | | | |
| **Fill:** Dark Brown fine to medium Sand, trace to little Silt, trace Gravel-Moist | | | 2-SS | 8 | | | | | | |
| | 5 | 665 | 3-SS | 8 | | | | | | |
| Brown fine to medium Sand-Moist | | | 4-SS | 11 | | | | | | |
| | 10 | 660 | 5-SS | 13 | | | | | | |
| | 15 | 655 | 6-SS | 11 | | | | | | |
| | 20 | 650 | 7-SS | 14 | | | | | | |
| | 25 | 645 | 8-SS | 21 | | | | | | |
| Boring Terminated at about 26 feet (EL. 644.5') | | | | | | | | | | |

| Water Observation Data | Remarks: |
|---|---|
| Water Encountered During Drilling: | |
| Water Level At End of Drilling: | |
| Cave Depth At End of Drilling: | |
| Water Level After Drilling: | |
| Cave Depth After Drilling: | |

GILES LOG REPORT 1G2502021-3.GPJ GILES.GDT 5/5/25

Changes in strata indicated by the lines are approximate boundary between soil types. The actual transition may be gradual and may vary considerably between test borings. Location of test boring is shown on the Boring Location Plan.

| BORING NO. & LOCATION: 3 | TEST BORING LOG | GILES ENGINEERING ASSOCIATES, INC. |
|---|---|---|
| SURFACE ELEVATION: 671.1 feet | PROPOSED ELEMENTARY SCHOOL | |
| COMPLETION DATE: 04/23/25 | 807 EAST AVENUE SOUTH<br>LA CROSSE, WISCONSIN | |
| FIELD REP: DAVIS LUCKETT | PROJECT NO: 1G-2502021-3 | |

| MATERIAL DESCRIPTION | Depth (ft) | Elevation | Sample No. & Type | N | $Q_u$ (tsf) | $Q_p$ (tsf) | $Q_s$ (tsf) | W (%) | PID | NOTES |
|---|---|---|---|---|---|---|---|---|---|---|
| **Fill:** Brown Sandy Silt-Moist | | | | | | | | | | |
| **Fill:** Dark Brown fine to medium Sand, little Silt-Moist | | 670 | 1-SS | 8 | | | | | | |
| | | | 2-SS | 8 | | | | | | |
| | 5 | | 3-SS | 9 | | | | | | |
| | | 665 | | | | | | | | |
| | | | 4-SS | 12 | | | | | | |
| Brown fine to medium Sand-Moist | 10 | | 5-SS | 13 | | | | | | |
| | | 660 | | | | | | | | |
| | 15 | | 6-SS | 13 | | | | | | |
| | | 655 | | | | | | | | |
| | 20 | | 7-SS | 15 | | | | | | |
| | | 650 | | | | | | | | |
| | 25 | | 8-SS | 27 | | | | | | |
| Boring Terminated at about 26 feet (EL. 645.1') | | | | | | | | | | |

| Water Observation Data | Remarks: |
|---|---|
| Water Encountered During Drilling: | |
| Water Level At End of Drilling: | |
| Cave Depth At End of Drilling: | |
| Water Level After Drilling: | |
| Cave Depth After Drilling: | |

GILES LOG REPORT 1G2502021-3.GPJ GILES.GDT 5/5/25

Changes in strata indicated by the lines are approximate boundary between soil types. The actual transition may be gradual and may vary considerably between test borings. Location of test boring is shown on the Boring Location Plan.

| BORING NO. & LOCATION: 4 | TEST BORING LOG | GILES ENGINEERING ASSOCIATES, INC. |
|---|---|---|
| SURFACE ELEVATION: 671.1 feet | PROPOSED ELEMENTARY SCHOOL | |
| COMPLETION DATE: 04/23/25 | 807 EAST AVENUE SOUTH<br>LA CROSSE, WISCONSIN | |
| FIELD REP: DAVIS LUCKETT | PROJECT NO: 1G-2502021-3 | |

| MATERIAL DESCRIPTION | Depth (ft) | Elevation | Sample No. & Type | N | $Q_u$ (tsf) | $Q_p$ (tsf) | $Q_s$ (tsf) | W (%) | PID | NOTES |
|---|---|---|---|---|---|---|---|---|---|---|
| **Fill:** Brown Sandy Silt, little Gravel-Moist | | | | | | | | | | |
| **Fill:** Dark Brown fine to medium Sand, little Silt-Moist | | 670 | 1-SS | 6 | | | | | | |
| | | | 2-SS | 8 | | | | | | |
| | 5 | | 3-SS | 6 | | | | | | |
| | | 665 | | | | | | | | |
| | | | 4-SS | 8 | | | | | | |
| Brown fine to medium Sand-Moist | 10 | | 5-SS | 12 | | | | | | |
| | | 660 | | | | | | | | |
| | 15 | | 6-SS | 11 | | | | | | |
| | | 655 | | | | | | | | |
| | 20 | | 7-SS | 13 | | | | | | |
| | | 650 | | | | | | | | |
| | 25 | | 8-SS | 23 | | | | | | |
| Boring Terminated at about 26 feet (EL. 645.1') | | | | | | | | | | |

| Water Observation Data | Remarks: |
|---|---|
| Water Encountered During Drilling: | |
| Water Level At End of Drilling: | |
| Cave Depth At End of Drilling: | |
| Water Level After Drilling: | |
| Cave Depth After Drilling: | |

GILES LOG REPORT 1G2502021-3.GPJ GILES.GDT 5/5/25

Changes in strata indicated by the lines are approximate boundary between soil types. The actual transition may be gradual and may vary considerably between test borings. Location of test boring is shown on the Boring Location Plan.

| BORING NO. & LOCATION: 5 | TEST BORING LOG | GILES ENGINEERING ASSOCIATES, INC. |
|---|---|---|
| SURFACE ELEVATION: 670.5 feet | PROPOSED ELEMENTARY SCHOOL | |
| COMPLETION DATE: 04/24/25 | 807 EAST AVENUE SOUTH<br>LA CROSSE, WISCONSIN | |
| FIELD REP: DAVIS LUCKETT | PROJECT NO: 1G-2502021-3 | |

| MATERIAL DESCRIPTION | Depth (ft) | Elevation | Sample No. & Type | N | $Q_u$ (tsf) | $Q_p$ (tsf) | $Q_s$ (tsf) | W (%) | PID | NOTES |
|---|---|---|---|---|---|---|---|---|---|---|
| **±6" Topsoil:** Dark Brown Sandy Silt, little Organic Matter-Moist | | 670 | 1-SS | 7 | | | | | | |
| **Fill:** Dark Brown fine to medium Sand-Moist | | | 2-SS | 7 | | | | | | |
| | 5 | 665 | 3-SS | 8 | | | | | | |
| Brown fine to medium Sand-Moist | | | 4-SS | 9 | | | | | | |
| | 10 | 660 | 5-SS | 10 | | | | | | |
| | 15 | 655 | 6-SS | 11 | | | | | | |
| | 20 | 650 | 7-SS | 10 | | | | | | |
| | 25 | 645 | 8-SS | 26 | | | | | | |
| Boring Terminated at about 26 feet (EL. 644.5') | | | | | | | | | | |

| Water Observation Data | Remarks: |
|---|---|
| Water Encountered During Drilling: | |
| Water Level At End of Drilling: | |
| Cave Depth At End of Drilling: | |
| Water Level After Drilling: | |
| Cave Depth After Drilling: | |

GILES LOG REPORT 1G2502021-3.GPJ GILES.GDT 5/5/25

Changes in strata indicated by the lines are approximate boundary between soil types. The actual transition may be gradual and may vary considerably between test borings. Location of test boring is shown on the Boring Location Plan.

| BORING NO. & LOCATION: 6 | TEST BORING LOG | GILES ENGINEERING ASSOCIATES, INC. |
|---|---|---|
| SURFACE ELEVATION: 670.5 feet | PROPOSED ELEMENTARY SCHOOL | |
| COMPLETION DATE: 04/24/25 | 807 EAST AVENUE SOUTH<br>LA CROSSE, WISCONSIN | |
| FIELD REP: DAVIS LUCKETT | PROJECT NO: 1G-2502021-3 | |

| MATERIAL DESCRIPTION | Depth (ft) | Elevation | Sample No. & Type | N | $Q_u$ (tsf) | $Q_p$ (tsf) | $Q_s$ (tsf) | W (%) | PID | NOTES |
|---|---|---|---|---|---|---|---|---|---|---|
| **±5" Topsoil:** Dark Brown Sandy Silt, little Organic Matter-Moist | | 670 | 1-SS | 7 | | | | | | |
| **Fill:** Dark Brown fine to medium Sand, little Silt, trace Gravel (Includes Asphalt Rubble)-Moist | | | 2-SS | 8 | | | | | | |
| | 5 | 665 | 3-SS | 11 | | | | | | |
| Brown fine to medium Sand-Moist | | | 4-SS | 10 | | | | | | |
| | 10 | 660 | 5-SS | 11 | | | | | | |
| | 15 | 655 | 6-SS | 11 | | | | | | |
| | 20 | 650 | 7-SS | 13 | | | | | | |
| | 25 | 645 | 8-SS | 19 | | | | | | |
| Boring Terminated at about 26 feet (EL. 644.5') | | | | | | | | | | |

| Water Observation Data | Remarks: |
|---|---|
| Water Encountered During Drilling: | |
| Water Level At End of Drilling: | |
| Cave Depth At End of Drilling: | |
| Water Level After Drilling: | |
| Cave Depth After Drilling: | |

GILES LOG REPORT 1G2502021-3.GPJ GILES.GDT 5/5/25

Changes in strata indicated by the lines are approximate boundary between soil types. The actual transition may be gradual and may vary considerably between test borings. Location of test boring is shown on the Boring Location Plan.

| BORING NO. & LOCATION: 7 | TEST BORING LOG | GILES ENGINEERING ASSOCIATES, INC. |
|---|---|---|
| SURFACE ELEVATION: 671 feet | PROPOSED ELEMENTARY SCHOOL | |
| COMPLETION DATE: 04/23/25 | 807 EAST AVENUE SOUTH<br>LA CROSSE, WISCONSIN | |
| FIELD REP: DAVIS LUCKETT | PROJECT NO: 1G-2502021-3 | |

| MATERIAL DESCRIPTION | Depth (ft) | Elevation | Sample No. & Type | N | $Q_u$ (tsf) | $Q_p$ (tsf) | $Q_s$ (tsf) | W (%) | PID | NOTES |
|---|---|---|---|---|---|---|---|---|---|---|
| **±10" Topsoil:** Dark Brown Sandy Silt, little Organic Matter-Moist | | 670 | 1-SS | 5 | | | | | | |
| **Fill:** Dark Brown fine to medium Sand, little Silt-Moist | | | 2-SS | 7 | | | | | | |
| | 5 | 665 | 3-SS | 8 | | | | | | |
| | | | 4-SS | 13 | | | | | | |
| Brown fine to medium Sand-Moist | 10 | 660 | 5-SS | 9 | | | | | | |
| | 15 | 655 | 6-SS | 13 | | | | | | |
| | 20 | 650 | 7-SS | 14 | | | | | | |
| | 25 | 645 | 8-SS | 25 | | | | | | |
| Boring Terminated at about 26 feet (EL. 645') | | | | | | | | | | |

| Water Observation Data | Remarks: |
|---|---|
| Water Encountered During Drilling: | |
| Water Level At End of Drilling: | |
| Cave Depth At End of Drilling: | |
| Water Level After Drilling: | |
| Cave Depth After Drilling: | |

GILES LOG REPORT 1G2502021-3.GPJ GILES.GDT 5/5/25

Changes in strata indicated by the lines are approximate boundary between soil types. The actual transition may be gradual and may vary considerably between test borings. Location of test boring is shown on the Boring Location Plan.

| BORING NO. & LOCATION: 8 | TEST BORING LOG | GILES ENGINEERING ASSOCIATES, INC. |
|---|---|---|
| SURFACE ELEVATION: 670.7 feet | PROPOSED ELEMENTARY SCHOOL | |
| COMPLETION DATE: 04/23/25 | 807 EAST AVENUE SOUTH<br>LA CROSSE, WISCONSIN | |
| FIELD REP: DAVIS LUCKETT | PROJECT NO: 1G-2502021-3 | |

| MATERIAL DESCRIPTION | Depth (ft) | Elevation | Sample No. & Type | N | $Q_u$ (tsf) | $Q_p$ (tsf) | $Q_s$ (tsf) | W (%) | PID | NOTES |
|---|---|---|---|---|---|---|---|---|---|---|
| **±8" Topsoil:** Dark Brown Sandy Silt, little Organic Matter-Moist | | 670 | 1-SS | 6 | | | | | | |
| **Fill:** Dark Brown fine to medium Sand, little Silt-Moist | | | 2-SS | 7 | | | | | | |
| | 5 | 665 | 3-SS | 10 | | | | | | |
| | | | 4-SS | 8 | | | | | | |
| Brown fine to medium Sand-Moist | 10 | 660 | 5-SS | 11 | | | | | | |
| | 15 | 655 | 6-SS | 9 | | | | | | |
| | 20 | 650 | 7-SS | 11 | | | | | | |
| | 25 | 645 | 8-SS | 23 | | | | | | |
| Boring Terminated at about 26 feet (EL. 644.7') | | | | | | | | | | |

| Water Observation Data | Remarks: |
|---|---|
| Water Encountered During Drilling: | |
| Water Level At End of Drilling: | |
| Cave Depth At End of Drilling: | |
| Water Level After Drilling: | |
| Cave Depth After Drilling: | |

GILES LOG REPORT 1G2502021-3.GPJ GILES.GDT 5/5/25

Changes in strata indicated by the lines are approximate boundary between soil types. The actual transition may be gradual and may vary considerably between test borings. Location of test boring is shown on the Boring Location Plan.

| BORING NO. & LOCATION: 9 | TEST BORING LOG | GILES ENGINEERING ASSOCIATES, INC. |
|---|---|---|
| SURFACE ELEVATION: 670.3 feet | PROPOSED ELEMENTARY SCHOOL | |
| COMPLETION DATE: 04/24/25 | 807 EAST AVENUE SOUTH<br>LA CROSSE, WISCONSIN | |
| FIELD REP: DAVIS LUCKETT | PROJECT NO: 1G-2502021-3 | |

| MATERIAL DESCRIPTION | Depth (ft) | Elevation | Sample No. & Type | N | $Q_u$ (tsf) | $Q_p$ (tsf) | $Q_s$ (tsf) | W (%) | PID | NOTES |
|---|---|---|---|---|---|---|---|---|---|---|
| **Fill:** Brown Silty fine to medium Sand, trace Gravel-Moist | 0 | 670 | 1-SS | 7 | | | | | | |
| **Fill:** Dark Brown fine to medium Sand, little Silt-Moist | | | 2-SS | 5 | | | | | | |
| | 5 | 665 | 3-SS | 6 | | | | | | |
| Brown fine to medium Sand-Moist | | | 4-SS | 11 | | | | | | |
| | 10 | 660 | 5-SS | 14 | | | | | | |
| | 15 | 655 | 6-SS | 13 | | | | | | |
| | 20 | 650 | 7-SS | 12 | | | | | | |
| | 25 | 645 | 8-SS | 21 | | | | | | |
| Boring Terminated at about 26 feet (EL. 644.3') | | | | | | | | | | |

| Water Observation Data | Remarks: |
|---|---|
| Water Encountered During Drilling: | |
| Water Level At End of Drilling: | |
| Cave Depth At End of Drilling: | |
| Water Level After Drilling: | |
| Cave Depth After Drilling: | |

GILES LOG REPORT 1G2502021-3.GPJ GILES.GDT 5/5/25

Changes in strata indicated by the lines are approximate boundary between soil types. The actual transition may be gradual and may vary considerably between test borings. Location of test boring is shown on the Boring Location Plan.

| BORING NO. & LOCATION: 10 | TEST BORING LOG | GILES ENGINEERING ASSOCIATES, INC. |
|---|---|---|
| SURFACE ELEVATION: 670.5 feet | PROPOSED ELEMENTARY SCHOOL | |
| COMPLETION DATE: 04/24/25 | 807 EAST AVENUE SOUTH<br>LA CROSSE, WISCONSIN | |
| FIELD REP: DAVIS LUCKETT | PROJECT NO: 1G-2502021-3 | |

| MATERIAL DESCRIPTION | Depth (ft) | Elevation | Sample No. & Type | N | $Q_u$ (tsf) | $Q_p$ (tsf) | $Q_s$ (tsf) | W (%) | PID | NOTES |
|---|---|---|---|---|---|---|---|---|---|---|
| **Fill:** Brown Sandy Silt, little Gravel-Moist | | 670 | 1-SS | 8 | | | | | | |
| **Fill:** Dark Brown fine to medium Sand, little Silt, trace Gravel-Moist | | | 2-SS | 7 | | | | | | |
| | 5 | 665 | 3-SS | 16 | | | | | | |
| Brown fine to medium Sand-Moist | | | 4-SS | 11 | | | | | | |
| | 10 | 660 | 5-SS | 12 | | | | | | |
| | 15 | 655 | 6-SS | 12 | | | | | | |
| | 20 | 650 | 7-SS | 14 | | | | | | |
| | 25 | 645 | 8-SS | 24 | | | | | | |
| Boring Terminated at about 26 feet (EL. 644.5') | | | | | | | | | | |

| | Water Observation Data | Remarks: |
|---|---|---|
| ▽ | Water Encountered During Drilling: | |
| ▼ | Water Level At End of Drilling: | |
| | Cave Depth At End of Drilling: | |
| ▼ | Water Level After Drilling: | |
| | Cave Depth After Drilling: | |

GILES LOG REPORT 1G2502021-3.GPJ GILES.GDT 5/5/25

Changes in strata indicated by the lines are approximate boundary between soil types. The actual transition may be gradual and may vary considerably between test borings. Location of test boring is shown on the Boring Location Plan.

| BORING NO. & LOCATION: 11 | TEST BORING LOG | GILES ENGINEERING ASSOCIATES, INC. |
|---|---|---|
| SURFACE ELEVATION: 670.6 feet | PROPOSED ELEMENTARY SCHOOL | |
| COMPLETION DATE: 04/24/25 | 807 EAST AVENUE SOUTH<br>LA CROSSE, WISCONSIN | |
| FIELD REP: DAVIS LUCKETT | PROJECT NO: 1G-2502021-3 | |

| MATERIAL DESCRIPTION | Depth (ft) | Elevation | Sample No. & Type | N | $Q_u$ (tsf) | $Q_p$ (tsf) | $Q_s$ (tsf) | W (%) | PID | NOTES |
|---|---|---|---|---|---|---|---|---|---|---|
| **±5" Topsoil:** Dark Brown Silty fine Sand, trace Organic Matter-Moist | | 670 | 1-SS | 6 | | | | | | |
| **Fill:** Dark Brown fine to medium Sand, trace Gravel, trace to little Silt-Moist | | | 2-SS | 4 | | | | | | |
| | 5 | 665 | 3-SS | 5 | | | | | | |
| Brown fine to medium Sand, trace Gravel-Moist | | | 4-SS | 8 | | | | | | |
| | 10 | 660 | 5-SS | 8 | | | | | | |
| | 15 | 655 | 6-SS | 8 | | | | | | |
| | 20 | 650 | 7-SS | 10 | | | | | | |
| | 25 | 645 | 8-SS | 13 | | | | | | |
| Boring Terminated at about 26 feet (EL. 644.6') | | | | | | | | | | |

| Water Observation Data | Remarks: |
|---|---|
| Water Encountered During Drilling: | |
| Water Level At End of Drilling: | |
| Cave Depth At End of Drilling: | |
| Water Level After Drilling: | |
| Cave Depth After Drilling: | |

GILES LOG REPORT 1G2502021-3.GPJ GILES.GDT 5/5/25

Changes in strata indicated by the lines are approximate boundary between soil types. The actual transition may be gradual and may vary considerably between test borings. Location of test boring is shown on the Boring Location Plan.

| BORING NO. & LOCATION: 12 | TEST BORING LOG | GILES ENGINEERING ASSOCIATES, INC. |
|---|---|---|
| SURFACE ELEVATION: 670.5 feet | PROPOSED ELEMENTARY SCHOOL | |
| COMPLETION DATE: 04/23/25 | 807 EAST AVENUE SOUTH<br>LA CROSSE, WISCONSIN | |
| FIELD REP: DAVIS LUCKETT | PROJECT NO: 1G-2502021-3 | |

| MATERIAL DESCRIPTION | Depth (ft) | Elevation | Sample No. & Type | N | $Q_u$ (tsf) | $Q_p$ (tsf) | $Q_s$ (tsf) | W (%) | PID | NOTES |
|---|---|---|---|---|---|---|---|---|---|---|
| **±6" Topsoil:** Dark Brown Sandy Silt, trace Organic Matter-Moist | 0 | 670 | 1-SS | 6 | | | | | | |
| **Fill:** Dark Brown fine to medium Sand, little Silt, trace Gravel-Moist | | | 2-SS | 9 | | | | | | |
| | 5 | 665 | 3-SS | 10 | | | | | | |
| | | | 4-SS | 11 | | | | | | |
| Brown fine to medium Sand-Moist | 10 | 660 | 5-SS | 10 | | | | | | |
| | 15 | 655 | 6-SS | 9 | | | | | | |
| | 20 | 650 | 7-SS | 12 | | | | | | |
| | 25 | 645 | 8-SS | 16 | | | | | | |
| Boring Terminated at about 26 feet (EL. 644.5') | | | | | | | | | | |

| Water Observation Data | Remarks: |
|---|---|
| Water Encountered During Drilling: | |
| Water Level At End of Drilling: | |
| Cave Depth At End of Drilling: | |
| Water Level After Drilling: | |
| Cave Depth After Drilling: | |

GILES LOG REPORT 1G2502021-3.GPJ GILES.GDT 5/5/25

Changes in strata indicated by the lines are approximate boundary between soil types. The actual transition may be gradual and may vary considerably between test borings. Location of test boring is shown on the Boring Location Plan.

| BORING NO. & LOCATION: 13 | TEST BORING LOG | GILES ENGINEERING ASSOCIATES, INC. |
|---|---|---|
| SURFACE ELEVATION: 670.3 feet | PROPOSED ELEMENTARY SCHOOL | |
| COMPLETION DATE: 04/24/25 | 807 EAST AVENUE SOUTH<br>LA CROSSE, WISCONSIN | |
| FIELD REP: DAVIS LUCKETT | PROJECT NO: 1G-2502021-3 | |

| MATERIAL DESCRIPTION | Depth (ft) | Elevation | Sample No. & Type | N | $Q_u$ (tsf) | $Q_p$ (tsf) | $Q_s$ (tsf) | W (%) | PID | NOTES |
|---|---|---|---|---|---|---|---|---|---|---|
| **±6" Topsoil:** Dark Brown Sandy Silt, trace Organic Matter-Moist | | 670 | 1-SS | 5 | | | | | | |
| **Fill:** Dark Brown fine to medium Sand-Moist | | | 2-SS | 4 | | | | | | |
| | 5 | 665 | 3-SS | 6 | | | | | | |
| Brown fine to medium Sand, little Silt-Moist | | | 4-SS | 8 | | | | | | |
| Brown fine to medium Sand-Moist | 10 | 660 | 5-SS | 9 | | | | | | |
| | 15 | 655 | 6-SS | 14 | | | | | | |
| | 20 | 650 | 7-SS | 7 | | | | | | |
| | 25 | 645 | 8-SS | 20 | | | | | | |
| Boring Terminated at about 26 feet (EL. 644.3') | | | | | | | | | | |

| Water Observation Data | Remarks: |
|---|---|
| Water Encountered During Drilling: | |
| Water Level At End of Drilling: | |
| Cave Depth At End of Drilling: | |
| Water Level After Drilling: | |
| Cave Depth After Drilling: | |

GILES LOG REPORT 1G2502021-3.GPJ GILES.GDT 5/5/25

Changes in strata indicated by the lines are approximate boundary between soil types. The actual transition may be gradual and may vary considerably between test borings. Location of test boring is shown on the Boring Location Plan.

| BORING NO. & LOCATION: 14 | TEST BORING LOG | GILES ENGINEERING ASSOCIATES, INC. |
|---|---|---|
| SURFACE ELEVATION: 670.4 feet | PROPOSED ELEMENTARY SCHOOL | |
| COMPLETION DATE: 04/24/25 | 807 EAST AVENUE SOUTH<br>LA CROSSE, WISCONSIN | |
| FIELD REP: DAVIS LUCKETT | PROJECT NO: 1G-2502021-3 | |

| MATERIAL DESCRIPTION | Depth (ft) | Elevation | Sample No. & Type | N | $Q_u$ (tsf) | $Q_p$ (tsf) | $Q_s$ (tsf) | W (%) | PID | NOTES |
|---|---|---|---|---|---|---|---|---|---|---|
| **±6" Topsoil:** Dark Brown Silty fine Sand, trace Organic Matter-Moist | | 670 | 1-SS | 6 | | | | | | |
| **Fill:** Dark Brown fine to medium Sand, trace Gravel, trace to little Silt-Moist | | | 2-SS | 4 | | | | | | |
| | 5 | 665 | 3-SS | 6 | | | | | | |
| | | | 4-SS | 8 | | | | | | |
| Brown fine to medium Sand-Moist | 10 | 660 | 5-SS | 10 | | | | | | |
| | 15 | 655 | 6-SS | 11 | | | | | | |
| | 20 | 650 | 7-SS | 14 | | | | | | |
| | 25 | 645 | 8-SS | 20 | | | | | | |
| Boring Terminated at about 26 feet (EL. 644.4') | | | | | | | | | | |

| Water Observation Data | Remarks: |
|---|---|
| Water Encountered During Drilling: | |
| Water Level At End of Drilling: | |
| Cave Depth At End of Drilling: | |
| Water Level After Drilling: | |
| Cave Depth After Drilling: | |

GILES LOG REPORT 1G2502021-3.GPJ GILES.GDT 5/5/25

Changes in strata indicated by the lines are approximate boundary between soil types. The actual transition may be gradual and may vary considerably between test borings. Location of test boring is shown on the Boring Location Plan.

| BORING NO. & LOCATION: 15 | TEST BORING LOG | GILES ENGINEERING ASSOCIATES, INC. |
|---|---|---|
| SURFACE ELEVATION: 670.3 feet | PROPOSED ELEMENTARY SCHOOL | |
| COMPLETION DATE: 04/24/25 | 807 EAST AVENUE SOUTH<br>LA CROSSE, WISCONSIN | |
| FIELD REP: DAVIS LUCKETT | PROJECT NO: 1G-2502021-3 | |

| MATERIAL DESCRIPTION | Depth (ft) | Elevation | Sample No. & Type | N | $Q_u$ (tsf) | $Q_p$ (tsf) | $Q_s$ (tsf) | W (%) | PID | NOTES |
|---|---|---|---|---|---|---|---|---|---|---|
| **±6" Topsoil:** Dark Brown Silty fine Sand, trace Organic Matter-Moist | | 670 | 1-SS | 5 | | | | | | |
| **Fill:** Dark Brown fine to medium Sand, trace Gravel, trace to little Silt-Moist | | | 2-SS | 5 | | | | | | |
| | 5 | 665 | 3-SS | 6 | | | | | | |
| | | | 4-SS | 10 | | | | | | |
| Brown fine to medium Sand-Moist | 10 | 660 | 5-SS | 9 | | | | | | |
| | 15 | 655 | 6-SS | 11 | | | | | | |
| | 20 | 650 | 7-SS | 14 | | | | | | |
| | 25 | 645 | 8-SS | 19 | | | | | | |
| Boring Terminated at about 26 feet (EL. 644.3') | | | | | | | | | | |

| Water Observation Data | Remarks: |
|---|---|
| Water Encountered During Drilling: | |
| Water Level At End of Drilling: | |
| Cave Depth At End of Drilling: | |
| Water Level After Drilling: | |
| Cave Depth After Drilling: | |

GILES LOG REPORT 1G2502021-3.GPJ GILES.GDT 5/5/25

Changes in strata indicated by the lines are approximate boundary between soil types. The actual transition may be gradual and may vary considerably between test borings. Location of test boring is shown on the Boring Location Plan.

| BORING NO. & LOCATION: 16 | TEST BORING LOG | GILES ENGINEERING ASSOCIATES, INC. |
|---|---|---|
| SURFACE ELEVATION: 670.5 feet | PROPOSED ELEMENTARY SCHOOL | |
| COMPLETION DATE: 04/23/25 | 807 EAST AVENUE SOUTH<br>LA CROSSE, WISCONSIN | |
| FIELD REP: DAVIS LUCKETT | PROJECT NO: 1G-2502021-3 | |

| MATERIAL DESCRIPTION | Depth (ft) | Elevation | Sample No. & Type | N | $Q_u$ (tsf) | $Q_p$ (tsf) | $Q_s$ (tsf) | W (%) | PID | NOTES |
|---|---|---|---|---|---|---|---|---|---|---|
| **±6" Topsoil:** Dark Brown Sandy Silt, little Organic Matter-Moist | | 670 | 1-SS | 5 | | | | | | |
| **Fill:** Dark Brown fine to medium Sand, little Silt-Moist | | | 2-SS | 7 | | | | | | |
| | 5 | 665 | 3-SS | 8 | | | | | | |
| | | | 4-SS | 11 | | | | | | |
| Brown fine to medium Sand-Moist | 10 | 660 | 5-SS | 8 | | | | | | |
| | 15 | 655 | 6-SS | 6 | | | | | | |
| | 20 | 650 | 7-SS | 10 | | | | | | |
| | 25 | 645 | 8-SS | 22 | | | | | | |
| Boring Terminated at about 26 feet (EL. 644.5') | | | | | | | | | | |

| Water Observation Data | Remarks: |
|---|---|
| Water Encountered During Drilling: | |
| Water Level At End of Drilling: | |
| Cave Depth At End of Drilling: | |
| Water Level After Drilling: | |
| Cave Depth After Drilling: | |

GILES LOG REPORT 1G2502021-3.GPJ GILES.GDT 5/5/25

Changes in strata indicated by the lines are approximate boundary between soil types. The actual transition may be gradual and may vary considerably between test borings. Location of test boring is shown on the Boring Location Plan.

| BORING NO. & LOCATION: 17 | TEST BORING LOG | GILES ENGINEERING ASSOCIATES, INC. |
|---|---|---|
| SURFACE ELEVATION: 671.5 feet | PROPOSED ELEMENTARY SCHOOL | |
| COMPLETION DATE: 04/23/25 | 807 EAST AVENUE SOUTH<br>LA CROSSE, WISCONSIN | |
| FIELD REP: DAVIS LUCKETT | PROJECT NO: 1G-2502021-3 | |

| MATERIAL DESCRIPTION | Depth (ft) | Elevation | Sample No. & Type | N | $Q_u$ (tsf) | $Q_p$ (tsf) | $Q_s$ (tsf) | W (%) | PID | NOTES |
|---|---|---|---|---|---|---|---|---|---|---|
| **±6" Asphalt Concrete** | | | | | | | | | | |
| **±10" Base Course:** Brown Gravelly fine to medium Sand-Damp | | | | | | | | | | |
| **Fill:** Dark Brown fine to medium Sand, little Silt, trace Gravel-Moist | | | | | | | | | | |
| Brown fine Sand-Moist | | | | | | | | | | |
| Brown fine to medium Sand-Moist | | | | | | | | | | |
| | | 670 | 1-SS | 10 | | | | | | |
| | | | 2-SS | 11 | | | | | | |
| | 5 | | 3-SS | 15 | | | | | | |
| | | 665 | 4-SS | 13 | | | | | | |
| | 10 | | 5-SS | 9 | | | | | | |
| | | 660 | | | | | | | | |
| | 15 | | 6-SS | 7 | | | | | | |
| Boring Terminated at about 16 feet (EL. 655.5') | | | | | | | | | | |

| | Water Observation Data | Remarks: |
|---|---|---|
| ▽ | Water Encountered During Drilling: | |
| ▼ | Water Level At End of Drilling: | |
| | Cave Depth At End of Drilling: | |
| ▼ | Water Level After Drilling: | |
| | Cave Depth After Drilling: | |

GILES LOG REPORT 1G2502021-3.GPJ GILES.GDT 5/5/25

Changes in strata indicated by the lines are approximate boundary between soil types. The actual transition may be gradual and may vary considerably between test borings. Location of test boring is shown on the Boring Location Plan.

| BORING NO. & LOCATION: 18 | TEST BORING LOG | GILES ENGINEERING ASSOCIATES, INC. |
|---|---|---|
| SURFACE ELEVATION: 671.5 feet | PROPOSED ELEMENTARY SCHOOL | |
| COMPLETION DATE: 04/23/25 | 807 EAST AVENUE SOUTH<br>LA CROSSE, WISCONSIN | |
| FIELD REP: DAVIS LUCKETT | PROJECT NO: 1G-2502021-3 | |

| MATERIAL DESCRIPTION | Depth (ft) | Elevation | Sample No. & Type | N | $Q_u$ (tsf) | $Q_p$ (tsf) | $Q_s$ (tsf) | W (%) | PID | NOTES |
|---|---|---|---|---|---|---|---|---|---|---|
| **±4" Asphalt Concrete** | | | | | | | | | | |
| **±8.5" Base Course:** Brown Sandy Gravel-Damp | | | | | | | | | | |
| **Fill:** Dark Brown fine Sand, little Silt-Moist | | 670 | 1-SS | 11 | | | | | | |
| Brown fine Sand, little Silt-Moist | | | 2-SS | 9 | | | | | | |
| Brown fine to medium Sand-Moist | 5 | | 3-SS | 16 | | | | | | |
| | | 665 | | | | | | | | |
| | | | 4-SS | 11 | | | | | | |
| | 10 | | 5-SS | 9 | | | | | | |
| | | 660 | | | | | | | | |
| | 15 | | 6-SS | 8 | | | | | | |
| Boring Terminated at about 16 feet (EL. 655.5') | | | | | | | | | | |

| Water Observation Data | Remarks: |
|---|---|
| Water Encountered During Drilling: | |
| Water Level At End of Drilling: | |
| Cave Depth At End of Drilling: | |
| Water Level After Drilling: | |
| Cave Depth After Drilling: | |

GILES LOG REPORT 1G2502021-3.GPJ GILES.GDT 5/5/25

Changes in strata indicated by the lines are approximate boundary between soil types. The actual transition may be gradual and may vary considerably between test borings. Location of test boring is shown on the Boring Location Plan.

Wisconsin Department of Safety and Professional Services
Division of Industry Services
4822 Madison Yards Way
PO Box 7302
Madison, WI 53707

Phone: 608-266-2112
Web: http://dsps.wi.gov
Email: dsps@wisconsin.gov

Program: Plumbing
Web: https://dsps.wi.gov/Pages/Programs/Plumbing

# Soil and Site Evaluation – Stormwater Infiltration

In accordance with SPS 382.365, 385, Wis. Adm. Code, and WDNR Standard 1002

Personal information you provide may be used for secondary purposes [Privacy Law, s. 15.04(1)(m)]



| Attach a complete site plan on paper not less than 8 ½ x 11 inches in size. Plan must include but is not limited to: vertical and horizontal reference point (BM); direction and percent of slope; scale or dimensions; north arrow; and BM referenced to nearest road.<br>**PLEASE PRINT ALL INFORMATION** | COUNTY<br>La Crosse<br>PARCEL ID |
|---|---|

| PROPERTY OWNER<br>PROPERTY OWNER'S MAILING ADDRESS<br>CITY, STATE, ZIP CODE PHONE | PROPERTY LOCATION<br>Govt. Lot: NW ¼ SW ¼ S4 T15N R7W<br>Lot #, Block #, Subd. Name or CSM #:<br>Municipality: La Crosse<br>☒ City ☐ Village ☐ Town<br>Nearest Road: East Avenue South |
|---|---|

| Drainage area ☐sq. ft. ☐acres<br>Test site suitable for (check all that apply):<br>☐ Site not suitable ☐Bioretention<br>☐ Reuse ☐Subsurface Dispersal System<br>☐ Irrigation ☐Other | HYDRAULIC APPLICATION TEST METHOD<br>☒ Morphological Evaluation<br>☐ Double Ring Infiltrometer<br>☐ Other: (specify) | SOIL MOISTURE<br>Date of soil borings: 4/30/2025<br>USDA-NRCS WETS VALUE:<br>☐ Dry = 1<br>☐ Normal = 2<br>☐ Wet = 3 |
|---|---|---|

1 #OBS. ☒ Pit ☐ Boring Ground Surface Elevation 671.5 ft. Elevation of Limiting Factor _____ ft.

| Horizon | Depth in. | Dominant Color Munsell | Redox Description Qu. Sz. Cont. Color | Texture | Structure Gr. Sz. Sh. | Consistence | Boundary | % Rock Frags. | % Fines | Hydraulic App. Rate Inches/Hr. |
|---|---|---|---|---|---|---|---|---|---|---|
| PVMT | 0-3 | -- | -- | -- | -- | -- | -- | -- | -- | -- |
| FILL | 3-13 | 7.5YR 7/4 | -- | SGr | MA | M, FI | A, S | 80 | 5 | -- |
| FILL | 13-36 | 7.5YR 4/4 | -- | LS | MA | M, FI | C, S | <5 | 10 | 1.63 |
| C | 36-60 | 7.5YR 6/4 | -- | S | MA | M, FI | -- | <5 | 5 | 3.60 |

Comments: Pulverized asphalt encountered within the fill material.
Caved at 60 inches

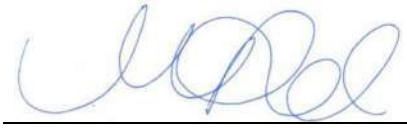
2 #OBS. ☒ Pit ☐ Boring Ground Surface Elevation 671.7 ft. Elevation of Limiting Factor _____ ft.

| Horizon | Depth in. | Dominant Color Munsell | Redox Description Qu. Sz. Cont. Color | Texture | Structure Gr. Sz. Sh. | Consistence | Boundary | % Rock Frags. | % Fines | Hydraulic App. Rate Inches/Hr. |
|---|---|---|---|---|---|---|---|---|---|---|
| A | 0-12 | 7.5YR 3/2 | -- | fS | MA | M, FR | A, S | <5 | 10 | -- |
| FILL | 12-36 | 7.5YR 4/4 | -- | LS | MA | M, F1 | C, S | 10 | 10 | 1.63 |
| C | 36-132 | 7.5YR 5/4 | -- | S | MA | M, FI | -- | <5 | <5 | 3.60 |

Comments: Metal debris encountered within the fill material.
Caved at 132 inches.

3 #OBS. ☒ Pit ☐ Boring Ground Surface Elevation 672 ft. Elevation of Limiting Factor _____ ft.

| Horizon | Depth in. | Dominant Color Munsell | Redox Description Qu. Sz. Cont. Color | Texture | Structure Gr. Sz. Sh. | Consistence | Boundary | % Rock Frags. | % Fines | Hydraulic App. Rate Inches/Hr. |
|---|---|---|---|---|---|---|---|---|---|---|
| A | 0-4 | 7.5YR 3/2 | -- | LS | MA | M, FR | A, S | <5 | 20 | -- |
| C | 4-108 | 7.5YR 5/4 | -- | LS | MA | M, FI | -- | <5 | 10 | 1.63 |
| Comments: Caved at 108 inches. | | | | | | | | | | |



| Michelle L. Peed, P.G. | | P.G. No.: 1370-13 |
|---|---|---|
| Name (Please Print) | Signature | Credential Number |
| N8 W22350 Johnson Drive, Waukesha, WI | April 30, 2025 | (262) 544-0118 |
| Address | Date Evaluation Conducted | Phone Number |

# APPENDIX B

## FIELD PROCEDURES

The field operations were conducted in general accordance with the procedures recommended by the American Society for Testing and Materials (ASTM) designation D

420 entitled “Standard Guide for Sampling Rock and Rock” and/or other relevant specifications. Soil samples were preserved and transported to *Giles'* laboratory in general accordance with the procedures recommended by ASTM designation D 4220 entitled “Standard Practice for Preserving and Transporting Soil Samples.” Brief descriptions of the sampling, testing and field procedures commonly performed by *Giles* are provided herein.

# GENERAL FIELD PROCEDURES

## Test Boring Elevations

The ground surface elevations reported on the Test Boring Logs are referenced to the assumed benchmark shown on the Boring Location Plan (Figure 1). Unless otherwise noted, the elevations were determined with a conventional hand-level and are accurate to within about 1 foot.

## Test Boring Locations

The test borings were located on-site based on the existing site features and/or apparent property lines. Dimensions illustrating the approximate boring locations are reported on the Boring Location Plan (Figure 1).

## Water Level Measurement

The water levels reported on the Test Boring Logs represent the depth of “free” water encountered during drilling and/or after the drilling tools were removed from the borehole. Water levels measured within a granular (sand and gravel) soil profile are typically indicative of the water table elevation. It is usually not possible to accurately identify the water table elevation with cohesive (clayey) soils, since the rate of seepage is slow. The water table elevation within cohesive soils must therefore be determined over a period of time with groundwater observation wells.

It must be recognized that the water table may fluctuate seasonally and during periods of heavy precipitation. Depending on the subsurface conditions, water may also become perched above the water table, especially during wet periods.

## Borehole Backfilling Procedures

Each borehole was backfilled upon completion of the field operations. If potential contamination was encountered, and/or if required by state or local regulations, boreholes were backfilled with an “impervious” material (such as bentonite slurry). Borings that penetrated pavements, sidewalks, etc. were “capped” with Portland Cement concrete, asphaltic concrete, or a similar surface material. It must, however, be recognized that the backfill material may settle, and the surface cap may subside, over a period of time. Further backfilling and/or re-surfacing by *Giles’* client or the property owner may be required.

## FIELD SAMPLING AND TESTING PROCEDURES

### Auger Sampling (AU)

Soil samples are removed from the auger flights as an auger is withdrawn above the ground surface. Such samples are used to determine general soil types and identify approximate soil stratifications. Auger samples are highly disturbed and are therefore not typically used for geotechnical strength testing.

### Split-Barrel Sampling (SS) – (ASTM D-1586)

A split-barrel sampler with a 2-inch outside diameter is driven into the subsoil with a 140-pound hammer free-falling a vertical distance of 30 inches. The summation of hammer-blows required to drive the sampler the final 12-inches of an 18-inch sample interval is defined as the "Standard Penetration Resistance" or N-value is an index of the relative density of granular soils and the comparative consistency of cohesive soils. A soil sample is collected from each SPT interval.

### Shelby Tube Sampling (ST) – (ASTM D-1587)

A relatively undisturbed soil sample is collected by hydraulically advancing a thin-walled Shelby Tube sampler into a soil mass. Shelby Tubes have a sharp cutting edge and are commonly 2 to 5 inches in diameter.

### Bulk Sample (BS)

A relatively large volume of soils is collected with a shovel or other manually-operated tool. The sample is typically transported to *Giles'* materials laboratory in a sealed bag or bucket.

### Dynamic Cone Penetration Test (DC) – (ASTM STP 399)

This test is conducted by driving a 1.5-inch-diameter cone into the subsoil using a 15-pound steel ring (hammer), free-falling a vertical distance of 20 inches. The number of hammer-blows required to drive the cone 1¾ inches is an indication of the soil strength and density, and is defined as "N". The Dynamic Cone Penetration test is commonly conducted in hand auger borings, test pits and within excavated trenches.

- Continued -

Ring-Lined Barrel Sampling – (ASTM D 3550)

In this procedure, a ring-lined barrel sampler is used to collect soil samples for classification and laboratory testing. This method provides samples that fit directly into laboratory test instruments without additional handling/disturbance.

Sampling and Testing Procedures

The field testing and sampling operations were conducted in general accordance with the procedures recommended by the American Society for Testing and Materials (ASTM) and/or other relevant specifications. Results of the field testing (i.e. N-values) are reported on the Test Boring Logs. Explanations of the terms and symbols shown on the logs are provided on the appendix enclosure entitled “General Notes”.

# APPENDIX C

## LABORATORY TESTING AND CLASSIFICATION

The laboratory testing was conducted under the supervision of a geotechnical engineer in accordance with the procedures recommended by the American Society for Testing and Materials (ASTM) and/or other relevant specifications. Brief descriptions of laboratory tests commonly performed by *Giles* are provided herein.

## LABORATORY TESTING AND CLASSIFICATION

### Photoionization Detector (PID)

In this procedure, soil samples are "scanned" in *Giles'* analytical laboratory using a Photoionization Detector (PID). The instrument is equipped with an 11.7 eV lamp calibrated to a Benzene Standard and is capable of detecting a minute concentration of **certain** Volatile Organic Compound (VOC) vapors, such as those commonly associated with petroleum products and some solvents. Results of the PID analysis are expressed in HNu (manufacturer's) units rather than actual concentration.

### Moisture Content (w) (ASTM D 2216)

Moisture content is defined as the ratio of the weight of water contained within a soil sample to the weight of the dry solids within the sample. Moisture content is expressed as a percentage.

### Unconfined Compressive Strength (qu) (ASTM D 2166)

An axial load is applied at a uniform rate to a cylindrical soil sample. The unconfined compressive strength is the maximum stress obtained or the stress when 15% axial strain is reached, whichever occurs first.

### Calibrated Penetrometer Resistance (qp)

The small, cylindrical tip of a hand-held penetrometer is pressed into a soil sample to a prescribed depth to measure the soils capacity to resist penetration. This test is used to evaluate unconfined compressive strength.

### Vane-Shear Strength (qs)

The blades of a vane are inserted into the flat surface of a soil sample and the vane is rotated until failure occurs. The maximum shear resistance measured immediately prior to failure is taken as the vane-shear strength.

### Loss-on-Ignition (ASTM D 2974; Method C)

The Loss-on-Ignition (L.O.I.) test is used to determine the organic content of a soil sample. The procedure is conducted by heating a dry soil sample to 440°C in order to burn-off or "ash" organic matter present within the sample. The L.O.I. value is the ratio of the weight loss due to ignition compared to the initial weight of the dry sample. L.O.I. is expressed as a percentage.

Particle Size Distribution (ASTB D 421, D 422, and D 1140)

This test is performed to determine the distribution of specific particle sizes (diameters) within a soil sample. The distribution of coarse-grained soil particles (sand and gravel) is determined from a “sieve analysis,” which is conducted by passing the sample through a series of nested sieves. The distribution of fine-grained soil particles (silt and clay) is determined from a “hydrometer analysis” which is based on the sedimentation of particles suspended in water.

Consolidation Test (ASTM D 2435)

In this procedure, a series of cumulative vertical loads are applied to a small, laterally confined soil sample. During each load increment, vertical compression (consolidation) of the sample is measured over a period of time. Results of this test are used to estimate settlement and time rate of settlement.

Classification of Samples

Each soil sample was visually-manually classified, based on texture and plasticity, in general accordance with the Unified Soil Classification System (ASTM D-2488-75). The classifications are reported on the Test Boring Logs.

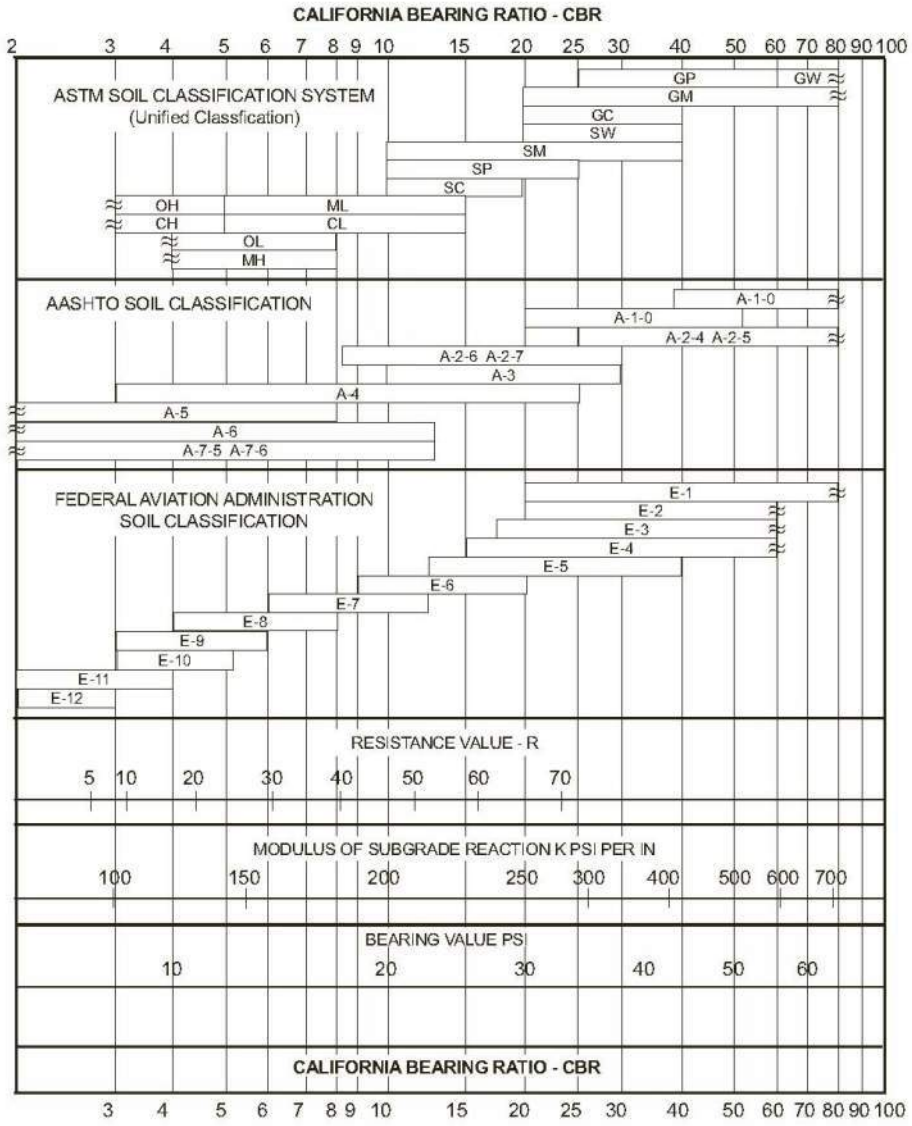
Laboratory Testing

The laboratory testing operations were conducted in general accordance with the procedures recommended by the American Society for Testing and Materials (ASTM) and/or other relevant specifications. Results of the laboratory tests are provided on the Test Boring Logs or other appendix enclosures. Explanation of the terms and symbols used on the logs is provided on the appendix enclosure entitled “General Notes.”

California Bearing Ratio (CBR) Test ASTM D-1833

The CBR test is used for evaluation of a soil subgrade for pavement design. The test consists of measuring the force required for a 3-square-inch cylindrical piston to penetrate 0.1 or 0.2 inch into a compacted soil sample. The result is expressed as a percent of force required to penetrate a standard compacted crushed stone.

Unless a CBR test has been specifically requested by the client, the CBR is estimated from published charts, based on soil classification and strength characteristics. A typical correlation chart is below.



# APPENDIX D

GENERAL INFORMATION

AND
IMPORTANT INFORMATION ABOUT
THIS GEOTECHNICAL REPORT

## GENERAL COMMENTS

The soil samples obtained during the subsurface exploration will be retained for a period of thirty days. If no instructions are received, they will be disposed of at that time.

This report has been prepared exclusively for the client in order to aid in the evaluation of this property and to assist the architects and engineers in the design and preparation of the project plans and specifications. Copies of this report may be provided to contractor(s), with contract documents, to disclose information relative to this project. The report, however, has not been prepared to serve as the plans and specifications for actual construction without the appropriate interpretation by the project architect, structural engineer, and/or civil engineer. Reproduction and distribution of this report must be authorized by the client and *Giles*.

This report has been based on assumed conditions/characteristics of the proposed development where specific information was not available. It is recommended that the architect, civil engineer and structural engineer along with any other design professionals involved in this project carefully review these assumptions to ensure they are consistent with the actual planned development. When discrepancies exist, they should be brought to our attention to ensure they do not affect the conclusions and recommendations provided herein. The project plans and specifications may also be submitted to *Giles* for review to ensure that the geotechnical related conclusions and recommendations provided herein have been correctly interpreted.

The analysis of this site was based on a subsoil profile interpolated from a limited subsurface exploration. If the actual conditions encountered during construction vary from those indicated by the borings, *Giles* must be contacted immediately to determine if the conditions alter the recommendations contained herein.

The conclusions and recommendations presented in this report have been promulgated in accordance with generally accepted professional engineering practices in the field of geotechnical engineering. No other warranty is either expressed or implied.

**GUIDE SPECIFICATIONS FOR SUBGRADE AND GRADE PREPARATION FOR FILL, FOUNDATION, FLOOR SLAB AND PAVEMENT SUPPORT; AND SELECTION, PLACEMENT AND COMPACTION OF FILL SOILS USING STANDARD PROCTOR PROCEDURES**

1. Construction monitoring and testing of subgrades and grades for fill, foundation, floor slab and pavement; and fill selection, placement and compaction shall be performed by an experienced soils engineer and/or his representatives.

2. All compaction fill, subgrades and grades shall be (a) underlain by suitable bearing material; (b) free of all organic, frozen, or other deleterious material, and (c) observed, tested and approved by qualified engineering personnel representing an experienced soils engineer. Preparation of subgrades after stripping vegetation, organic or other unsuitable materials shall consist of (a) proof-rolling to detect soil, wet yielding soils or other unstable materials that must be undercut, (b) scarifying top 6 to 8 inches, (c) moisture conditioning the soils as required, and (d) recompaction to same minimum in-situ density required for similar materials indicated under Item 5. Note: compaction requirements for pavement subgrade are higher than other areas. Weather and construction equipment may damage compacted fill surface and reworking and retesting may be necessary to assure proper performance.

3. In overexcavation and fill areas, the compacted fill must extend (a) a minimum 1 foot lateral distance beyond the exterior edge of the foundation at bearing grade or pavement subgrade and down to compacted fill subgrade on a maximum 0.5(H):1(V) slope, (b) 1 foot above footing grade outside the building, and (c) to floor subgrade inside the building. Fill shall be placed and compacted on a 5(H):1(V) slope or must be stepped or benched as required to flatten if not specifically approved by qualified personnel under the direction of an experienced soil engineer.

4. The compacted fill materials shall be free of deleterious, organic, or frozen matter, shall contain no chemicals that may result in the material being classified as "contaminated", and shall be low-expansive with a maximum Liquid Limit (ASTM D-423) and Plasticity Index (ASTM D-424) of 30 and 15, respectively, unless specifically tested and found to have low expansive properties and approved by an experienced soils engineer. The top 12 inches of compacted fill should have a maximum 3-inch-particle diameter and all underlying compacted fill a maximum 6-inch-diameter unless specifically approved by an experienced soils engineer. All fill materials must be tested and approved under the direction of an experienced soils engineer prior to placement. If the fill is to provide non-frost susceptible characteristics, it must be classified as a clean GW, GP, SW or SP per the Unified Soil Classification System (ASTM D-2487).

5. For structural fill depths less than 20 feet, the density of the structural compacted fill and scarified subgrade and grades shall not be less than 95 percent of the maximum dry density as determined by Standard Proctor (ASTM-698) with the exception of the top 12 inches of pavement subgrade which shall have a minimum in-situ density of 100 percent of maximum dry density, or 5 percent higher than underlying fill materials. Where the structural fill depth is greater than 20 feet, the portions below 20 feet should have a minimum in-place density of 100 percent of its maximum dry density of 5 percent greater than the top 20 feet. The moisture content of cohesive soil shall not vary by more than -1 to +3 percent and granular soil ±3 percent of the optimum when placed and compacted or recompacted, unless specifically recommended/approved by the soils engineer monitoring the placement and compaction. Cohesive soils with moderate to high expansion potentials (PI>15) should, however, be placed, compacted and maintained prior to construction at a moisture content 3±1 percent above optimum moisture content to limit further heave. The fill shall be placed in layers with a maximum loose thickness of 8 inches for foundations and 10 inches for floor slabs and pavement, unless specifically approved by the soils engineer taking into consideration the type of materials and compaction equipment being used. The compaction equipment should consist of suitable mechanical equipment specifically designed for soil compaction. Bulldozers or similar tracked vehicles are typically not suitable for compaction.

6. Excavation, filling, subgrade and grade preparation shall be performed in a manner and sequence that will provide drainage at all times and proper control of erosion. Precipitation, springs and seepage water encountered shall be pumped or drained to provide a suitable working platform. Springs or water seepage encountered during grading/foundation construction must be called to the soil engineer's attention immediately for possible construction procedure revision or inclusion of an underdrain system.

7. Non-structural fill adjacent to structural fill should typically be placed in unison to provide lateral support. Backfill along walls must be placed and compacted with care to ensure excessive unbalanced lateral pressures do not develop. The type of fill material placed adjacent to below-grade walls (i.e. basement walls and retaining walls) must be properly tested and approved by an experienced soils engineer with consideration for the lateral pressure used in the wall design.

8. Whenever, in the opinion of the soils engineer or the Owner's Representatives, an unstable condition is being created either by cutting or filling, the work shall not proceed into that area until an appropriate geotechnical exploration and analysis has been performed and the grading plan revised, if found necessary.

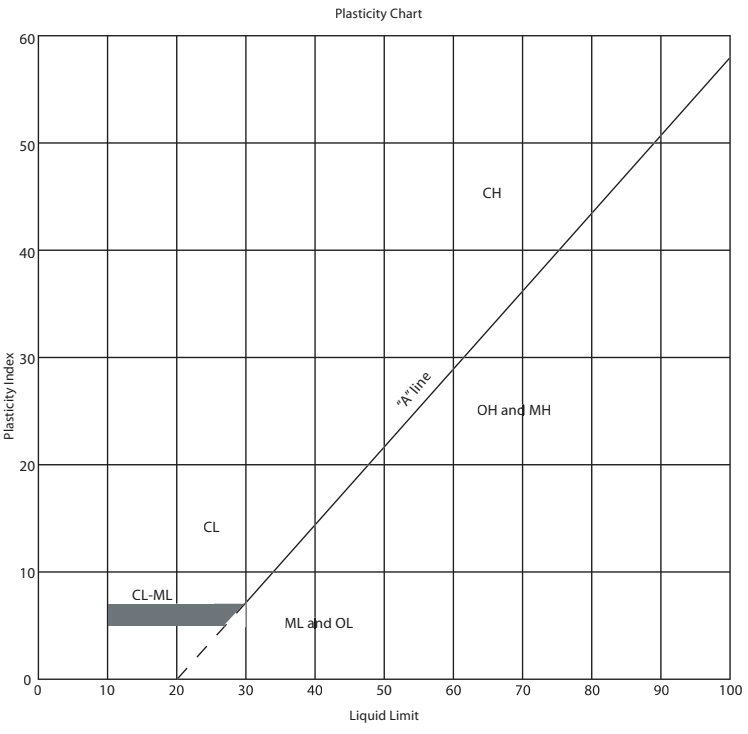
| CHARACTERISTICS AND RATINGS OF UNIFIED SOIL SYSTEM CLASSES FOR SOIL CONSTRUCTION * | | | | | | | | | |
|---|---|---|---|---|---|---|---|---|---|
| **Class** | **Compaction Characteristics** | **Max. Dry Density Standard Proctor (pcf)** | **Compressibility and Expansion** | **Drainage and Permeability** | **Value as an Embankment Material** | **Value as Subgrade When Not Subject to Frost** | **Value as Base Course** | **Value as Temporary Pavement** | |
| | | | | | | | | **With Dust Palliative** | **With Bituminous Treatment** |
| GW | Good: tractor, rubber-tired, steel wheel or vibratory roller | 125-135 | Almost none | Good drainage, pervious | Very stable | Excellent | Good | Fair to poor | Excellent |
| GP | Good: tractor, rubber-tired, steel wheel or vibratory roller | 115-125 | Almost none | Good drainage, pervious | Reasonably stable | Excellent to good | Poor to fair | Poor | |
| GM | Good: rubber-tired or light sheepsfoot roller | 120-135 | Slight | Poor drainage, semipervious | Reasonably stable | Excellent to good | Fair to poor | Poor | Poor to fair |
| GC | Good to fair: rubber-tired or sheepsfoot roller | 115-130 | Slight | Poor drainage, impervious | Reasonably stable | Good | Good to fair ** | Excellent | Excellent |
| SW | Good: tractor, rubber-tired or vibratory roller | 110-130 | Almost none | Good drainage, pervious | Very stable | Good | Fair to poor | Fair to poor | Good |
| SP | Good: tractor, rubber-tired or vibratory roller | 100-120 | Almost none | Good drainage, pervious | Reasonably stable when dense | Good to fair | Poor | Poor | Poor to fair |
| SM | Good: rubber-tired or sheepsfoot roller | 110-125 | Slight | Poor drainage, impervious | Reasonably stable when dense | Good to fair | Poor | Poor | Poor to fair |
| SC | Good to fair: rubber-tired or sheepsfoot roller | 105-125 | Slight to medium | Poor drainage, impervious | Reasonably stable | Good to fair | Fair to poor | Excellent | Excellent |
| ML | Good to poor: rubber-tired or sheepsfoot roller | 95-120 | Slight to medium | Poor drainage, impervious | Poor stability, high density required | Fair to poor | Not suitable | Poor | Poor |
| CL | Good to fair: sheepsfoot or rubber-tired roller | 95-120 | Medium | No drainage, impervious | Good stability | Fair to poor | Not suitable | Poor | Poor |
| OL | Fair to poor: sheepsfoot or rubber-tired roller | 80-100 | Medium to high | Poor drainage, impervious | Unstable, should not be used | Poor | Not suitable | Not suitable | Not suitable |
| MH | Fair to poor: sheepsfoot or rubber-tired roller | 70-95 | High | Poor drainage, impervious | Poor stability, should not be used | Poor | Not suitable | Very poor | Not suitable |
| CH | Fair to poor: sheepsfoot roller | 80-105 | Very high | No drainage, impervious | Fair stability, may soften on expansion | Poor to very poor | Not suitable | Very poor | Not suitable |
| OH | Fair to poor: sheepsfoot roller | 65-100 | High | No drainage, impervious | Unstable, should not be used | Very poor | Not suitable | Not suitable | Not suitable |
| Pt | Not suitable | | Very high | Fair to poor drainage | Should not be used | Not suitable | Not suitable | Not suitable | Not suitable |

* "The Unified Classification: Appendix A - Characteristics of Soil, Groups Pertaining to Roads and Airfields, and Appendix B - Characteristics of Soil Groups Pertaining to Embankments and Foundations," Technical Memorandum 357, U.S. Waterways Ixperiment Station, Vicksburg, 1953.

** Not suitable if subject to frost.

# UNIFIED SOIL CLASSIFICATION SYSTEM (ASTM D-2487)

| Major Divisions | | | Group Symbols | Typical Names | Laboratory Classification Criteria | |
|---|---|---|---|---|---|---|
| Coarse-grained soils (more than half of material is larger than No. 200 sieve size) | Gravels (More than half of coarse fraction is larger than No. 4 sieve size) | Clean gravels (little or no fines) | GW | Well-graded gravels, gravel-sand mixtures, little or no fines | Determine percentages of sand and gravel from grain-size curve. Depending on percentage of fines (fraction smaller than No. 200 sieve size), coarse-grained soils are classified as follows: Less than 5 percent: GW, GP, SW, SP. More than 12 percent: GM, GC, SM, SC. 5 to 12 percent: *Borderline* cases requiring dual symbols[b] | $C_u = \frac{D_{60}}{D_{10}}$ greater than 4; $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3 |
| | | | GP | Poorly graded gravels, gravel-sand mixtrues, little or no fines | | Not meeting all gradation requirements for GW |
| | | Gravels with fines (appreciable amount of fines) | GM[a] d | Silty gravels, gravel-sand-silt mixtures | Atterberg limits below "A" line or P.I. less than 4 | Limits plotting within shaded area, above "A" line with P.I. between 4 and 7 are *borderline* cases requiring use of dual symbols |
| | | | GM[a] u | | | |
| | | | GC | Clayey gravels, gravel-sand-clay mixtures | Atterberg limits above "A" line or P.I. greater than 7 | |
| | Sands (More than half of coarse fraction is smaller than No. 4 sieve size) | Clean sands (Little or no fines) | SW | Well-graded sands, gravelly sands, little or no fines | $C_u = \frac{D_{60}}{D_{10}}$ greater than 4; $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3 | |
| | | | SP | Poorly graded sands, gravelly sands, little or no fines | | Not meeting all gradation requirements for SW |
| | | Sands with fines (Appreciable amount of fines) | SM[a] d | Silty sands, sand-silt mixtures | Atterberg limits below "A" line or P.I. less than 4 | Limits plotting within shaded area, above "A" line with P.I. between 4 and 7 are *borderline* cases requiring use of dual symbols |
| | | | SM[a] u | | | |
| | | | SC | Clayey sands, sand-clay mixtures | Atterberg limits above "A" line or P.I. greater than 7 | |
| Fine-grained soils (More than half material is smaller than No. 200 sieve size) | Silts and clays (Liquid limit less than 50) | | ML | Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity | Plasticity Chart | |
| | | | CL | Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays | | |
| | | | OL | Organic silts and organic silty clays of low plasticity | | |
| | Silts and clays (Liquid limit greater than 50) | | MH | Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts | | |
| | | | CH | Inorganic clays of high plasticity, fat clays | | |
| | | | OH | Organic clays of medium to high plasticity, organic silts | | |
| | Highly organic soils | | Pt | Peat and other highly organic soils | | |




[a] Division of GM and SM groups into subdivisions of d and u are for roads and airfields only. Subdivision is based on Atterberg limits, suffix d used when L.L. is 28 or less and the P.I. is 6 or less; the suffix u is used when L.L. is greater than 28.

[b] Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group sympols. For example GW-GC, well-graded gravel-sand mixture with clay binder.

# GENERAL NOTES

***SAMPLE IDENTIFICATION***

All samples are visually classified in general accordance with the Unified Soil Classification System (ASTM D-2487-75 or D-2488-75)

***DESCRIPTIVE TERM (% BY DRY WEIGHT)***

| | |
|---|---|
| Trace: | 1-10% |
| Little: | 11-20% |
| Some: | 21-35% |
| And/Adjective | 36-50% |

***PARTICLE SIZE (DIAMETER)***

| | |
|---|---|
| Boulders: | 8 inch and larger |
| Cobbles: | 3 inch to 8 inch |
| Gravel: | coarse - ¾ to 3 inch |
| | fine – No. 4 (4.76 mm) to ¾ inch |
| Sand: | coarse – No. 4 (4.76 mm) to No. 10 (2.0 mm) |
| | medium – No. 10 (2.0 mm) to No. 40 (0.42 mm) |
| | fine – No. 40 (0.42 mm) to No. 200 (0.074 mm) |
| Silt: | No. 200 (0.074 mm) and smaller (non-plastic) |
| Clay: | No 200 (0.074 mm) and smaller (plastic) |

***SOIL PROPERTY SYMBOLS***

| | |
|---|---|
| Dd: | Dry Density (pcf) |
| LL: | Liquid Limit, percent |
| PL: | Plastic Limit, percent |
| PI: | Plasticity Index (LL-PL) |
| LOI: | Loss on Ignition, percent |
| Gs: | Specific Gravity |
| K: | Coefficient of Permeability |
| w: | Moisture content, percent |
| qp: | Calibrated Penetrometer Resistance, tsf |
| qs: | Vane-Shear Strength, tsf |
| qu: | Unconfined Compressive Strength, tsf |
| qc: | Static Cone Penetrometer Resistance (correlated to Unconfined Compressive Strength, tsf) |
| PID: | Results of vapor analysis conducted on representative samples utilizing a Photoionization Detector calibrated to a benzene standard. Results expressed in HNU-Units. (BDL=Below Detection Limit) |

***DRILLING AND SAMPLING SYMBOLS***

| | |
|---|---|
| SS: | Split-Spoon |
| ST: | Shelby Tube – 3 inch O.D. (except where noted) |
| CS: | 3 inch O.D. California Ring Sampler |
| DC: | Dynamic Cone Penetrometer per ASTM Special Technical Publication No. 399 |
| AU: | Auger Sample |
| DB: | Diamond Bit |
| CB: | Carbide Bit |
| WS: | Wash Sample |
| RB: | Rock-Roller Bit |
| BS: | Bulk Sample |
| Note: | Depth intervals for sampling shown on Record of Subsurface Exploration are not indicative of sample recovery, but position where sampling initiated |

N: Penetration Resistance per 12 inch interval, or fraction thereof, for a standard 2 inch O.D. (1⅜ inch I.D.) split spoon sampler driven with a 140 pound weight free-falling 30 inches. Performed in general accordance with Standard Penetration Test Specifications (ASTM D-1586). N in blows per foot equals sum of N-Values where plus sign (+) is shown.

Nc: Penetration Resistance per 1¾ inches of Dynamic Cone Penetrometer. Approximately equivalent to Standard Penetration Test N-Value in blows per foot.

Nr: Penetration Resistance per 12 inch interval, or fraction thereof, for California Ring Sampler driven with a 140 pound weight free-falling 30 inches per ASTM D-3550. Not equivalent to Standard Penetration Test N-Value.

***SOIL STRENGTH CHARACTERISTICS***

***COHESIVE (CLAYEY) SOILS***

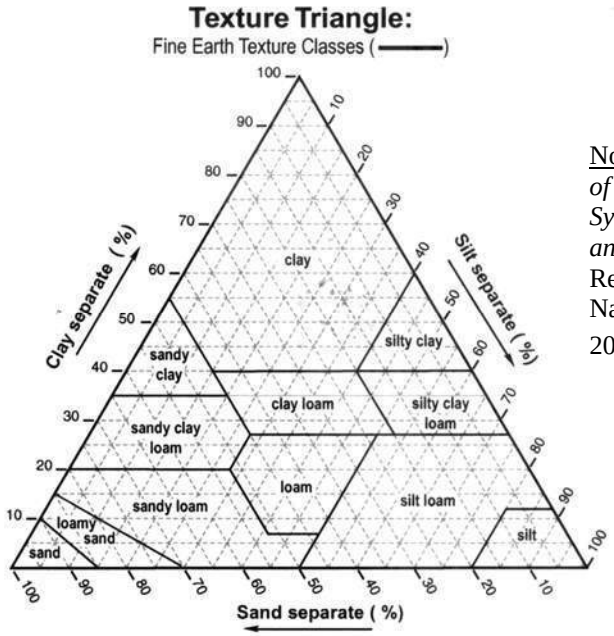
| COMPARATIVE CONSISTENCY | BLOWS PER FOOT (N) | UNCONFINED COMPRESSIVE STRENGTH (TSF) |
|---|---|---|
| Very Soft | 0 - 2 | 0 - 0.25 |
| Soft | 3 - 4 | 0.25 - 0.50 |
| Medium Stiff | 5 – 8 | 0.50 - 1.00 |
| Stiff | 9 – 15 | 1.00 - 2.00 |
| Very Stiff | 16 – 30 | 2.00 - 4.00 |
| Hard | 31+ | 4.00+ |

***NON-COHESIVE (GRANULAR) SOILS***

| RELATIVE DENSITY | BLOWS PER FOOT (N) |
|---|---|
| Very Loose | 0 - 4 |
| Loose | 5 - 10 |
| Firm | 11 - 30 |
| Dense | 31 - 50 |
| Very Dense | 51+ |

| DEGREE OF PLASTICITY | PI | DEGREE OF EXPANSIVE POTENTIAL | PI |
|---|---|---|---|
| None to Slight | 0 - 4 | Low | 0 - 15 |
| Slight | 5 - 10 | Medium | 15 - 25 |
| Medium | 11 - 30 | High | 25+ |
| High to Very High | 31+ | | |

# SOIL CLASSIFICATION NOTES




Note: *Texture Triangle* and *Comparison of Particle Size Classes in Different Systems* from *Field Book for Describing and Sampling Soil*, USDA Natural Resources Conservation Service National Soil Survey Center (September 2002).

## *Comparison of Particle Size Classes in Different Systems*

| | FINE EARTH | | | | | | | | | | ROCK FRAGMENTS | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | | | | | | | | | | | channers | | | flagst. | stones | boulders |
| **USDA** [1] | Clay [2] | | Silt | | Sand | | | | | | Gravel | | | Cobbles | Stones | Boulders |
| | fine | co. | fine | co. | v.fi. | fi. | med. | co. | v. co. | | fine | medium | coarse | | | |
| millimeters: | 0.0002 | .002 mm | | .02 | .05 | .1 | .25 | .5 | 1 | 2 mm | 5 | 20 | | 76 | 250 | 600 mm |
| U.S. Standard Sieve No. *(opening)*: | | | | | 300 [3] | 140 | 60 | 35 | 18 | 10 | 4 | *(3/4")* | | *(3")* | *(10")* | *(25")* |

| | | | | | | | |
|---|---|---|---|---|---|---|---|
| ***International*** [4] | Clay | Silt | Sand | | Gravel | Stones | |
| | | | fine | coarse | | | |
| millimeters: | | .002 mm | .02 | .20 | 2 mm | 20 mm | |
| U.S. Standard Sieve No. *(opening)*: | | | | | 10 | *(3/4")* | |

| | | | | | | | | |
|---|---|---|---|---|---|---|---|---|
| ***Unified*** [5] | Silt or Clay | Sand | | | Gravel | | Cobbles | Boulders |
| | | fine | medium | co. | fine | coarse | | |
| millimeters: | | .074 | .42 | 2 mm | 4.8 | 19 | 76 | 300 mm |
| U.S. Standard Sieve No. *(opening)*: | | 200 | 40 | 10 | 4 | *(3/4")* | *(3")* | |

| | | | | | | | |
|---|---|---|---|---|---|---|---|
| ***AASHTO*** [6,7] | Clay | Silt | Sand | | Gravel or Stones | | Broken Rock (angular), or Boulders (rounded) |
| | | | fine | coarse | fine | med. | co. |
| millimeters: | .005 mm | | .074 | .42 | 2 mm | 9.5 | 25 | 75 mm |
| U.S. Standard Sieve No.: | | | 200 | 40 | 10 | *(3/8")* | *(1")* | *(3")* |

| | | | | | | | | | | | | | | | | | | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| phi #: | 12 | 10 | 9 | 8 | 7 | 6 | 5 | 4 | 3 | 2 | 1 | 0 | -1 | -2 | -3 | -4 | -5 | -6 | -7 | -8 | -9 | -10 | -12 |
| ***Modified Wentworth*** [8] | clay | | | silt | | | | sand | | | | | pebbles | | | | | | Cobbles | | boulders | | |

| | | | | | | | | | | | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| millimeters: | .002 | .004 | .008 | .016 | .031 | .062 | .125 | .25 | .5 | 1 | 2 mm | 8 | 16 | 32 | 64 | 256 | 4092 mm |
| U.S. Standard Sieve No.: | | | | | | 230 | 120 | 60 | 35 | 18 | 10 | 5 | | | | | |


1. Soil Survey Staff. 1995. Soil survey Laboratory information manual. USDA, Natural Resources Conservation Service, Soil Survey Investigations Report No. 45, Version 1.0, National Soil Survey Center, Lincoln, NE. 305 p.
2. Soil Survey Staff. 1995. Soil Survey Lab information manual. USDA-NRCS, Soil Survey Investigation Report #45, version 1.0, National Soil Survey Center, Lincoln, NE. Note: Mineralogy studies may subdivide clay into three size ranges; fine (<0.08µm), medium (0.08-0.2µm), and coarse (0.2-2µm); Jackson, 1969.
3. The Soil Survey Lab (Lincoln, NE) uses a no. 300 sieve (0.047 mm opening) for the USDA-sand/silt measurement. A no. 270 sieve (0.053 mm opening) is more readily available and widely used.
4. International Soil Science Society. 1951. *In*: Soil Survey Manual. Soil Survey Staff, USDA-Soil Conservation Service, Agricultural Handbook No. 18, U.S. Gov. Print. Office, Washington, D.C. 214 p.
5. ASTM. 1993. Standard classification of soils for engineering purposes (Unified Soil Classification System). ASTM designation D2487-92. *In*: Soil and rock; dimension stone; geosynthetics. Annual book of ASTM standards-Vol. 04.08.
6. AASHTO. 1986a. Recommended practice for the classification of soils and soil-aggregate mixtures for highway construction purposes. AASHTO designation M145-82. *In*: Standard specifications for transportation materials and methods of sampling and testing; Part 1: Specifications (14th ed.). American Association of State Highway and Transportation Officials, Washington, D.C.
7. AASHTO. 1986b. Standard definitions of terms relating to subgrade, soil-aggregate, and fill materials. AASHTO designation M146-70 (1980). *In*: sampling and testing; Part 1: Specifications (14th ed.). American Association of State Highway and Transportation Officials, Washington, D.C.
8. Ingram, R.L. 1982. Modified Wentworth scale. *In*: Grain-size scales. AGI Date Sheet 29.1. *In*: Dutro, J.T., Dietrich, R.V., and Foose, R.M. 1989. AGI data sheets for geology in the field, laboratory, and office, 3rd edition. American Geological Institute, Washington, D.C.

# Important Information about This

# Geotechnical-Engineering Report

**Subsurface problems are a principal cause of construction delays, cost overruns, claims, and disputes.**

**While you cannot eliminate all such risks, you can manage them. The following information is provided to help.**

**The Geoprofessional Business Association (GBA) has prepared this advisory to help you – assumedly a client representative – interpret and apply this geotechnical-engineering report as effectively as possible. In that way, you can benefit from a lowered exposure to problems associated with subsurface conditions at project sites and development of them that, for decades, have been a principal cause of construction delays, cost overruns, claims, and disputes. If you have questions or want more information about any of the issues discussed herein, contact your GBA-member geotechnical engineer. Active engagement in GBA exposes geotechnical engineers to a wide array of risk-confrontation techniques that can be of genuine benefit for everyone involved with a construction project.**

## Understand the Geotechnical-Engineering Services Provided for this Report

Geotechnical-engineering services typically include the planning, collection, interpretation, and analysis of exploratory data from widely spaced borings and/or test pits. Field data are combined with results from laboratory tests of soil and rock samples obtained from field exploration (if applicable), observations made during site reconnaissance, and historical information to form one or more models of the expected subsurface conditions beneath the site. Local geology and alterations of the site surface and subsurface by previous and proposed construction are also important considerations. Geotechnical engineers apply their engineering training, experience, and judgment to adapt the requirements of the prospective project to the subsurface model(s). Estimates are made of the subsurface conditions that will likely be exposed during construction as well as the expected performance of foundations and other structures being planned and/or affected by construction activities.

The culmination of these geotechnical-engineering services is typically a geotechnical-engineering report providing the data obtained, a discussion of the subsurface model(s), the engineering and geologic engineering assessments and analyses made, and the recommendations developed to satisfy the given requirements of the project. These reports may be titled investigations, explorations, studies, assessments, or evaluations. Regardless of the title used, the geotechnical-engineering report is an engineering interpretation of the subsurface conditions within the context of the project and does not represent a close examination, systematic inquiry, or thorough investigation of all site and subsurface conditions.

## Geotechnical-Engineering Services are Performed for Specific Purposes, Persons, and Projects, and At Specific Times

Geotechnical engineers structure their services to meet the specific needs, goals, and risk management preferences of their clients. A geotechnical-engineering study conducted for a given civil engineer will not likely meet the needs of a civil-works constructor or even a different civil engineer. Because each geotechnical-engineering study is unique, each geotechnical-engineering report is unique, prepared *solely* for the client.

Likewise, geotechnical-engineering services are performed for a specific project and purpose. For example, it is unlikely that a geotechnical-engineering study for a refrigerated warehouse will be the same as one prepared for a parking garage; and a few borings drilled during a preliminary study to evaluate site feasibility will not be adequate to develop geotechnical design recommendations for the project.

*Do not rely on this report* if your geotechnical engineer prepared it:

- for a different client;
- for a different project or purpose;
- for a different site (that may or may not include all or a portion of the original site); or
- before important events occurred at the site or adjacent to it; e.g., man-made events like construction or environmental remediation, or natural events like floods, droughts, earthquakes, or groundwater fluctuations.

Note, too, the reliability of a geotechnical-engineering report can be affected by the passage of time, because of factors like changed subsurface conditions; new or modified codes, standards, or regulations; or new techniques or tools. *If you are the least bit uncertain* about the continued reliability of this report, contact your geotechnical engineer before applying the recommendations in it. A minor amount of additional testing or analysis after the passage of time – if any is required at all – could prevent major problems.

## Read this Report in Full

Costly problems have occurred because those relying on a geotechnical-engineering report did not read the report in its entirety. Do not rely on an executive summary. Do not read selective elements only. *Read and refer to the report in full.*

## You Need to Inform Your Geotechnical Engineer About Change

Your geotechnical engineer considered unique, project-specific factors when developing the scope of study behind this report and developing the confirmation-dependent recommendations the report conveys. Typical changes that could erode the reliability of this report include those that affect:

- the site's size or shape;
- the elevation, configuration, location, orientation, function or weight of the proposed structure and the desired performance criteria;
- the composition of the design team; or
- project ownership.

As a general rule, *always* inform your geotechnical engineer of project or site changes – even minor ones – and request an assessment of their impact. *The geotechnical engineer who prepared this report cannot accept*

*responsibility or liability for problems that arise because the geotechnical engineer was not informed about developments the engineer otherwise would have considered.*

## Most of the "Findings" Related in This Report Are Professional Opinions

Before construction begins, geotechnical engineers explore a site's subsurface using various sampling and testing procedures. *Geotechnical engineers can observe actual subsurface conditions only at those specific locations where sampling and testing is performed.* The data derived from that sampling and testing were reviewed by your geotechnical engineer, who then applied professional judgement to form opinions about subsurface conditions throughout the site. Actual sitewide-subsurface conditions may differ – maybe significantly – from those indicated in this report. Confront that risk by retaining your geotechnical engineer to serve on the design team through project completion to obtain informed guidance quickly, whenever needed.

## This Report's Recommendations Are Confirmation-Dependent

The recommendations included in this report – including any options or alternatives – are confirmation-dependent. In other words, they are not final, because the geotechnical engineer who developed them relied heavily on judgement and opinion to do so. Your geotechnical engineer can finalize the recommendations *only after observing actual subsurface conditions* exposed during construction. If through observation your geotechnical engineer confirms that the conditions assumed to exist actually do exist, the recommendations can be relied upon, assuming no other changes have occurred. *The geotechnical engineer who prepared this report cannot assume responsibility or liability for confirmation-dependent recommendations if you fail to retain that engineer to perform construction observation.*

## This Report Could Be Misinterpreted

Other design professionals' misinterpretation of geotechnical-engineering reports has resulted in costly problems. Confront that risk by having your geotechnical engineer serve as a continuing member of the design team, to:

- confer with other design-team members;
- help develop specifications;
- review pertinent elements of other design professionals' plans and specifications; and
- be available whenever geotechnical-engineering guidance is needed.

You should also confront the risk of constructors misinterpreting this report. Do so by retaining your geotechnical engineer to participate in prebid and preconstruction conferences and to perform construction-phase observations.

## Give Constructors a Complete Report and Guidance

Some owners and design professionals mistakenly believe they can shift unanticipated-subsurface-conditions liability to constructors by limiting the information they provide for bid preparation. To help prevent the costly, contentious problems this practice has caused, include the complete geotechnical-engineering report, along with any attachments or appendices, with your contract documents, *but be certain to note conspicuously that you've included the material for information purposes only.* To avoid misunderstanding, you may also want to note that "informational purposes" means constructors have no right to rely on the interpretations, opinions, conclusions, or recommendations in the report. Be certain that constructors know they may learn about specific project requirements, including options selected from the report, *only* from the design drawings and specifications. Remind constructors that they may perform their own studies if they want to, and *be sure to allow enough time* to permit them to do so. Only then might you be in a position to give constructors the information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions. Conducting prebid and preconstruction conferences can also be valuable in this respect.

## Read Responsibility Provisions Closely

Some client representatives, design professionals, and constructors do not realize that geotechnical engineering is far less exact than other engineering disciplines. This happens in part because soil and rock on project sites are typically heterogeneous and not manufactured materials with well-defined engineering properties like steel and concrete. That lack of understanding has nurtured unrealistic expectations that have resulted in disappointments, delays, cost overruns, claims, and disputes. To confront that risk, geotechnical engineers commonly include explanatory provisions in their reports. Sometimes labeled "limitations," many of these provisions indicate where geotechnical engineers' responsibilities begin and end, to help others recognize their own responsibilities and risks. *Read these provisions closely.* Ask questions. Your geotechnical engineer should respond fully and frankly.

## Geoenvironmental Concerns Are Not Covered

The personnel, equipment, and techniques used to perform an environmental study – e.g., a "phase-one" or "phase-two" environmental site assessment – differ significantly from those used to perform a geotechnical-engineering study. For that reason, a geotechnical-engineering report does not usually provide environmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. *Unanticipated subsurface environmental problems have led to project failures.* If you have not obtained your own environmental information about the project site, ask your geotechnical consultant for a recommendation on how to find environmental risk-management guidance.

## Obtain Professional Assistance to Deal with Moisture Infiltration and Mold

While your geotechnical engineer may have addressed groundwater, water infiltration, or similar issues in this report, the engineer's services were not designed, conducted, or intended to prevent migration of moisture – including water vapor – from the soil through building slabs and walls and into the building interior, where it can cause mold growth and material-performance deficiencies. Accordingly, ***proper implementation of the geotechnical engineer's recommendations will not of itself be sufficient to prevent moisture infiltration. Confront the risk of moisture infiltration*** by including building-envelope or mold specialists on the design team. ***Geotechnical engineers are not building-envelope or mold specialists.***

GEOPROFESSIONAL BUSINESS ASSOCIATION

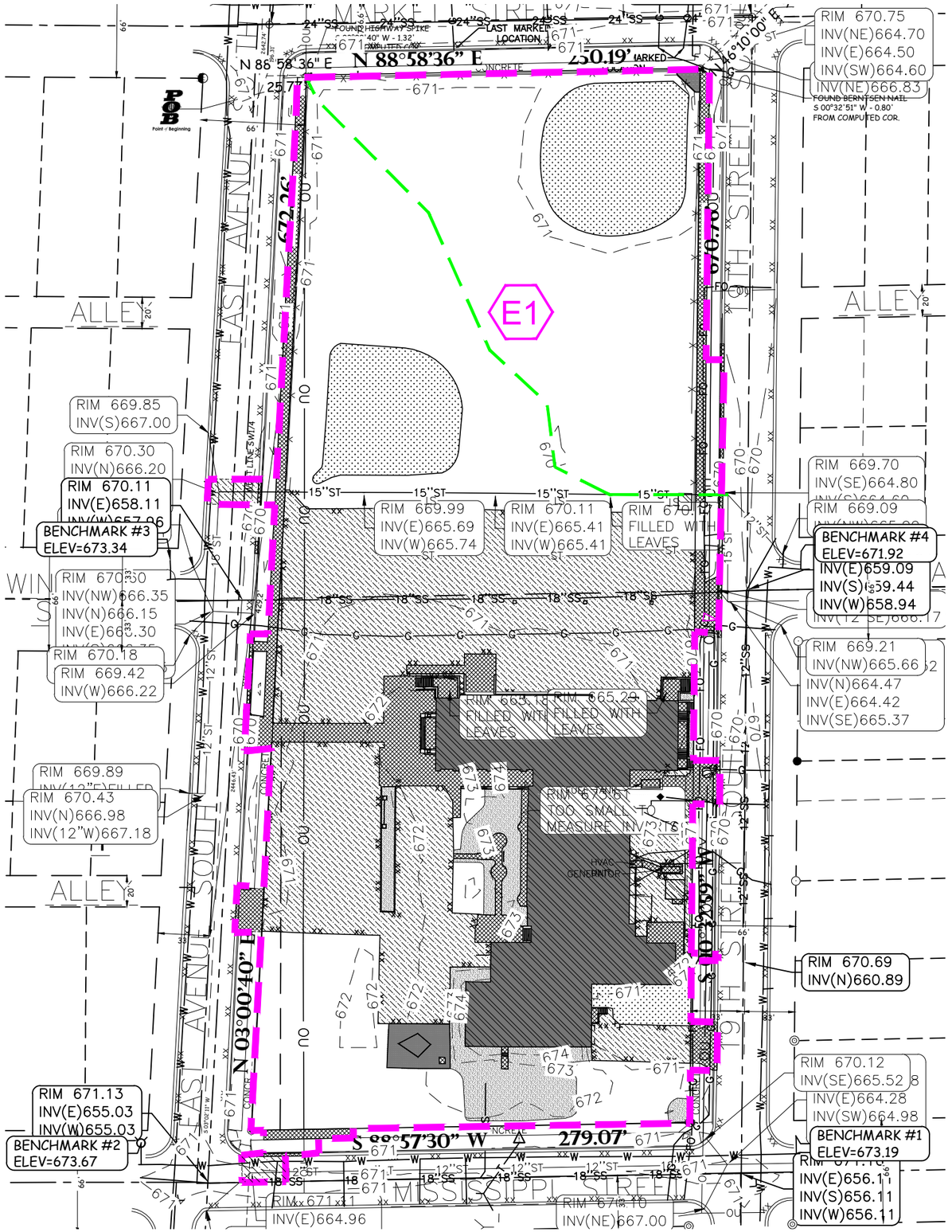
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# APPENDIX C

## Existing Drainage Map and Calculations




**LEGEND:**

EXISTING DRAINAGE AREA LIMITS

EXISTING Tc FLOW PATH

EXISTING DRAINAGE AREA LABEL — E1

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REVISIONS

| | |
|---|---|
| CHECKED: | JL |
| DRAWN: | MK |
| DATE | 11/04/2025 |
| PROJECT NO. | 25.0034 |

# EXISTING DRAINAGE

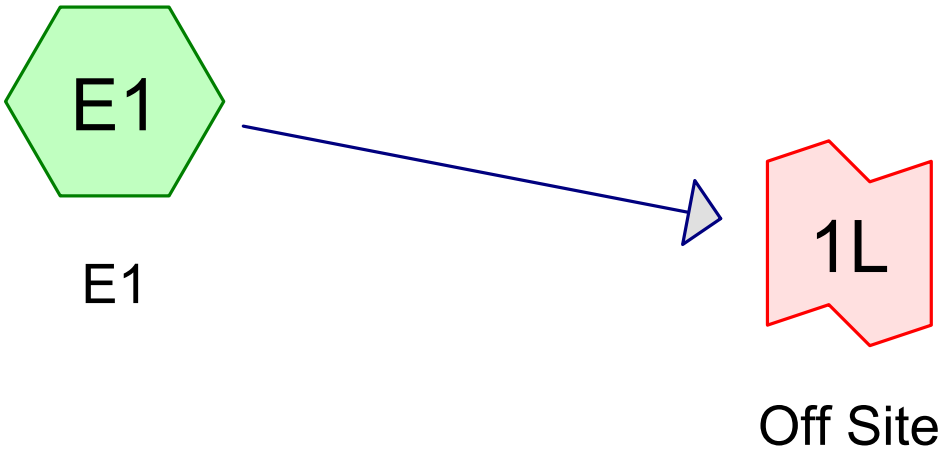
**LACROSSE SCHOOL DISTRICT**
**NEW ELEMENTARY SCHOOL**
**CITY OF LACROSSE**
**LACROSSE CO, WISCONSIN**

Civil Engineering
Land Surveying
Landscape Architecture

4941 Kirschling Court
Stevens Point, WI 54481
Phone: 715.344.9999 Fax: 715.344.9922

POB Point of Beginning

SHEET EX




Subcat Reach Pond Link

**Routing Diagram for 25.0034 - EXISTING**
Prepared by Point of Beginning, Inc., Printed 10/14/2025
HydroCAD® 10.10-6a s/n 05316 © 2020 HydroCAD Software Solutions LLC

## Pipe Listing (all nodes)

| Line# | Node Number | In-Invert (feet) | Out-Invert (feet) | Length (feet) | Slope (ft/ft) | n | Width (inches) | Diam/Height (inches) | Inside-Fill (inches) |
|---|---|---|---|---|---|---|---|---|---|
| 1 | E1 | 0.00 | 0.00 | 76.0 | 0.0048 | 0.013 | 0.0 | 15.0 | 0.0 |

Time span=1.00-48.00 hrs, dt=0.01 hrs, 4701 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**SubcatchmentE1: E1** Runoff Area=187,985 sf 47.29% Impervious Runoff Depth=0.81"
Flow Length=378' Tc=68.4 min CN=72 Runoff=1.53 cfs 12,752 cf

**Link 1L: Off Site** Inflow=1.53 cfs 12,752 cf
Primary=1.53 cfs 12,752 cf

**Total Runoff Area = 187,985 sf Runoff Volume = 12,752 cf Average Runoff Depth = 0.81"**
**52.71% Pervious = 99,094 sf 47.29% Impervious = 88,891 sf**

## Summary for Subcatchment E1: E1

Runoff = 1.53 cfs @ 13.00 hrs, Volume= 12,752 cf, Depth= 0.81"
Routed to Link 1L : Off Site

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs
MSE 24-hr 4 2-Year Rainfall=3.01"

| | Area (sf) | CN | Description |
|---|---|---|---|
| | 57,825 | 98 | Paved parking, HSG B |
| * | 10,378 | 98 | Concrete pavement, HSG B |
| | 20,688 | 98 | Roofs, HSG B |
| * | 14,649 | 96 | Baseball Infield |
| * | 1,116 | 77 | Soft Surface Play Area |
| | 83,329 | 39 | >75% Grass cover, Good, HSG A |
| | 187,985 | 72 | Weighted Average |
| | 99,094 | | 52.71% Pervious Area |
| | 88,891 | | 47.29% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|---|---|---|---|---|---|
| 1.8 | 17 | 0.0454 | 0.16 | | **Sheet Flow,** Grass: Short n= 0.150 P2= 2.94" |
| 17.8 | 100 | 0.0050 | 0.09 | | **Sheet Flow,** Grass: Short n= 0.150 P2= 2.94" |
| 35.9 | 96 | 0.0008 | 0.04 | | **Sheet Flow,** Grass: Short n= 0.150 P2= 2.94" |
| 11.9 | 50 | 0.0034 | 0.07 | | **Sheet Flow,** Grass: Short n= 0.150 P2= 2.94" |
| 0.7 | 39 | 0.0043 | 0.98 | | **Shallow Concentrated Flow,** Grassed Waterway Kv= 15.0 fps |
| 0.3 | 76 | 0.0048 | 3.65 | 4.48 | **Pipe Channel, CMP_Round 15"** 15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31' n= 0.013 Concrete pipe, bends & connections |
| 68.4 | 378 | Total | | | |

## Summary for Link 1L: Off Site

Inflow Area = 187,985 sf, 47.29% Impervious, Inflow Depth = 0.81" for 2-Year event
Inflow = 1.53 cfs @ 13.00 hrs, Volume= 12,752 cf
Primary = 1.53 cfs @ 13.00 hrs, Volume= 12,752 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs

Time span=1.00-48.00 hrs, dt=0.01 hrs, 4701 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**SubcatchmentE1: E1** Runoff Area=187,985 sf 47.29% Impervious Runoff Depth=1.79"
Flow Length=378' Tc=68.4 min CN=72 Runoff=3.65 cfs 28,054 cf

**Link 1L: Off Site** Inflow=3.65 cfs 28,054 cf
Primary=3.65 cfs 28,054 cf

**Total Runoff Area = 187,985 sf Runoff Volume = 28,054 cf Average Runoff Depth = 1.79"**
**52.71% Pervious = 99,094 sf 47.29% Impervious = 88,891 sf**

## Summary for Subcatchment E1: E1

Runoff = 3.65 cfs @ 12.99 hrs, Volume= 28,054 cf, Depth= 1.79"
Routed to Link 1L : Off Site

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs
MSE 24-hr 4 10-Year Rainfall=4.46"

| | Area (sf) | CN | Description |
|---|---|---|---|
| | 57,825 | 98 | Paved parking, HSG B |
| * | 10,378 | 98 | Concrete pavement, HSG B |
| | 20,688 | 98 | Roofs, HSG B |
| * | 14,649 | 96 | Baseball Infield |
| * | 1,116 | 77 | Soft Surface Play Area |
| | 83,329 | 39 | >75% Grass cover, Good, HSG A |
| | 187,985 | 72 | Weighted Average |
| | 99,094 | | 52.71% Pervious Area |
| | 88,891 | | 47.29% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|---|---|---|---|---|---|
| 1.8 | 17 | 0.0454 | 0.16 | | **Sheet Flow,** Grass: Short n= 0.150 P2= 2.94" |
| 17.8 | 100 | 0.0050 | 0.09 | | **Sheet Flow,** Grass: Short n= 0.150 P2= 2.94" |
| 35.9 | 96 | 0.0008 | 0.04 | | **Sheet Flow,** Grass: Short n= 0.150 P2= 2.94" |
| 11.9 | 50 | 0.0034 | 0.07 | | **Sheet Flow,** Grass: Short n= 0.150 P2= 2.94" |
| 0.7 | 39 | 0.0043 | 0.98 | | **Shallow Concentrated Flow,** Grassed Waterway Kv= 15.0 fps |
| 0.3 | 76 | 0.0048 | 3.65 | 4.48 | **Pipe Channel, CMP_Round 15"** 15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31' n= 0.013 Concrete pipe, bends & connections |
| 68.4 | 378 | Total | | | |

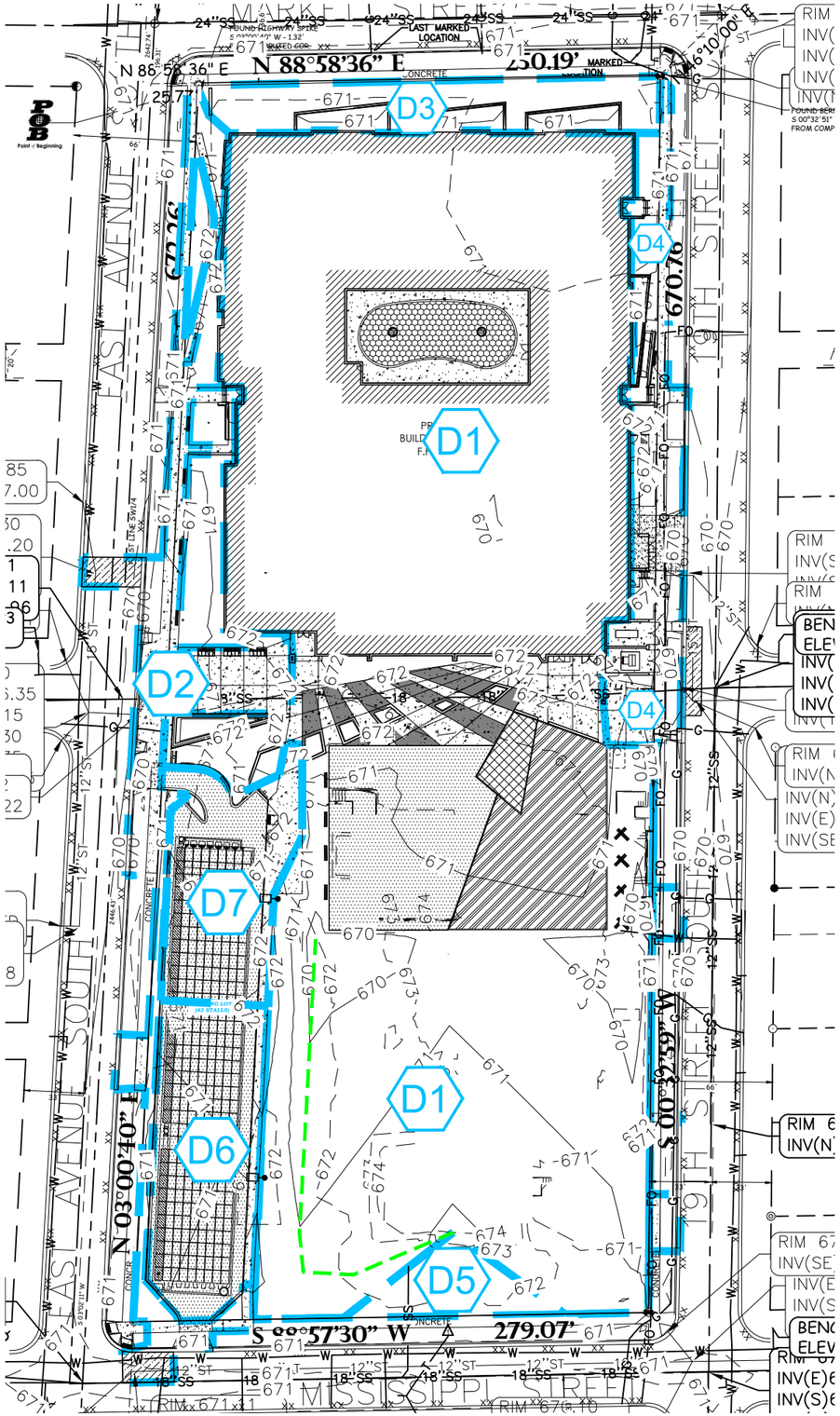
## Summary for Link 1L: Off Site

Inflow Area = 187,985 sf, 47.29% Impervious, Inflow Depth = 1.79" for 10-Year event
Inflow = 3.65 cfs @ 12.99 hrs, Volume= 28,054 cf
Primary = 3.65 cfs @ 12.99 hrs, Volume= 28,054 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs

# APPENDIX D

## Proposed Drainage Map and Calculations




**LEGEND:**

PROPOSED DRAINAGE AREA LIMITS

PROPOSED Tc FLOW PATH

PROPOSED DRAINAGE AREA LABEL D1

REVISIONS

| | |
|---|---|
| CHECKED: | JL |
| DRAWN: | MK |
| DATE | 11/04/2025 |
| PROJECT NO. | 25.0034 |

**PROPOSED DRAINAGE**

**LACROSSE SCHOOL DISTRICT**
**NEW ELEMENTARY SCHOOL**
**CITY OF LACROSSE**
**LACROSSE CO, WISCONSIN**


Civil Engineering
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Landscape Architecture

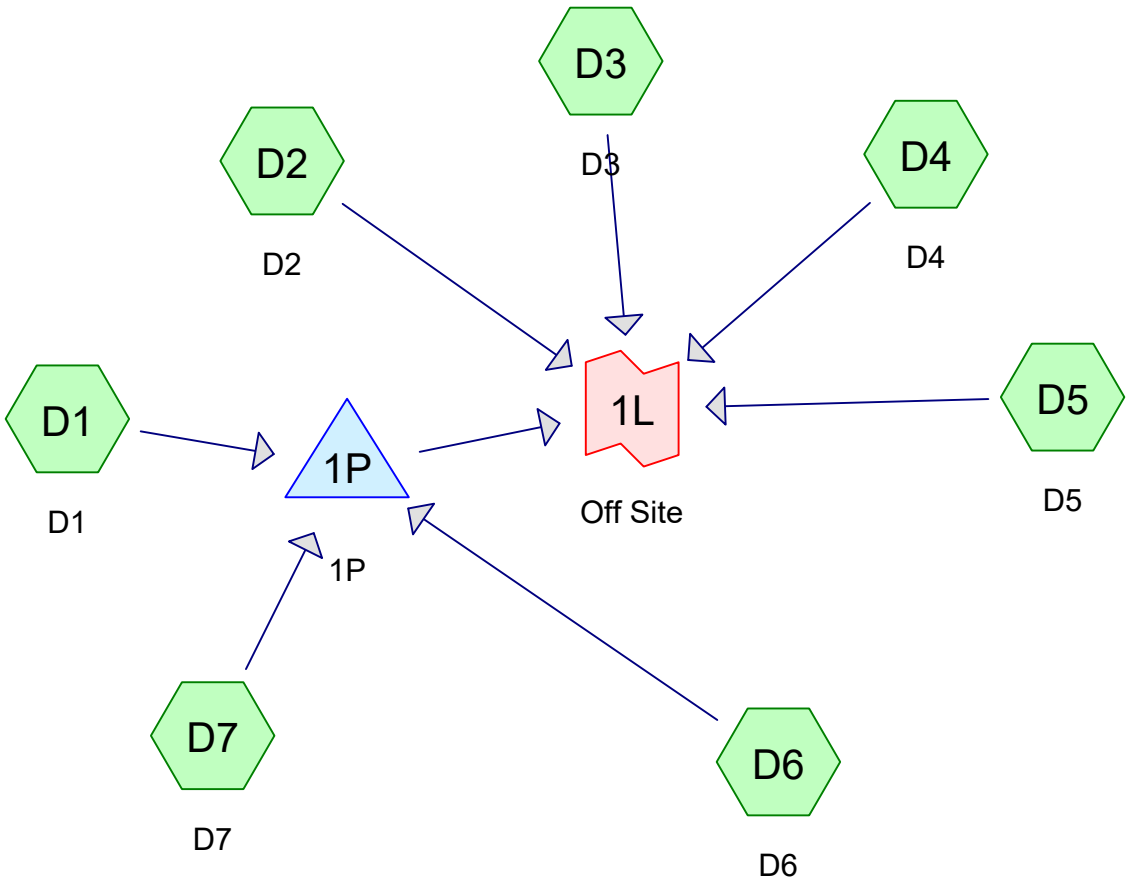
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Subcat Reach Pond Link

**Routing Diagram for ADS Update25.0034 - PROPOSED-5-30-25**
Prepared by Point of Beginning, Inc., Printed 10/14/2025
HydroCAD® 10.10-6a s/n 05316 © 2020 HydroCAD Software Solutions LLC

## Pipe Listing (all nodes)

| Line# | Node Number | In-Invert (feet) | Out-Invert (feet) | Length (feet) | Slope (ft/ft) | n | Width (inches) | Diam/Height (inches) | Inside-Fill (inches) |
|---|---|---|---|---|---|---|---|---|---|
| 1 | 1P | 665.26 | 665.07 | 52.0 | 0.0037 | 0.012 | 0.0 | 12.0 | 0.0 |

Time span=1.00-48.00 hrs, dt=0.01 hrs, 4701 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**SubcatchmentD1: D1** Runoff Area=138,302 sf 56.09% Impervious Runoff Depth=0.91"
Flow Length=266' Slope=0.0109 '/' Tc=28.5 min CN=74 Runoff=2.25 cfs 10,541 cf

**SubcatchmentD2: D2** Runoff Area=12,328 sf 63.60% Impervious Runoff Depth=1.08"
Tc=6.0 min CN=77 Runoff=0.49 cfs 1,107 cf

**SubcatchmentD3: D3** Runoff Area=7,013 sf 0.00% Impervious Runoff Depth=0.00"
Tc=6.0 min CN=39 Runoff=0.00 cfs 0 cf

**SubcatchmentD4: D4** Runoff Area=8,494 sf 69.74% Impervious Runoff Depth=1.26"
Tc=6.0 min CN=80 Runoff=0.40 cfs 890 cf

**SubcatchmentD5: D5** Runoff Area=4,311 sf 27.28% Impervious Runoff Depth=0.15"
Flow Length=93' Slope=0.0294 '/' Tc=8.3 min UI Adjusted CN=53 Runoff=0.00 cfs 54 cf

**SubcatchmentD6: D6** Runoff Area=9,583 sf 100.00% Impervious Runoff Depth=2.78"
Tc=6.0 min CN=98 Runoff=0.86 cfs 2,219 cf

**SubcatchmentD7: D7** Runoff Area=7,972 sf 97.82% Impervious Runoff Depth=2.67"
Tc=6.0 min CN=97 Runoff=0.70 cfs 1,773 cf

**Pond 1P: 1P** Peak Elev=665.81' Storage=2,836 cf Inflow=2.54 cfs 14,532 cf
Discarded=0.96 cfs 13,421 cf Primary=0.21 cfs 1,112 cf Outflow=1.16 cfs 14,532 cf

**Link 1L: Off Site** Inflow=0.91 cfs 3,163 cf
Primary=0.91 cfs 3,163 cf

**Total Runoff Area = 188,003 sf Runoff Volume = 16,584 cf Average Runoff Depth = 1.06"**
**41.54% Pervious = 78,101 sf 58.46% Impervious = 109,902 sf**

## Summary for Subcatchment D1: D1

Runoff = 2.25 cfs @ 12.44 hrs, Volume= 10,541 cf, Depth= 0.91"
Routed to Pond 1P : 1P

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs
MSE 24-hr 4 2-Year Rainfall=3.01"

| | Area (sf) | CN | Description |
|---|---|---|---|
| | 8,930 | 98 | Unconnected pavement, HSG A |
| | 57,281 | 98 | Unconnected roofs, HSG A |
| * | 3,398 | 98 | Rubber pavement, HSG A |
| * | 7,971 | 98 | Paved Play Area |
| | 54,014 | 39 | >75% Grass cover, Good, HSG A |
| * | 6,708 | 77 | Soft Surface Play Area |
| | 138,302 | 74 | Weighted Average |
| | 60,722 | | 43.91% Pervious Area |
| | 77,580 | | 56.09% Impervious Area |
| | 66,211 | | 85.35% Unconnected |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|---|---|---|---|---|---|
| 28.5 | 266 | 0.0109 | 0.16 | | **Sheet Flow,** Grass: Short n= 0.150 P2= 2.94" |

## Summary for Subcatchment D2: D2

Runoff = 0.49 cfs @ 12.14 hrs, Volume= 1,107 cf, Depth= 1.08"
Routed to Link 1L : Off Site

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs
MSE 24-hr 4 2-Year Rainfall=3.01"

| Area (sf) | CN | Description |
|---|---|---|
| 566 | 98 | Paved parking, HSG A |
| 7,275 | 98 | Unconnected pavement, HSG A |
| 4,487 | 39 | >75% Grass cover, Good, HSG A |
| 12,328 | 77 | Weighted Average |
| 4,487 | | 36.40% Pervious Area |
| 7,841 | | 63.60% Impervious Area |
| 7,275 | | 92.78% Unconnected |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|---|---|---|---|---|---|
| 6.0 | | | | | **Direct Entry,** |

## Summary for Subcatchment D3: D3

Runoff = 0.00 cfs @ 1.00 hrs, Volume= 0 cf, Depth= 0.00"
Routed to Link 1L : Off Site

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs
MSE 24-hr 4 2-Year Rainfall=3.01"

| Area (sf) | CN | Description |
|---|---|---|
| 7,013 | 39 | >75% Grass cover, Good, HSG A |
| 7,013 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|---|---|---|---|---|---|
| 6.0 | | | | | **Direct Entry,** |

## Summary for Subcatchment D4: D4

Runoff = 0.40 cfs @ 12.14 hrs, Volume= 890 cf, Depth= 1.26"
Routed to Link 1L : Off Site

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs
MSE 24-hr 4 2-Year Rainfall=3.01"

| Area (sf) | CN | Description |
|---|---|---|
| 1,089 | 98 | Paved parking, HSG A |
| 4,835 | 98 | Unconnected pavement, HSG A |
| 2,570 | 39 | >75% Grass cover, Good, HSG A |
| 8,494 | 80 | Weighted Average |
| 2,570 | | 30.26% Pervious Area |
| 5,924 | | 69.74% Impervious Area |
| 4,835 | | 81.62% Unconnected |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|---|---|---|---|---|---|
| 6.0 | | | | | **Direct Entry,** |

## Summary for Subcatchment D5: D5

Runoff = 0.00 cfs @ 12.42 hrs, Volume= 54 cf, Depth= 0.15"
Routed to Link 1L : Off Site

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs
MSE 24-hr 4 2-Year Rainfall=3.01"

| Area (sf) | CN | Adj | Description |
|---|---|---|---|
| 828 | 98 | | Paved parking, HSG A |
| 3,135 | 39 | | >75% Grass cover, Good, HSG A |
| 348 | 98 | | Unconnected pavement, HSG A |
| 4,311 | 55 | 53 | Weighted Average, UI Adjusted |
| 3,135 | | | 72.72% Pervious Area |
| 1,176 | | | 27.28% Impervious Area |
| 348 | | | 29.59% Unconnected |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|---|---|---|---|---|---|
| 8.3 | 93 | 0.0294 | 0.19 | | **Sheet Flow,** Grass: Short n= 0.150 P2= 2.94" |

## Summary for Subcatchment D6: D6

Runoff = 0.86 cfs @ 12.13 hrs, Volume= 2,219 cf, Depth= 2.78"
Routed to Pond 1P : 1P

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs
MSE 24-hr 4 2-Year Rainfall=3.01"

| Area (sf) | CN | Description |
|---|---|---|
| 8,668 | 98 | Paved parking, HSG A |
| 915 | 98 | Unconnected pavement, HSG A |
| 9,583 | 98 | Weighted Average |
| 9,583 | | 100.00% Impervious Area |
| 915 | | 9.55% Unconnected |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|---|---|---|---|---|---|
| 6.0 | | | | | **Direct Entry,** |

## Summary for Subcatchment D7: D7

Runoff = 0.70 cfs @ 12.13 hrs, Volume= 1,773 cf, Depth= 2.67"
Routed to Pond 1P : 1P

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs
MSE 24-hr 4 2-Year Rainfall=3.01"

| Area (sf) | CN | Description |
|---|---|---|
| 6,186 | 98 | Paved parking, HSG A |
| 1,612 | 98 | Unconnected pavement, HSG B |
| 174 | 39 | >75% Grass cover, Good, HSG A |
| 7,972 | 97 | Weighted Average |
| 174 | | 2.18% Pervious Area |
| 7,798 | | 97.82% Impervious Area |
| 1,612 | | 20.67% Unconnected |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|---|---|---|---|---|---|
| 6.0 | | | | | **Direct Entry,** |

## Summary for Pond 1P: 1P

Inflow Area = 155,857 sf, 60.93% Impervious, Inflow Depth = 1.12" for 2-Year event
Inflow = 2.54 cfs @ 12.42 hrs, Volume= 14,532 cf
Outflow = 1.16 cfs @ 12.87 hrs, Volume= 14,532 cf, Atten= 54%, Lag= 27.1 min
Discarded = 0.96 cfs @ 12.04 hrs, Volume= 13,421 cf
Primary = 0.21 cfs @ 12.87 hrs, Volume= 1,112 cf
Routed to Link 1L : Off Site

Routing by Stor-Ind method, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs / 2
Peak Elev= 665.81' @ 12.87 hrs Surf.Area= 11,502 sf Storage= 2,836 cf

Plug-Flow detention time= 16.9 min calculated for 14,529 cf (100% of inflow)
Center-of-Mass det. time= 16.9 min ( 854.5 - 837.5 )

| Volume | Invert | Avail.Storage | Storage Description |
|---|---|---|---|
| #1A | 665.26' | 10,149 cf | **44.25'W x 259.94'L x 3.50'H Field A**<br>40,258 cf Overall - 14,885 cf Embedded = 25,373 cf x 40.0% Voids |
| #2A | 665.76' | 14,885 cf | **ADS_StormTech SC-740 +Cap** x 324 Inside #1<br>Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf<br>Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap<br>324 Chambers in 9 Rows |
| | | 25,034 cf | Total Available Storage |

Storage Group A created with Chamber Wizard

| Device | Routing | Invert | Outlet Devices |
|---|---|---|---|
| #1 | Primary | 665.26' | **12.0" Round Culvert**<br>L= 52.0' CPP, square edge headwall, Ke= 0.500<br>Inlet / Outlet Invert= 665.26' / 665.07' S= 0.0037 '/' Cc= 0.900<br>n= 0.012, Flow Area= 0.79 sf |
| #2 | Device 1 | 665.26' | **3.5" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads |
| #3 | Device 1 | 666.01' | **6.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads |
| #4 | Device 1 | 668.25' | **2.5' long Sharp-Crested Rectangular Weir** 2 End Contraction(s) |
| #5 | Discarded | 665.26' | **3.600 in/hr Exfiltration over Surface area from 660.26' - 665.26'**<br>Excluded Surface area = 0 sf |

**Discarded OutFlow** Max=0.96 cfs @ 12.04 hrs HW=665.30' (Free Discharge)
└**5=Exfiltration** (Exfiltration Controls 0.96 cfs)

**Primary OutFlow** Max=0.21 cfs @ 12.87 hrs HW=665.81' (Free Discharge)
└**1=Culvert** (Passes 0.21 cfs of 0.80 cfs potential flow)
  ├**2=Orifice/Grate** (Orifice Controls 0.21 cfs @ 3.08 fps)
  ├**3=Orifice/Grate** ( Controls 0.00 cfs)
  └**4=Sharp-Crested Rectangular Weir** ( Controls 0.00 cfs)

## Summary for Link 1L: Off Site

Inflow Area = 188,003 sf, 58.46% Impervious, Inflow Depth = 0.20" for 2-Year event
Inflow = 0.91 cfs @ 12.14 hrs, Volume= 3,163 cf
Primary = 0.91 cfs @ 12.14 hrs, Volume= 3,163 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs

Time span=1.00-48.00 hrs, dt=0.01 hrs, 4701 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**SubcatchmentD1: D1** Runoff Area=138,302 sf 56.09% Impervious Runoff Depth=1.94"
Flow Length=266' Slope=0.0109 '/' Tc=28.5 min CN=74 Runoff=5.03 cfs 22,378 cf

**SubcatchmentD2: D2** Runoff Area=12,328 sf 63.60% Impervious Runoff Depth=2.18"
Tc=6.0 min CN=77 Runoff=1.00 cfs 2,238 cf

**SubcatchmentD3: D3** Runoff Area=7,013 sf 0.00% Impervious Runoff Depth=0.10"
Tc=6.0 min CN=39 Runoff=0.00 cfs 61 cf

**SubcatchmentD4: D4** Runoff Area=8,494 sf 69.74% Impervious Runoff Depth=2.43"
Tc=6.0 min CN=80 Runoff=0.77 cfs 1,718 cf

**SubcatchmentD5: D5** Runoff Area=4,311 sf 27.28% Impervious Runoff Depth=0.62"
Flow Length=93' Slope=0.0294 '/' Tc=8.3 min UI Adjusted CN=53 Runoff=0.07 cfs 224 cf

**SubcatchmentD6: D6** Runoff Area=9,583 sf 100.00% Impervious Runoff Depth=4.22"
Tc=6.0 min CN=98 Runoff=1.28 cfs 3,373 cf

**SubcatchmentD7: D7** Runoff Area=7,972 sf 97.82% Impervious Runoff Depth=4.11"
Tc=6.0 min CN=97 Runoff=1.06 cfs 2,730 cf

**Pond 1P: 1P** Peak Elev=666.47' Storage=9,116 cf Inflow=5.50 cfs 28,481 cf
Discarded=0.96 cfs 22,671 cf Primary=0.77 cfs 5,810 cf Outflow=1.73 cfs 28,481 cf

**Link 1L: Off Site** Inflow=1.95 cfs 10,051 cf
Primary=1.95 cfs 10,051 cf

**Total Runoff Area = 188,003 sf Runoff Volume = 32,722 cf Average Runoff Depth = 2.09"**
**41.54% Pervious = 78,101 sf 58.46% Impervious = 109,902 sf**

## Summary for Subcatchment D1: D1

Runoff = 5.03 cfs @ 12.41 hrs, Volume= 22,378 cf, Depth= 1.94"
Routed to Pond 1P : 1P

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs
MSE 24-hr 4 10-Year Rainfall=4.46"

| | Area (sf) | CN | Description |
|---|---|---|---|
| | 8,930 | 98 | Unconnected pavement, HSG A |
| | 57,281 | 98 | Unconnected roofs, HSG A |
| * | 3,398 | 98 | Rubber pavement, HSG A |
| * | 7,971 | 98 | Paved Play Area |
| | 54,014 | 39 | >75% Grass cover, Good, HSG A |
| * | 6,708 | 77 | Soft Surface Play Area |
| | 138,302 | 74 | Weighted Average |
| | 60,722 | | 43.91% Pervious Area |
| | 77,580 | | 56.09% Impervious Area |
| | 66,211 | | 85.35% Unconnected |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|---|---|---|---|---|---|
| 28.5 | 266 | 0.0109 | 0.16 | | **Sheet Flow,** Grass: Short n= 0.150 P2= 2.94" |

## Summary for Subcatchment D2: D2

Runoff = 1.00 cfs @ 12.13 hrs, Volume= 2,238 cf, Depth= 2.18"
Routed to Link 1L : Off Site

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs
MSE 24-hr 4 10-Year Rainfall=4.46"

| Area (sf) | CN | Description |
|---|---|---|
| 566 | 98 | Paved parking, HSG A |
| 7,275 | 98 | Unconnected pavement, HSG A |
| 4,487 | 39 | >75% Grass cover, Good, HSG A |
| 12,328 | 77 | Weighted Average |
| 4,487 | | 36.40% Pervious Area |
| 7,841 | | 63.60% Impervious Area |
| 7,275 | | 92.78% Unconnected |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|---|---|---|---|---|---|
| 6.0 | | | | | **Direct Entry,** |

## Summary for Subcatchment D3: D3

Runoff = 0.00 cfs @ 13.24 hrs, Volume= 61 cf, Depth= 0.10"
Routed to Link 1L : Off Site

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs
MSE 24-hr 4 10-Year Rainfall=4.46"

| Area (sf) | CN | Description |
|---|---|---|
| 7,013 | 39 | >75% Grass cover, Good, HSG A |
| 7,013 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|---|---|---|---|---|---|
| 6.0 | | | | | **Direct Entry,** |

## Summary for Subcatchment D4: D4

Runoff = 0.77 cfs @ 12.13 hrs, Volume= 1,718 cf, Depth= 2.43"
Routed to Link 1L : Off Site

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs
MSE 24-hr 4 10-Year Rainfall=4.46"

| Area (sf) | CN | Description |
|---|---|---|
| 1,089 | 98 | Paved parking, HSG A |
| 4,835 | 98 | Unconnected pavement, HSG A |
| 2,570 | 39 | >75% Grass cover, Good, HSG A |
| 8,494 | 80 | Weighted Average |
| 2,570 | | 30.26% Pervious Area |
| 5,924 | | 69.74% Impervious Area |
| 4,835 | | 81.62% Unconnected |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|---|---|---|---|---|---|
| 6.0 | | | | | **Direct Entry,** |

## Summary for Subcatchment D5: D5

Runoff = 0.07 cfs @ 12.18 hrs, Volume= 224 cf, Depth= 0.62"
Routed to Link 1L : Off Site

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs
MSE 24-hr 4 10-Year Rainfall=4.46"

| Area (sf) | CN | Adj | Description |
|---|---|---|---|
| 828 | 98 | | Paved parking, HSG A |
| 3,135 | 39 | | >75% Grass cover, Good, HSG A |
| 348 | 98 | | Unconnected pavement, HSG A |
| 4,311 | 55 | 53 | Weighted Average, UI Adjusted |
| 3,135 | | | 72.72% Pervious Area |
| 1,176 | | | 27.28% Impervious Area |
| 348 | | | 29.59% Unconnected |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|---|---|---|---|---|---|
| 8.3 | 93 | 0.0294 | 0.19 | | **Sheet Flow,** Grass: Short n= 0.150 P2= 2.94" |

## Summary for Subcatchment D6: D6

Runoff = 1.28 cfs @ 12.13 hrs, Volume= 3,373 cf, Depth= 4.22"
Routed to Pond 1P : 1P

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs
MSE 24-hr 4 10-Year Rainfall=4.46"

| Area (sf) | CN | Description |
|---|---|---|
| 8,668 | 98 | Paved parking, HSG A |
| 915 | 98 | Unconnected pavement, HSG A |
| 9,583 | 98 | Weighted Average |
| 9,583 | | 100.00% Impervious Area |
| 915 | | 9.55% Unconnected |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|---|---|---|---|---|---|
| 6.0 | | | | | **Direct Entry,** |

## Summary for Subcatchment D7: D7

Runoff = 1.06 cfs @ 12.13 hrs, Volume= 2,730 cf, Depth= 4.11"
Routed to Pond 1P : 1P

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs
MSE 24-hr 4 10-Year Rainfall=4.46"

| Area (sf) | CN | Description |
|---|---|---|
| 6,186 | 98 | Paved parking, HSG A |
| 1,612 | 98 | Unconnected pavement, HSG B |
| 174 | 39 | >75% Grass cover, Good, HSG A |
| 7,972 | 97 | Weighted Average |
| 174 | | 2.18% Pervious Area |
| 7,798 | | 97.82% Impervious Area |
| 1,612 | | 20.67% Unconnected |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|---|---|---|---|---|---|
| 6.0 | | | | | **Direct Entry,** |

## Summary for Pond 1P: 1P

Inflow Area = 155,857 sf, 60.93% Impervious, Inflow Depth = 2.19" for 10-Year event
Inflow = 5.50 cfs @ 12.39 hrs, Volume= 28,481 cf
Outflow = 1.73 cfs @ 13.02 hrs, Volume= 28,481 cf, Atten= 69%, Lag= 38.1 min
Discarded = 0.96 cfs @ 11.86 hrs, Volume= 22,671 cf
Primary = 0.77 cfs @ 13.02 hrs, Volume= 5,810 cf
Routed to Link 1L : Off Site

Routing by Stor-Ind method, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs / 2
Peak Elev= 666.47' @ 13.02 hrs Surf.Area= 11,502 sf Storage= 9,116 cf

Plug-Flow detention time= 48.7 min calculated for 28,475 cf (100% of inflow)
Center-of-Mass det. time= 48.7 min ( 876.0 - 827.3 )

| Volume | Invert | Avail.Storage | Storage Description |
|---|---|---|---|
| #1A | 665.26' | 10,149 cf | **44.25'W x 259.94'L x 3.50'H Field A**<br>40,258 cf Overall - 14,885 cf Embedded = 25,373 cf x 40.0% Voids |
| #2A | 665.76' | 14,885 cf | **ADS_StormTech SC-740 +Cap** x 324 Inside #1<br>Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf<br>Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap<br>324 Chambers in 9 Rows |
| | | 25,034 cf | Total Available Storage |

Storage Group A created with Chamber Wizard

| Device | Routing | Invert | Outlet Devices |
|---|---|---|---|
| #1 | Primary | 665.26' | **12.0" Round Culvert**<br>L= 52.0' CPP, square edge headwall, Ke= 0.500<br>Inlet / Outlet Invert= 665.26' / 665.07' S= 0.0037 '/' Cc= 0.900<br>n= 0.012, Flow Area= 0.79 sf |
| #2 | Device 1 | 665.26' | **3.5" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads |
| #3 | Device 1 | 666.01' | **6.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads |
| #4 | Device 1 | 668.25' | **2.5' long Sharp-Crested Rectangular Weir** 2 End Contraction(s) |
| #5 | Discarded | 665.26' | **3.600 in/hr Exfiltration over Surface area from 660.26' - 665.26'**<br>Excluded Surface area = 0 sf |

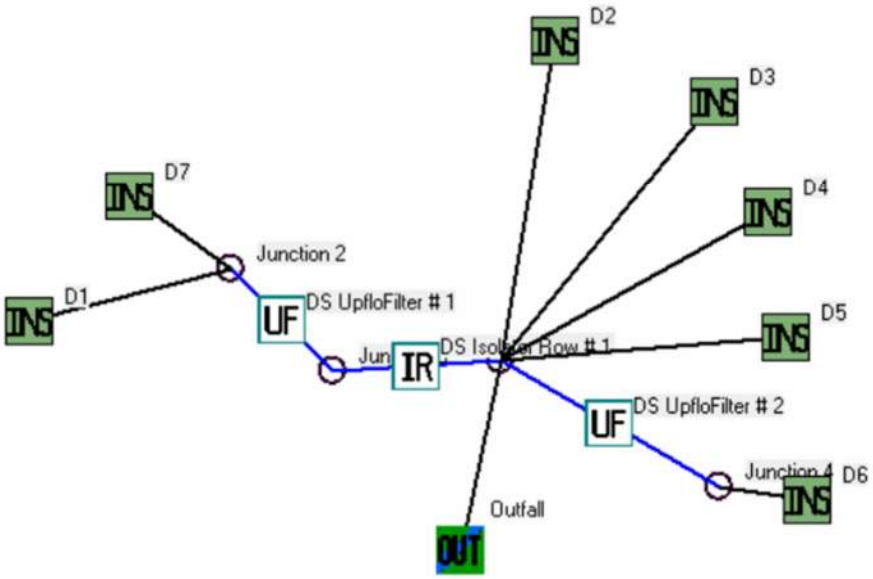
**Discarded OutFlow** Max=0.96 cfs @ 11.86 hrs HW=665.30' (Free Discharge)
└**5=Exfiltration** (Exfiltration Controls 0.96 cfs)

**Primary OutFlow** Max=0.77 cfs @ 13.02 hrs HW=666.47' (Free Discharge)
└**1=Culvert** (Passes 0.77 cfs of 2.62 cfs potential flow)
├**2=Orifice/Grate** (Orifice Controls 0.33 cfs @ 4.97 fps)
├**3=Orifice/Grate** (Orifice Controls 0.44 cfs @ 2.32 fps)
└**4=Sharp-Crested Rectangular Weir** ( Controls 0.00 cfs)

## Summary for Link 1L: Off Site

Inflow Area = 188,003 sf, 58.46% Impervious, Inflow Depth = 0.64" for 10-Year event
Inflow = 1.95 cfs @ 12.14 hrs, Volume= 10,051 cf
Primary = 1.95 cfs @ 12.14 hrs, Volume= 10,051 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs


D2
INS
D3
INS
D4
INS
D5
INS
D6
INS
D7
INS
D1
INS
Junction 2
UF
DS UpfloFilter # 1
IR
DS Isolation Flow # 1
UF
DS UpfloFilter # 2
Junction 4
Outfall
OUT

Data file name:  O:\Projects\2025\25.0034 - Hogan Administrative Center - Bray\14 - Storm Water\HydroCAD and WinSLAMM Models\100925 ADS Update Model.mdb
WinSLAMM Version 10.5.0
Rain file name:  C:\WinSLAMM Files\Rain Files\WisReg - Madison WI 1981.RAN
Particulate Solids Concentration file name:  C:\WinSLAMM Files\v10.1 WI_AVG01.pscx
Runoff Coefficient file name:  C:\WinSLAMM Files\WI_SL06 Dec06.rsvx
Residential Street Delivery file name:  C:\WinSLAMM Files\WI_Res and Other Urban Dec06.std
Institutional Street Delivery file name:  C:\WinSLAMM Files\WI_Com Inst Indust Dec06.std
Commercial Street Delivery file name:  C:\WinSLAMM Files\WI_Com Inst Indust Dec06.std
Industrial Street Delivery file name:  C:\WinSLAMM Files\WI_Com Inst Indust Dec06.std
Other Urban Street Delivery file name:  C:\WinSLAMM Files\WI_Res and Other Urban Dec06.std
Freeway Street Delivery file name:  C:\WinSLAMM Files\Freeway Dec06.std
Apply Street Delivery Files to Adjust the After Event Load Street Dirt Mass Balance:  False
Pollutant Relative Concentration file name:  C:\WinSLAMM Files\WI_GEO03.ppdx
Source Area PSD and Peak to Average Flow Ratio File:  C:\WinSLAMM Files\NURP Source Area PSD Files.csv
Cost Data file name:
If Other Device Pollutant Load Reduction Values = 1, Off-site Pollutant Loads are Removed from Pollutant Load % Reduction calculations
Seed for random number generator:  -42
Study period starting date:  01/01/81  Study period ending date:  12/31/81
Start of Winter Season:  12/06  End of Winter Season:  03/28
Date:  10-14-2025  Time:  12:27:08
Site information:

LU# 1 - Institutional:  D1  Total area (ac):  3.175
  1 - Roofs 1:  1.315 ac.  Flat  Connected  Source Area PSD File: C:\WinSLAMM Files\NURP.cpz  OD-CP#4
  31 - Sidewalks 1:  0.205 ac.  Connected  Source Area PSD File: C:\WinSLAMM Files\NURP.cpz  OD-CP#5
  45 - Large Landscaped Areas 1:  1.240 ac.  Normal Sandy  Source Area PSD File: C:\WinSLAMM Files\NURP.cpz  OD-CP#7
  63 - Paved Playground 1:  0.183 ac.  Connected  Source Area PSD File: C:\WinSLAMM Files\NURP.cpz  OD-CP#8
  71 - Other Pervious Areas 1:  0.154 ac.  Normal Sandy  Source Area PSD File: C:\WinSLAMM Files\NURP.cpz  OD-CP#6
  72 - Other Pervious Areas 2:  0.078 ac.  Normal Sandy  Source Area PSD File: C:\WinSLAMM Files\NURP.cpz  OD-CP#9

LU# 2 - Institutional:  D7  Total area (ac):  0.183
  13 - Paved Parking 1:  0.142 ac.  Connected  Source Area PSD File: C:\WinSLAMM Files\NURP.cpz
  31 - Sidewalks 1:  0.037 ac.  Connected  Source Area PSD File: C:\WinSLAMM Files\NURP.cpz  OD-CP#16

```
     45 - Large Landscaped Areas 1:  0.004 ac.    Normal Sandy    Source Area PSD
File: C:\WinSLAMM Files\NURP.cpz    OD-CP#17

LU# 3 - Institutional:  D6     Total area (ac):  0.220
     13 - Paved Parking 1:  0.199 ac.    Connected    Source Area PSD File:
C:\WinSLAMM Files\NURP.cpz
     31 - Sidewalks 1:  0.021 ac.    Connected    Source Area PSD File: C:\WinSLAMM
Files\NURP.cpz

LU# 4 - Institutional:  D5     Total area (ac):  0.099
     13 - Paved Parking 1:  0.019 ac.    Connected    Source Area PSD File:
C:\WinSLAMM Files\NURP.cpz
     31 - Sidewalks 1:  0.008 ac.    Connected    Source Area PSD File: C:\WinSLAMM
Files\NURP.cpz    OD-CP#15
     45 - Large Landscaped Areas 1:  0.072 ac.    Normal Sandy    Source Area PSD
File: C:\WinSLAMM Files\NURP.cpz    OD-CP#14

LU# 5 - Institutional:  D4     Total area (ac):  0.195
     13 - Paved Parking 1:  0.025 ac.    Connected    Source Area PSD File:
C:\WinSLAMM Files\NURP.cpz
     31 - Sidewalks 1:  0.111 ac.    Connected    Source Area PSD File: C:\WinSLAMM
Files\NURP.cpz    OD-CP#12
     45 - Large Landscaped Areas 1:  0.059 ac.    Normal Sandy    Source Area PSD
File: C:\WinSLAMM Files\NURP.cpz    OD-CP#13

LU# 6 - Institutional:  D3     Total area (ac):  0.161
     45 - Large Landscaped Areas 1:  0.161 ac.    Normal Sandy    Source Area PSD
File: C:\WinSLAMM Files\NURP.cpz

LU# 7 - Institutional:  D2     Total area (ac):  0.283
     13 - Paved Parking 1:  0.013 ac.    Connected    Source Area PSD File:
C:\WinSLAMM Files\NURP.cpz
     31 - Sidewalks 1:  0.167 ac.    Connected    Source Area PSD File: C:\WinSLAMM
Files\NURP.cpz    OD-CP#10
     45 - Large Landscaped Areas 1:  0.103 ac.    Normal Sandy    Source Area PSD
File: C:\WinSLAMM Files\NURP.cpz    OD-CP#11


      Control Practice 1:  Upflo Filter CP# 1 (DS) - DS UpfloFilter # 1
         Media Type:  CPZ
         Fraction of Area Served by Upflo Filters (0-1):  1.0
         Height from Outlet Invert to Structure Top (ft):  3.0
         Sump Depth (ft):  4.00
         The program will determine the Sump Cleaning/Filter Replacement Frequency
         Solve for Given Conditions
         Number of filters:  6
      Control Practice 2:  Isolator Row CP# 1 (DS) - DS Isolator Row # 1
         Total available system length (ft) = 245
         Total available system width (ft) = 45
         Available height from chamber base to surface (ft) = 5.00
```

```
   Number of isolator rows = 1
   Native soil infiltration rate (in/hr) = 1.63
   Assumed stone porosity () = 0.40
   Sizing option:  Use all available area
   Selected Chamber Information
      Chamber type:  SC-740
      Chamber height (in):  30.00
      Chamber width (in):  51.00
      Chamber segment length (in):  85.40
      Final storage volume (cf):  25529
      Number of rows:  10
      Row length (ft):  234.9
      Total system length (ft):  2348.5
      Total system width (ft):  42.5
      Number of chambers:  330
   Overflow weir invert elevation (ft) = 2.99
   Orifice 1 invert elevation (ft) = 0.00
   Orifice 1 diameter (ft) = 0.29
   Drain Tile Present

Control Practice 3:  Upflo Filter CP# 2 (DS) - DS UpfloFilter # 2
   Media Type:  CPZ
   Fraction of Area Served by Upflo Filters (0-1):  1.0
   Height from Outlet Invert to Structure Top (ft):  3.0
   Sump Depth (ft):  4.00
   The program will determine the Sump Cleaning/Filter Replacement Frequency
   Solve for Given Conditions
   Number of filters:  2
Control Practice 4:  Other Device CP# 1 (SA) - SA Device, LU# 1 ,SA# 1
   Fraction of drainage area served by device (ac) = 1.00
   Particulate Concentration reduction fraction = 1.00
   Filterable Concentration reduction fraction = 0.00
   Runoff volume reduction fraction = 0

Control Practice 5:  Other Device CP# 2 (SA) - SA Device, LU# 1 ,SA# 31
   Fraction of drainage area served by device (ac) = 1.00
   Particulate Concentration reduction fraction = 1.00
   Filterable Concentration reduction fraction = 0.00
   Runoff volume reduction fraction = 0

Control Practice 6:  Other Device CP# 3 (SA) - SA Device, LU# 1 ,SA# 71
   Fraction of drainage area served by device (ac) = 1.00
   Particulate Concentration reduction fraction = 1.00
   Filterable Concentration reduction fraction = 0.00
   Runoff volume reduction fraction = 0

Control Practice 7:  Other Device CP# 4 (SA) - SA Device, LU# 1 ,SA# 45
   Fraction of drainage area served by device (ac) = 1.00
   Particulate Concentration reduction fraction = 1.00
   Filterable Concentration reduction fraction = 0.00
```

```
   Runoff volume reduction fraction = 0

Control Practice 8:  Other Device CP# 5 (SA) - SA Device, LU# 1 ,SA# 63
   Fraction of drainage area served by device (ac) = 1.00
   Particulate Concentration reduction fraction = 1.00
   Filterable Concentration reduction fraction = 0.00
   Runoff volume reduction fraction = 0

Control Practice 9:  Other Device CP# 6 (SA) - SA Device, LU# 1 ,SA# 72
   Fraction of drainage area served by device (ac) = 1.00
   Particulate Concentration reduction fraction = 1.00
   Filterable Concentration reduction fraction = 0.00
   Runoff volume reduction fraction = 0

Control Practice 10:  Other Device CP# 7 (SA) - SA Device, LU# 7 ,SA# 31
   Fraction of drainage area served by device (ac) = 1.00
   Particulate Concentration reduction fraction = 1.00
   Filterable Concentration reduction fraction = 0.00
   Runoff volume reduction fraction = 0

Control Practice 11:  Other Device CP# 8 (SA) - SA Device, LU# 7 ,SA# 45
   Fraction of drainage area served by device (ac) = 1.00
   Particulate Concentration reduction fraction = 1.00
   Filterable Concentration reduction fraction = 0.00
   Runoff volume reduction fraction = 0

Control Practice 12:  Other Device CP# 9 (SA) - SA Device, LU# 5 ,SA# 31
   Fraction of drainage area served by device (ac) = 1.00
   Particulate Concentration reduction fraction = 1.00
   Filterable Concentration reduction fraction = 0.00
   Runoff volume reduction fraction = 0

Control Practice 13:  Other Device CP# 10 (SA) - SA Device, LU# 5 ,SA# 45
   Fraction of drainage area served by device (ac) = 1.00
   Particulate Concentration reduction fraction = 1.00
   Filterable Concentration reduction fraction = 0.00
   Runoff volume reduction fraction = 0

Control Practice 14:  Other Device CP# 11 (SA) - SA Device, LU# 4 ,SA# 45
   Fraction of drainage area served by device (ac) = 1.00
   Particulate Concentration reduction fraction = 1.00
   Filterable Concentration reduction fraction = 0.00
   Runoff volume reduction fraction = 0

Control Practice 15:  Other Device CP# 12 (SA) - SA Device, LU# 4 ,SA# 31
   Fraction of drainage area served by device (ac) = 1.00
   Particulate Concentration reduction fraction = 1.00
   Filterable Concentration reduction fraction = 0.00
   Runoff volume reduction fraction = 0
```

Control Practice 16:  Other Device CP# 13 (SA) - SA Device, LU# 2 ,SA# 31
   Fraction of drainage area served by device (ac) = 1.00
   Particulate Concentration reduction fraction = 1.00
   Filterable Concentration reduction fraction = 0.00
   Runoff volume reduction fraction = 0

Control Practice 17:  Other Device CP# 14 (SA) - SA Device, LU# 2 ,SA# 45
   Fraction of drainage area served by device (ac) = 1.00
   Particulate Concentration reduction fraction = 1.00
   Filterable Concentration reduction fraction = 0.00
   Runoff volume reduction fraction = 0

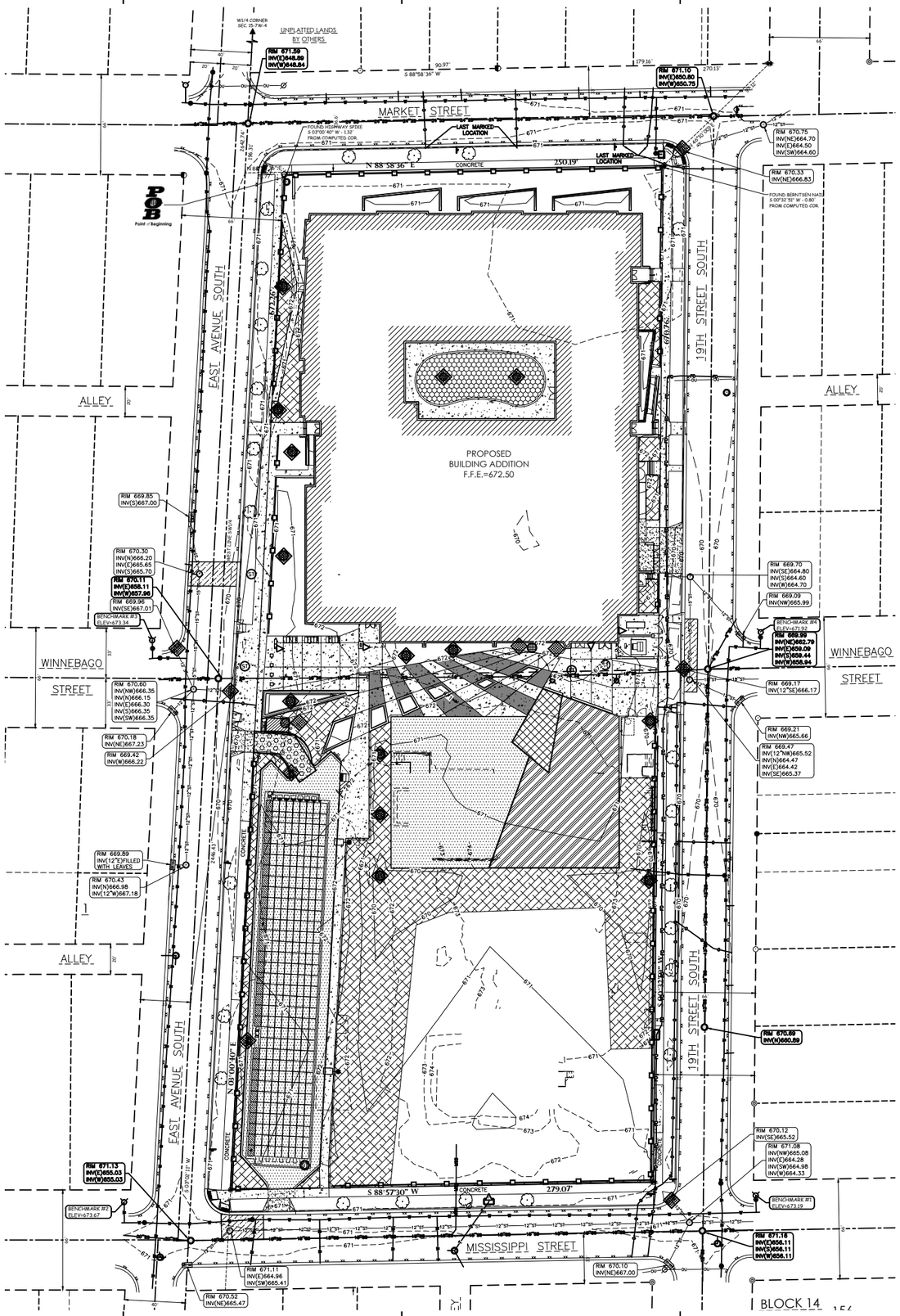
SLAMM for Windows Version 10.5.0
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Data file name:  O:\Projects\2025\25.0034 - Hogan Administrative Center - Bray\14 - Storm Water\HydroCAD and WinSLAMM Models\100925 ADS Update Model.mdb
Data file description:
Rain file name:  C:\WinSLAMM Files\Rain Files\WisReg - Madison WI 1981.RAN
Particulate Solids Concentration file name:  C:\WinSLAMM Files\v10.1 WI_AVG01.pscx
Runoff Coefficient file name:  C:\WinSLAMM Files\WI_SL06 Dec06.rsvx
Pollutant Relative Concentration file name:  C:\WinSLAMM Files\WI_GEO03.ppdx
Residential Street Delivery file name:  C:\WinSLAMM Files\WI_Res and Other Urban Dec06.std
Institutional Street Delivery file name:  C:\WinSLAMM Files\WI_Com Inst Indust Dec06.std
Commercial Street Delivery file name:  C:\WinSLAMM Files\WI_Com Inst Indust Dec06.std
Industrial Street Delivery file name:  C:\WinSLAMM Files\WI_Com Inst Indust Dec06.std
Other Urban Street Delivery file name:  C:\WinSLAMM Files\WI_Res and Other Urban Dec06.std
Freeway Street Delivery file name:  C:\WinSLAMM Files\Freeway Dec06.std
Apply Street Delivery Files to Adjust the After Event Load Street Dirt Mass Balance:  False
Source Area PSD and Peak to Average Flow Ratio File:  C:\WinSLAMM Files\NURP Source Area PSD Files.csv
Cost Data file name:
If Other Device Pollutant Load Reduction Values = 1, Off-site Pollutant Loads are Removed from Pollutant Load % Reduction calculations
Seed for random number generator:  -42
Start of Winter Season:  12/06  End of Winter Season:  03/28
Model Run Start Date:  01/01/81  Model Run End Date:  12/31/81
Date of run:  10-14-2025  Time of run:  12:27:40
Total Area Modeled (acres):  4.316
Years in Model Run:  1.00

| | Runoff Volume (cu ft) | Percent Runoff Volume Reduction | Particulate Solids Conc. (mg/L) | Particulate Solids Yield (lbs) | Percent Particulate Solids Reduction |
|---|---|---|---|---|---|
| Total of all Land Uses without Controls: | 193840 | - | 21.13 | 255.7 | - |
| Outfall Total with Controls: | 64927 | 66.50% | 18.18 | 73.69 | 71.18% |
| Annualized Total After Outfall Controls: | 65105 | | | 73.90 | |

# APPENDIX E

## Proposed Erosion Control Plan




## GENERAL NOTES:

1. CONTRACTOR SHALL LOCATE ALL PUBLIC AND PRIVATE UTILITIES PRIOR TO COMMENCEMENT OF WORK.
2. NOTIFY THE LOCAL MUNICIPALITY AT LEAST 2 WORKING DAYS PRIOR TO THE START OF SOIL DISTURBING ACTIVITIES.
3. INSTALL ALL TEMPORARY EROSION CONTROL ELEMENTS PRIOR TO THE START OF DEMOLITION/CONSTRUCTION.
4. ALL ACTIVITIES SHALL BE CONDUCTED IN A LOGICAL SEQUENCE TO MINIMIZE THE AMOUNT OF BARE SOIL EXPOSED AT ANY ONE TIME. MAINTAIN EXISTING VEGETATION AS LONG AS POSSIBLE.
5. CRUSHED ROCK DRIVES FOR SEDIMENT TRACKING UTILIZING 3" CRUSHED ROCK SHALL BE MAINTAINED AT ALL CONSTRUCTION ENTRANCES TO THE SITE. THE ROCK DRIVE SHALL BE A MINIMUM OF 12" THICK AND BE A MINIMUM OF 50 FEET IN LENGTH BY THE WIDTH OF THE DRIVEWAY.
6. OFFSITE SEDIMENT DEPOSITS RESULTING FROM STORMWATER RUNOFF SHALL BE CLEANED BY THE END OF THE NEXT WORKDAY. OFFSITE SEDIMENT DEPOSITS RESULTING FROM CONTRACTOR ACTIVITIES, INCLUDING SOIL TRACKING, SHALL BE CLEANED EACH WORKDAY. EXCESSIVE AMOUNTS OF SEDIMENT OR DEBRIS TRACKED ONTO ADJACENT STREETS SHALL BE CLEANED IMMEDIATELY. FINE SEDIMENT ACCUMULATIONS ON ADJACENT STREETS SHALL SWEPT MECHANICALLY OR MANUALLY AT LEAST WEEKLY AND BEFORE IMMINENT RAINFALL.
7. DISTURBED GROUND OUTSIDE OF THE EVERYDAY CONSTRUCTION AREAS, INCLUDING SOIL STOCKPILES, THAT ARE LEFT INACTIVE FOR MORE THAN 7 DAYS SHALL BE TEMPORARILY STABILIZED BY SEEDING/MULCHING OR OTHER APPROVED METHODS.
8. WASTE MATERIAL THAT IS GENERATED ON THE CONSTRUCTION SITE SHALL BE PROPERLY DISPOSED OF AND NOT ALLOWED TO RUN INTO RECEIVING WATERS.
9. EROSION CONTROL DEVICES DESTROYED AS A RESULT OF CONSTRUCTION ACTIVITIES SHALL BE REPAIRED BY THE END OF EACH WORK DAY.
10. INSPECT ALL EROSION CONTROL MEASURES AT LEAST ONCE A WEEK AND AFTER ANY RAINFALL OF 0.5" OR MORE. MAKE NEEDED REPAIRS AND DOCUMENT ALL ACTIVITIES AS PER THE REQUIREMENTS OF THE NOTICE OF INTENT SUBMITTED BY THE PROJECT CIVIL ENGINEER.
11. ALL TEMPORARY EROSION CONTROL ELEMENTS SHALL REMAIN IN PLACE UNTIL A SUFFICIENT GROWTH OF VEGETATION IS ESTABLISHED AND THEN BE REMOVED AS PART OF THE BASE BID.
12. IF SEDIMENT LADEN WATER NEEDS TO BE REMOVED FROM THE SITE, FILTER BAGS OR SCREENING SHALL BE USED IN ACCORDANCE WITH WI DNR TECHNICAL STANDARD 1061 TO PREVENT SEDIMENT DISCHARGE TO THE MAXIMUM EXTENT PRACTICABLE.
13. COORDINATE ALL EARTHWORK ACTIVITIES WITH THE RESPECTIVE TRADES RESPONSIBLE FOR THE INSTALLATION OF GAS, CABLE, TELEPHONE AND ELECTRICAL (INCLUDING MAIN SERVICE, SITE LIGHTING, CONDUITS AND SIGNAGE).
14. INSTALL WisDOT TYPE HR FILTER FABRIC BENEATH ALL RIP RAP.
15. IF BARE SOIL IS EXPOSED DURING THE WINTER MONTHS, STABILIZATION BY MULCHING OR ANIONIC POLYACRYLAMIDE SHALL OCCUR PRIOR TO SNOWFALL OR GROUND FREEZE.
16. SILT FENCE SHALL BE INSTALLED AROUND THE TOPSOIL STOCKPILE.
17. THE CONTRACTOR SHALL PERFORM INSPECTIONS AND MONITORING OF EROSION CONTROL PRACTICES IN ACCORDANCE WITH THE WI DNR "CONSTRUCTION SITE INSPECTION REPORT" FORM 3400-187. THIS FORM CAN BE FOUND IN THE CONSTRUCTION SPECIFICATIONS.

## EROSION CONTROL LEGEND:

EXISTING CONTOUR — — — 888 — — —

PROPOSED CONTOUR ——— 888 ———

PROPOSED SILT FENCE ——— 9 C6.1

PROPOSED INLET PROTECTION ◆ 10 C6.1

EROSION CONTROL BLANKET ▨ 11 C6.1

ROCK CONSTRUCTION ENTRANCE ▨ 12 C6.1

CONCRETE WASHOUT AREA ▨ 8 C6.1

STORM WATER OVERLAND FLOW DIRECTION →

## EROSION CONTROL SEQUENCING:

1. INSTALL PERIMETER EROSION CONTROL
2. BEGIN DEMOLITION
3. BEGIN ROUGH GRADING AND UTILITY INSTALLATION
4. DURING GRADING ACTIVITIES EXISTING GRASS AND VEGETATION, TO BE REMOVED, SHALL REMAIN IN PLACE FOR AS LONG AS POSSIBLE, TO AVOID SEDIMENT TRANSPORT.
5. TEMPORARY STABILIZATION ACTIVITY SHALL COMMENCE WHEN LAND DISTURBING CONSTRUCTION ACTIVITIES HAVE TEMPORARILY CEASED AND WILL NOT RESUME FOR A PERIOD EXCEEDING 14 CALENDAR DAYS.
6. FINAL STABILIZATION ACTIVITY SHALL COMMENCE WHEN LAND DISTURBING ACTIVITIES CEASE AND FINAL GRADE HAS BEEN REACHED ON ANY PORTION OF THE SITE.
7. IF DISTURBED AREAS MUST BE LEFT OVER WINTER, AN ANIONIC POLYACRYLAMIDE SHALL BE APPLIED TO ALL DISTURBED AREAS PRIOR TO GROUND FREEZE. SEE SPECIFICATIONS FOR DETAILS.

## BENCHMARK:

ELEVATIONS ARE REFERENCED TO NAVD 88 DATUM.

BENCHMARK #1
NORTHWEST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHEAST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND 19TH STREET SOUTH.
ELEVATION = 673.19

BENCHMARK #2
NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND EAST AVENUE SOUTH.
ELEVATION = 673.67

BENCHMARK #3
NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST CORNER OF THE INTERSECTION OF EAST AVENUE SOUTH AND WINNEBAGO STREET.
ELEVATION = 673.34

BENCHMARK #4
BURY BOLT ON HYDRANT, LOCATED AT THE NORTHEAST CORNER OF THE INTERSECTION OF 19TH STREET SOUTH AND WINNEBAGO STREET.
ELEVATION = 671.92

## UTILITY DISCLAIMER:

THE LOCATIONS, SIZES, AND TYPES OF UNDERGROUND PUBLIC AND PRIVATE UTILITIES OR SUBSTRUCTURES SHOW HEREON WERE OBTAINED FROM VISUAL INSPECTION, FIELD MEASUREMENTS, AND/OR AS-BUILT PLANS. SANITARY SEWER AND STORM SEWER PIPE SIZES, INVERTS, DIRECTION, AND LOCATIONS BETWEEN MANHOLES ARE SUPPLEMENTED BY AS-BUILT PLANS AND/OR ESTIMATED BASED ON FIELD OBSERVATIONS. PRIOR TO CONSTRUCTION IN THE VICINITY OF ANY UTILITIES SHOWN HEREON, IT IS RECOMMENDED THAT THE LOCATIONS, DEPTHS, AND SIZES BE FIELD VERIFIED. THE LOCATIONS SHOWN HEREON ARE ONLY APPROXIMATE, WITH POSSIBILITY THAT ADDITIONAL UTILITY LINES NOT DISCOVERED, OR MARKED, DURING THE SEARCH OF RECORDS AND THE FIELD SURVEY MAY EXIST. ANY CONTRACTOR USING THE INFORMATION SHOWN HEREON IS HEREBY FOREWARNED THAT ANY EXCAVATION UPON THIS SITE MAY RESULT IN THE DISCOVERY OF ADDITIONAL UNDERGROUND UTILITIES NOT SHOWN HEREON. IN GENERAL, UNDERGROUND UTILITY LOCATIONS ARE SHOWN FROM UTILITY MARKINGS, BY OTHERS, AND/OR AS-BUILT PLANS, PROVIDED BY OTHERS. POINT OF BEGINNING MAKES NO WARRANTY OF ANY KIND, EXPRESS OR IMPLIED, WITH RESPECT TO THE EXISTING UTILITIES SHOWN HEREON, AND BELIEVES THAT THE INFORMATION CONTAINED HEREIN IS RELIABLE AND GENERALLY ACCURATE FOR THE PURPOSE INTENDED.

0' 30' 60'
1" = 30'

BRAY ARCHITECTS
Milwaukee
829 South 1st Street
Milwaukee, Wisconsin 53204
T: 414.226.0200
Sheboygan • Davenport • Moline
www.brayarch.com
POB Point of Beginning
Civil Engineering
Land Surveying
Landscape Architecture
4941 Kirschling Court
Stevens Point, WI 54481
715.344.9999(Ph)
715.344.9922(Fx)

Project Title:
ADDITIONS AND ALTERATIONS TO:
LA CROSSE NEW ELEMENTARY SCHOOL
LA CROSSE SCHOOL DISTRICT
807 EAST AVE S.
LA CROSSE, WI 54601

REVISIONS:

| | DATE | DESCRIPTION |
|---|---|---|

NOT FOR CONSTRUCTION

Project Number: 3752

Issued For: CONTRACT DOCUMENTS 09/30/2025

Sheet Title: Erosion Control Plan

Sheet Number: C4.0

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# APPENDIX F

## State of Wisconsin Construction Site Inspection Report, Post Construction Long-Term Storm Water Management Checklist, And Notice of DNR Notice of Termination

State of Wisconsin
Department of Natural Resources (DNR)
PO Box 7921, Madison WI 53707-7921
dnr.wi.gov

# CONSTRUCTION SITE INSPECTION REPORT

Form 3400-187 (R 11/16) 

**Notice:** This form was developed in accordance with s. NR 216.48 Wis. Adm. Code for WPDES permittees' convenience; however, use of this specific form is voluntary. Multiple copies of this form may be made to compile the inspection report. Inspections of the construction site and implemented erosion and sediment control best management practices (BMPs) must be performed weekly and within 24 hours after a rainfall event 0.5 inches or greater.

| **Construction Site Name and Location (Project, Municipality, and County):** | **Site/Facility ID No. (FIN):** |
|---|---|
| **Onsite Contact/Contractor:** | **Onsite Phone/Cell:** |

**Note: Inspection reports, along with erosion control and storm water management plans, are required to be maintained on site in accordance with s. NR 216.48 (4) and made available upon request. PLEASE PRINT LEGIBLY.**

**Date of inspection:**

**Time of inspection:**
**Start:** _____ ◯ am ◯ pm
**End:** _____ ◯ am ◯ pm

**Type of inspection:** ◯ **Weekly** ◯ **Precipitation Event** ◯ **Other (specify)**

**Weather/Site Conditions:**

**Temp.** _____ **°F** **Antecedent Soil Moisture**

◯ **Dry** ◯ **Frozen or snow covered**
◯ **Variable** ◯ **Frozen (Thaw predicted in next week)**
◯ **Wet** ◯ **Melting Snow/slush**

**Last Rainfall Depth:** _____ **inches**

**Last Rainfall Date:** _____

**Describe current phase of construction:**

Scheduled Final Stabilization Date for Universal Soil Loss Equation (USLE) [1] :

**Project on Schedule[2]?** ◯ **Yes** ◯ **No**

**Name(s) of individual(s) performing inspection:**

**Inspector Phone/Cell:**

I certify that the information contained on this form is an accurate assessment of site conditions at the time of inspection:

**Inspector Signature** _____ **Date:** _____

| **Inspection Questions:** | **Yes** | **No (Identify Actions Required):** | **Location/Comments:** | **Actions Completed by Date & Initials** |
|---|---|---|---|---|
| 1. Is the erosion control plan accessible to operators? | ☐ | ☐ Provide onsite copy | | |
| 2. Is the permit certificate posted where visible? | ☐ | ☐ Post certificate | | |
| 3. Is the current phase of construction on sequence with the site-specific erosion and sediment control plan, including installation/stabilization of ponds and ditches? | ☐ | ☐ Add sediment control<br>☐ Install missing ditch/pipe/pond<br>☐ Stabilize bare soil | | |
| 4. Are all erosion and sediment control BMPs shown on plan properly installed and in functional condition? | ☐ | ☐ Repair<br>☐ Modify<br>☐ Install/Replace | | |
| 5. Is inlet protection properly installed and functioning in all inlets likely to receive runoff from the site? | ☐ | ☐ Clean<br>☐ Replace<br>☐ Install | | |
| 6. Is the air free of fugitive dust resulting from construction activity and bare soil exposure? | ☐ | ☐ Apply water<br>☐ Apply dust control product | | |

[1] The Universal Soil Loss Equation (USLE) model and the Construction Site Soil Loss and Sediment Discharge Guidance are available at: http://dnr.wi.gov/topic/stormwater/standards/const_standards.html

[2] If the project is not on schedule then the soil loss summary for the project should be reviewed and schedule, plan or practices modified accordingly.

# CONSTRUCTION SITE INSPECTION REPORT



| Inspection Questions: | Yes | No (Identify Actions Required): | Location/Comments: | Actions Completed by Date & Initials |
|---|---|---|---|---|
| 7. Is the public right of way curb line free of tracked soil and accumulation? | ☐ | ☐ Install tracking pad<br>☐ Widen/lengthen pad<br>☐ Amend stone/Add geotextile<br>☐ Install wheel washing station<br>☐ Close entrance/exit<br>☐ Limit traffic across disturbed areas<br>☐ Sweep road and curb line | | |
| 8. Are wetlands, lakes, streams, ditches, or storm sewers downstream of the site free of sedimentation and turbid water leaving the site?[3] | ☐ | ☐ Repair/Replace erosion control<br>☐ Add sediment controls<br>☐ Modify operations<br>☐ Contact DNR to verify extent of cleanup required | | |
| 9. Is dewatering and/or vehicle and equipment washing being done in a manner that prevents erosion and sediment discharge? | ☐ | ☐ Install treatment train<br>☐ Install energy dissipation<br>☐ Modify discharge location<br>☐ Modify intake to reduce sediment | | |
| 10. Are soil stockpiles existing for more than 7 days covered and stabilized? | ☐ | ☐ Seed<br>☐ Install mat/mulch/polymer<br>☐ Cover with tarp/plastic sheeting | | |
| 11. Are downstream channels and other downhill areas protected from scour and erosion? | ☐ | ☐ Install energy dissipation at outfall<br>☐ Install ditch checks<br>☐ Install slope interruption<br>☐ Install onsite detention | | |
| 12. Are good housekeeping practices or treatment controls in place to prevent the discharge of chemicals, cement, trash, and other materials into wetlands, waterways, storm sewers, ditches, or drainage-ways?[4] | ☐ | ☐ Properly dispose of trash<br>☐ Provide concrete washout station<br>☐ Contact DNR to verify extent of cleanup required | | |
| 13. Is the plan reflective of current site operations and does it address all erosion and sediment control issues identified during the inspection? | ☐ | ☐ Revise sequence<br>☐ Revise sediment control BMP<br>☐ Revise erosion control BMP<br>☐ Revise post-construction storm water BMP | | |
| 14. Are all areas where construction has temporarily ceased (and will not resume for more than 2 weeks) temporarily stabilized? | ☐ | ☐ Topsoil & seed<br>☐ Install mat/mulch/polymer<br>☐ Cover with tarp/plastic sheeting | | |
| 15. **Are all areas at final grade permanently vegetated or stabilized with other treatments?** | ☐ | ☐ Topsoil & seed<br>☐ Install mat/mulch/polymer<br>☐ Sod<br>☐ Install stone base | | |
| 16. **Have temporary sediment controls been removed in areas of the site that meet the permit definition of 'final stabilization'?** | ☐ | ☐ Water to establish vegetation<br>☐ Repair or reseed areas<br>☐ Remove temporary practices | | |

[3] If sediment discharge enters a wetland or waterbody, the permittee should consult with DNR staff to determine if sediment cleanup and/or additional control measures are required.

[4] The permittee shall notify the DNR immediately via the spills hotline at (800)943-0003 of any release or spill of a hazardous substance to the environment in accordance with s. 292.11, Wis. Stats., and ch. NR 706, Wis. Adm. Code.

# Construction Site Inspection Corrective Action Photos



## Corrective Action Photo Documentation Pages (Attach as many as needed):

**Notice:** Use of this specific form is voluntary, and is provided as an optional attachment to Form 3400-187 for use in documenting erosion and sediment control maintenance actions. This form is provided for the convenience of the permittee to meet the requirements of s. NR 216.48(4), Wis. Adm. Code.

| **Construction Site Name (Project):** | **Site/Facility ID No. (FIN):** |
|---|---|
| **Photo Location:** | |

## BEFORE CONDITION:

| | |
|---|---|
| | **Photo #:** |
| | **Date/Time of Photo:** |
| | **Photo By:** |
| | **Photo Description:** |

## AFTER CONDITION:

| | |
|---|---|
| | **Photo #:** |
| | **Date/Time of Photo:** |
| | **Photo By:** |
| | **Photo Description:** |

# Construction Site Inspection Corrective Action Photos



| Construction Site Name (Project): | Site/Facility ID No. (FIN): |
|---|---|
| Photo Location: | |

## BEFORE CONDITION:

| | |
|---|---|
| | Photo #: |
| | Date/Time of Photo: |
| | Photo By: |
| | Photo Description: |

## AFTER CONDITION:

| | |
|---|---|
| | Photo #: |
| | Date/Time of Photo: |
| | Photo By: |
| | Photo Description: |

State of Wisconsin
Department of Natural Resources
dnr.wi.gov

# Notice of Termination (NOT)
# Industrial Storm Water Discharges Permit

Form 3400-170 (R 6/15) 

**Notice:** This Notice of Termination (NOT) form is authorized by s. 283.37, Wis. Stats. The authorized representative of the facility shall submit this NOT form to the Department to certify that the facility no longer claims coverage under any general or individual permit for the discharge of storm water from industrial activity. Submittal of this NOT form constitutes notice that the party identified in Section I of this form is no longer authorized to discharge storm water from industrial activity under a WPDES permit. Termination of coverage will be effective when confirmed by the Department to the permittee. Personally identifiable information found on this form is not intended to be used for any other purpose.

**All necessary information must be provided on this NOT form. Failure to complete this form correctly may result in rejection of this NOT form by the Department. Please read all instructions on page 3 of this form before completing it.**

**Please type or clearly print your answers to all questions**

## Section I: Owner/Operator Contact Information

| Business Name | Authorized Representative | | |
|---|---|---|---|
| Mailing Address | City | State: WI | Zip Code |
| E-mail Address | Phone Number (area code) | Alternate Phone Number | |

## Section II: Facility/Site Information

| Facility/Site Name | Facility Identification (FID) or FIN Number: FID FIN |
|---|---|
| Location Address/Description | County |

| PLSS Information | Township: N | Range: ☐ East ☐ West | Section | Quarter | Quarter-Quarter |
|---|---|---|---|---|---|

## Section III: Termination Information

Reason for Termination Request

If facility was sold or transferred, please provide the following:

| Name of New Owner | E-mail Address | Phone Number (area code) |
|---|---|---|
| Mailing Address of New Owner | City | State: WI / Zip Code |

If this is a move, provide new address of facility:

If this facility will be inactive, are any significant materials (see back of form) exposed to storm water? ○ Yes ○ No
If yes, please explain:

Have you retained any control over the industrial activities or materials at the facility? ○ Yes ○ No
If yes, please explain:

**Required:** Attach photos that are representative of the current outdoor conditions at the facility. Current photos may eliminate the need for an inspection and facilitate a more timely acknowledgment by the Department.

Date Photos were taken: _____

## Section IV: Certification & Signature

**This form must be signed by an official representative of the permitted facility, in accordance with s. NR 216.22(7), Wis. Adm. Code. If this form is not signed, or is found to be incomplete, it will be returned.**

State regulations require this form to be signed by an official representative of the facility as follows:

1. For a corporation, by a principal executive officer of at least the level of Vice President, or a duly authorized representative having overall responsibility for the operation covered by this permit.
2. For a unit of government, a principal executive officer, a ranking elected official or other duly authorized representative.
3. For a partnership, by a general partner; for a sole proprietorship, by the proprietor.
4. For a limited liability company, by a member or manager.

**Certification:** I understand that by submitting this Notice of Termination, the site described herein is no longer authorized to discharge, and does not discharge, storm water associated with industrial activity by the general WPDES permit; and that discharging pollutants in storm water associated with industrial activity to waters of Wisconsin is unlawful where the discharge is not authorized by a WPDES permit.

| Signature of Landowner/Authorized Representative | Date Signed |
|---|---|
| | |

| Printed Name of Landowner/Authorized Representative | Title |
|---|---|
| | |

**Mail this completed NOT form to the appropriate Wisconsin Department of Natural Resources office in the region where the facility is located. See the instructions on page 3 of this form for regional office addresses.**

## Instructions

### Section I: Owner/Operator Contact Information

Provide the legal name of the person, firm, public organization, or any other entity that operates the industrial activity described in Section II of this form and holds or qualifies for an applicable general or individual industrial storm water discharge permit. The operator of the activity is the legal entity which controls the activity's operation. The mailing address and phone number given should be for the facility contact person.

### Section II: Facility/Site Information

Provide the name of the facility as it appears on the permit application or permit cover letter. If known, provide the Facility Identification (FID) and/or FIN Number.

### Section III: Termination Information

Provide some details about the reason for this termination request. If you moved your activities to a new site, you need to reapply for the storm water permit coverage. If new owners or operators are continuing activity at this site, they need to apply for a storm water permit separately. The storm water permit coverage is site specific and is not transferable.

Examples of significant materials are: industrial machinery, raw materials, intermediate and finished products, waste products, fuels, solvents, detergents, hazardous substances, and fertilizers.

### Section IV: Certification & Signature

State Statutes provide for severe penalties for submitting false information on this NOT form. State regulations require this form to be signed as follows:

1. For a corporation, by a principal executive officer of at least the level of Vice President, or a duly authorized representative having overall responsibility for the operation covered by this permit.
2. For a unit of government, a principal executive officer, a ranking elected official, or other duly authorized representative.
3. For a partnership, by a general partner; for a sole proprietorship, by the proprietor.
4. For a limited liability company, by a member or manager.

Sign the form, print or type the name of the individual signing the NOT and the date of signature, and provide the contact information.

### Mailing Address

Unless otherwise directed, mail this completed NOT Form to the Wisconsin Department of Natural Resources office associated with the county of the facility site location as follows:

| | | | | |
|---|---|---|---|---|
| **NORTHERN REGION (NOR)** | | | | |
| Ashland<br>Barron<br>Bayfield<br>Burnett | Douglas<br>Florence<br>Forest<br>Iron | Langlade<br>Lincoln<br>Oneida<br>Polk<br>Price | Rusk<br>Sawyer<br>Taylor<br>Vilas<br>Washburn | WDNR Baldwin Service Center<br>890 Spruce Street<br>Baldwin, WI 54002<br>715-684-2914 ext. 109 |
| **NORTHEAST REGION (NER)** | | | | |
| Brown<br>Calumet<br>Door<br>Fond du Lac | Green Lake<br>Kewaunee<br>Manitowoc<br>Marinette | Marquette<br>Menominee<br>Oconto<br>Oneida Reservation | Outagamie<br>Shawano<br>Waupaca<br>Waushara<br>Winnebago | WDNR Northeast Regional Headquarters<br>2984 Shawano Avenue<br>Green Bay, WI 54313-6727<br>920-662-5100 |
| **WEST CENTRAL REGION (WCR)** | | | | |
| Adams<br>Buffalo<br>Chippewa<br>Clark | Crawford<br>Dunn<br>Eau Claire<br>Jackson<br>Juneau | La Crosse<br>Marathon<br>Monroe<br>Pepin<br>Pierce | Portage<br>St. Croix<br>Trempealeau<br>Vernon<br>Wood | WDNR Baldwin Service Center<br>890 Spruce Street<br>Baldwin, WI 54002<br>715-684-2914 ext. 109 |
| **SOUTH CENTRAL REGION (SCR)** | | | | |
| Columbia<br>Dane<br>Dodge | Grant<br>Green<br>Iowa | Jefferson<br>LaFayette<br>Richland | Rock<br>Sauk | WDNR South Central Regional Headquarters<br>3911 Fish Hatchery Road<br>Fitchburg, WI 53711<br>608-275-3266 |
| **SOUTHEAST REGION (SER)** | | | | |
| Kenosha<br>Milwaukee | Ozaukee<br>Racine | Sheboygan<br>Walworth | Washington<br>Waukesha | WDNR Waukesha Service Center<br>141 N.W. Barstow Street, Room 180<br>Waukesha, WI 53188<br>262-574-2100 |

# Storm Water Management Practices
# Post Construction Long-Term Storm Water Management Checklist

**Site Name:** New Elementary School

**Location:** City of La Crosse, La Crosse County, Wisconsin

**Responsible Party**: The owner is responsible for the post construction long-term storm water management upkeep. This checklist may be utilized when performing inspections after any rainfall event exceeding one inch of rainfall, and at a minimum semi-annually in early spring and fall.

**Date of Inspection:** (mm/dd/yy)

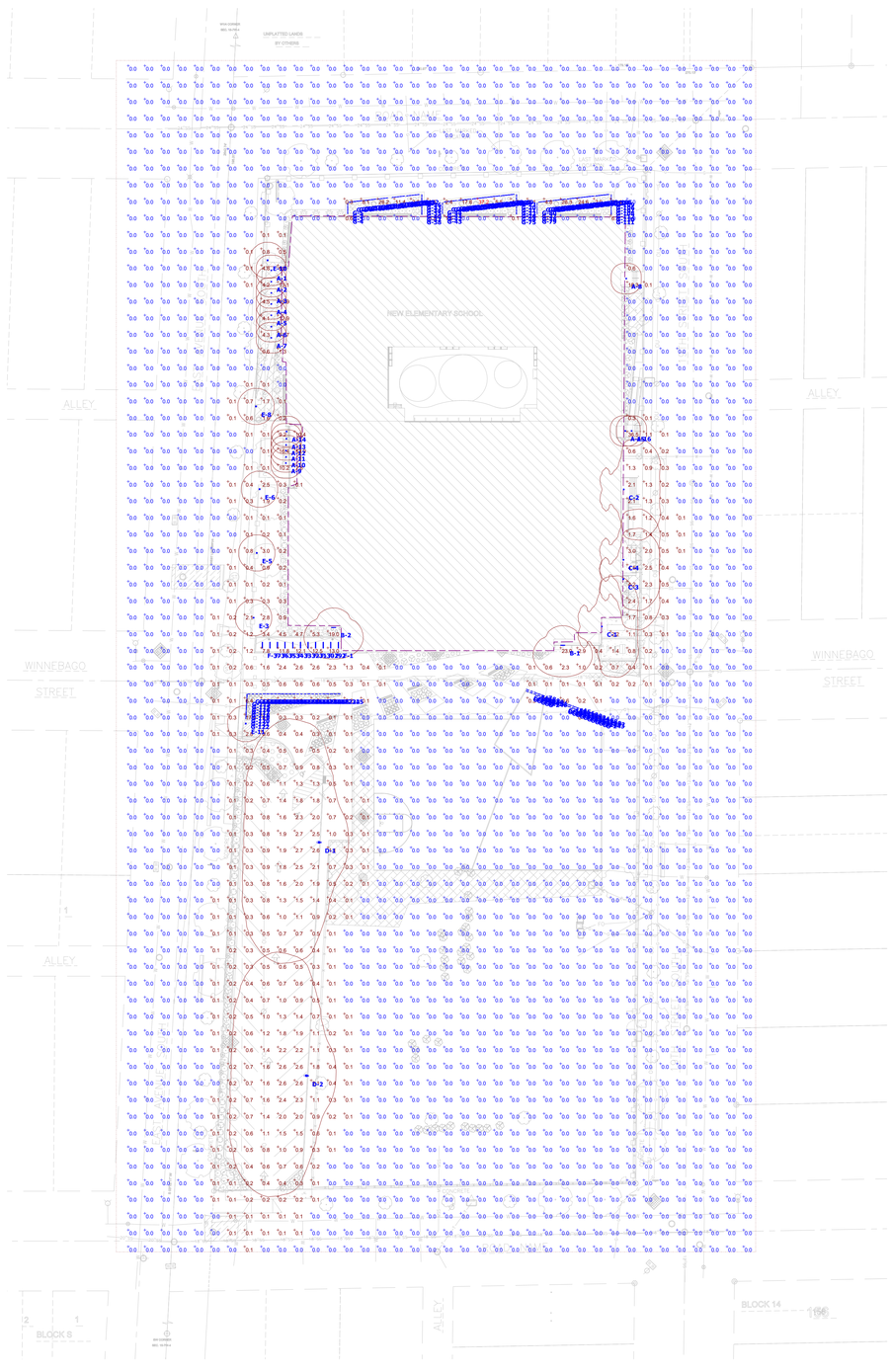
**Time of Inspection:** (start/end)

**Type of Inspection:** (annual/quarterly/precipitation event)

**Weather:**

**Inspector's Name:**

| **Component Inspected:** | **Repairs Required:** | **Comments:** |
|---|---|---|
| Grass and Plants throughout Site | | |
| -Bare Spots | | |
| -Dead Plant Material | | |
| -Washouts | | |
| Storm Sewer Pipes: | | |
| -Sediment Deposits | | |
| -Trash/Debris | | |
| -Cracks | | |
| StormTech Chamber System: | | |
| -Sediment Deposits | | |
| -Trash | | |
| -Inlet/Outlet Pipes | | |
| -Pond Depth | | |




**Plan View**
Scale - 1" = 40ft

**Schedule**

| Label | Image | QTY | Symbol | Manufacturer | Catalog Number | Description | Lamp | Number Lamps | Filename | Lumens per Lamp | Lumen Multiplier | LLF | Wattage | Efficiency | Distribution | Polar Plot | Notes |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| A | | 16 | | Gotham Architectural Lighting | IVO6SQ D 20LM 40K 80CRI MWD P AR LSS | IVO6SQ Downlight 2000 Lumens 4000K 80+ CRI Medium Wide Batwing Open Parabolic Clear Anodized Semi-Specular | | 1 | IVO6SQ D 20LM 40K 80CRI MWD P AR LSS.ies | 1914 | 1 | 0.95 | 20.4785 | 100% | DIRECT, SC-0=0.96, SC-90=0.94 | Max: 2110cd | |
| B | | 2 | | Luminaire LED | BLD 36IN 30W 40K DP | Catalog Number: BLD36-30W 4000K2 Surface wall mounted, extruded and cast aluminum housing, formed white enamel aluminum reflector, clear lightly frosted plastic enclosure. 108 white LEDs, three Luminaire LED MP-LED-MOD Rev 3.0C boards with 36 LEDs per board. One ULT Everline D10CC30UNVTW-C LED driver labeled as 940mA, 120.0Vac, 60.00Hz, 0.2519A, 29.61W, 0.980PF, 7.5%THD(i) | | 1 | BLD 36IN 30W 40K DP.ies | 3009 | 1 | 0.95 | 29.6 | 100% | DIRECT, SC-0=1.81, SC-90=1.26 | Max: 1496cd | |
| C | | 4 | | Lithonia Lighting | WDGE2 LED P2 40K 80CRI T3M | WDGE2 LED WITH P2 - PERFORMANCE PACKAGE, 4000K, 80CRI, TYPE 3 MEDIUM OPTIC | | 1 | WDGE2 LED P2 40K 80CRI T3M.ies | 2062 | 1 | 0.95 | 18.9815 | 100% | TYPE III, MEDIUM, BUG RATING:: B1 - U0 - G1 | Max: 1512cd | |
| D | | 2 | | Lithonia Lighting | DSX1 LED P3 40K 80CRI T2M HS | D-Series Size 1 Area Luminaire P3 Performance Package 4000K CCT 80 CRI Type 2 Medium Houseside Shield | | 1 | DSX1 LED P3 40K 80CRI T2M HS.ies | 10831 | 1 | 0.95 | 102.1727 | 100% | TYPE III, MEDIUM, BUG RATING:: B1 - U0 - G3 | Max: 9538cd | |
| E | | 6 | | U.S. ARCHITECTURAL LIGHTING | BSA6-CL-24LED-350mA-40K | 6-EXTRUSION VERTICAL POWER ARRAY (VPA). | WHITE LED, 164 RATED LUMENS PER LED AT 350MA; | 24 | BSA6-CL-24LED--350mA-40K.ies | 44 | 1 | 0.95 | 26.4 | 100% | | Max: 198cd | |
| F | | 11 | | Mark Architectural Lighting | SL4L 4 FLP 80CRI 40K 400LMF | Slot 4 LED, 4ft Length, Flushed Lens Position, 80CRI, 4000K LED Boards, 400LMF | LED | 1 | SL4L 4 FLP 80CRI 40K 400LMF.ies | 1293 | 1 | 0.95 | 13.82 | 100% | DIRECT, SC-0=1.22, SC-90=1.22 | Max: 473cd | |
| G | | 173 | | ACOLYTE | CHANNEL | CHAS7MRB90SWMC203.030 | | 1 | CHAS7MRB90SWMC203.030.ies | 152 | 1 | 0.95 | 4.86 | 100% | | Max: 5ficd | |

**Statistics**

| Description | Symbol | Avg | Max | Min | Max/Min | Avg/Min |
|---|---|---|---|---|---|---|
| Calc Zone #1 | + | 0.4 fc | 37.2 fc | 0.0 fc | N/A | N/A |

ENGINEERING DESIGN BY
MSA
(800) 362-4505 • www.msa-pc.com

**LaCROSSE NEW ELEMENTARY SCHOOL**
**SITE LIGHTING CALCULATION**

**Designer**
KS
**Date**
09/18/2025
**Scale**
1"=40'-0"
**Drawing No.**
E0.1
**Summary**

LA CROSSE
WISCONSIN

Department of Planning and Development
400 La Crosse St, La Crosse, WI 54601 | P: (608) 789-7512

## Memorandum

To: **Design Review Committee**
**Joe Ledvina, La Crosse School District**
**Nathan Derks, Bray Architects**
**Melissa Kluck**

From: **Tim Acklin, Planning and Development Department**

Date: **September 26, 2025**

**Re: Design Review Project**
**New Elementary School- 807 East Ave.**

Design Review Committee Members:
Steve Pataska, Police Department
Tim Acklin, Planning & Development Department
Matt Gallager, Engineering Department
Yuri Nasonovs, Engineering Department
Eddie Young, Building & Inspections Department
Jason Riley, Building & Inspections Department
Bee Xiong, Fire Department- Community Risk Management
Brian Asp, Utilities Department
Leah Miller, Parks, Recreation, and Forestry Department
Jamie Hassemer, Engineering Department
Stephanie Sward, Engineering Department
Cullen Haldeman, Engineering Department

On September 11, 2025, plans were submitted to the Design Review Committee for review of the project located at 807 East Ave. (New Elementary School) The following comments/feedback have been provided.

All revised plans in accordance with this memo must be submitted to the Planning and Development Department for review, unless otherwise stated. No permits will be issued for this project by the Division of Fire Prevention and Building Safety until they receive written confirmation/approval from the Planning and Development Department.

**Requirements Prior to Issuance of a Demolition or Footing & Foundation Permit**

1) Approval of a Certified Survey Map. (If applicable)
2) Combination of parcels for project site. (If applicable)
3) Approval of Final Plans from the Engineering Department.
4) Approval of Final Plans from the Utility/Water Department.
5) Approval of Final Plans from the Division of Fire Prevention and Building Safety (Inspections)

**Requirements Prior to Issuance of a Building Permit**

1) Approval of Final Plans from the Planning and Development Department.

**Requirements Prior to Issuance of an Occupancy Permit**

1) A Letter of Credit in the estimated cost amount of the proposed landscaping has been submitted to the Planning and Development Department to guarantee the proper installation and growth of all landscape improvements proposed in the approved Landscape Plan **OR** all proposed landscaping in the approved Landscape Plan has been installed.
2) A stamped letter of substantial completion from the design engineer of the project within 10 days of completion.
3) Field review and approval of the completed stormwater management facility by the City's Utilities Department.

**Engineering Department - (Contact-Matt Gallager-789-7392, Stephanie Sward-789-8171, Jamie Hassemer- 789-8182, Cullen Haldeman- 789-8185)**

1) The ROW vacation and CSM were approved by Council in June. Once the design review process is complete, the vacation can be recorded followed by the CSM. The easement needed for utilities in the proposed vacated Winnebago St will be addressed by another document. Am working with the consultant on this. (Kyle Coman)
2) Need dimension (rectangle) of parking stalls. (Stephanie Sward)
3) Any replacement of public sidewalk within 15ft of an intersection will require you to replace all public ADA facilities within that intersection. (Stephanie Sward)
4) Driveway off of Mississippi Street cannot cut of curb head. Will need to fully replace. (Stephanie Sward)
5) Any replacement of curb, pavement, sidewalk, curb ramp, driveways need to follow City Standard Specifications and Detail Drawings. (Stephanie Sward)
6) Will need to submit a Photometric Plan for entire site. Include a summary of fixtures and model numbers used. Plan must also provide calculations/foot candles 25ft outside off of property lines. (Jamie Hassemer)
7) Must place "Do not Enter" signs at exit of parking lot. (Jackson Overby)

**Building and Inspections Department**
**(Contact-Eddie Young- 789-7582, Jason Riley- 789-7585)**

1) Lots to be combined to create 1 parcel.
2) Demo permit

3) Require State approved Building, plumbing and HVAC plans.
4) Submit a separate permit for any proposed signs.
5) With 13' setback, dumpster enclosure and architectural wall need to meet this setback if attached to the building. If enclosure is not attached to the building may be considered a fence and would need fence permit. Planter walls are fine since not attached and landscape item.
6) No plumbing permits until Utilities issues are satisfied. And stormwater permit has been approved.

**Police Department- Steve Pataska-789-7205**

1) No concerns at this time.

**Planning Department-(Contact-Tim Acklin-789-7391)**

1) Must submit a Photometric Plan including light specifications and cutsheets.
2) Provide calculation of percent of first floor windows to total façade on all street facing facades. (East, Market, 19th). (The ground/first floor must comprise of windows and doors that equal 20% of the entire façade.)

**Utilities Department- (Brian Asp-789-3897)**

1) Sanitary sewer needs to be extended to the north from the intersection of Winnebago to eliminate long lateral, work with the utility to coordinate work and payment. (Coordinate with Utilities)
2) Watermain needs to be extended to Market St. (Coordinate with Utilities)
3) Storm sewer on East Ave needs to stay in the ROW and make a 90 degree turn into the easement. (Coordinate with Utilities)
4) Sanitary Sewer Connection Fee – work with utilities office to calculate
5) Filed easement is required for extension of Winnebago St.
6) Horizontal and Vertical separation must be maintained between utilities per city standard specs – show dimensions on plans
7) Running utilities up 19th street in 2026. Coordinate where to stud out to with Engineering Department.
8) May be easier to connect from east/Winnebago to 19th/Winnebago. (Contact Brian Asp for more discussion.)

**Engineering Department (Stormwater) - (Contact-Yuri Nasonovs-789-7594)**

1) Mostly covered by Brian and Matt.
2) Need water quality management letter. Requirements are on the website.
3) Stormwater management permit per Sec. 105 of code.
4) Since over 1 acre of disturbance, get DNR permit.
5) Fees associated with water quality management letter and stormwater management permit. May be waived by Board of Public Works.
6) Submit plans and models for review in accordance with Chapter 105.

**Fire Department- (Contact- Bee Xiong 789-7260)**

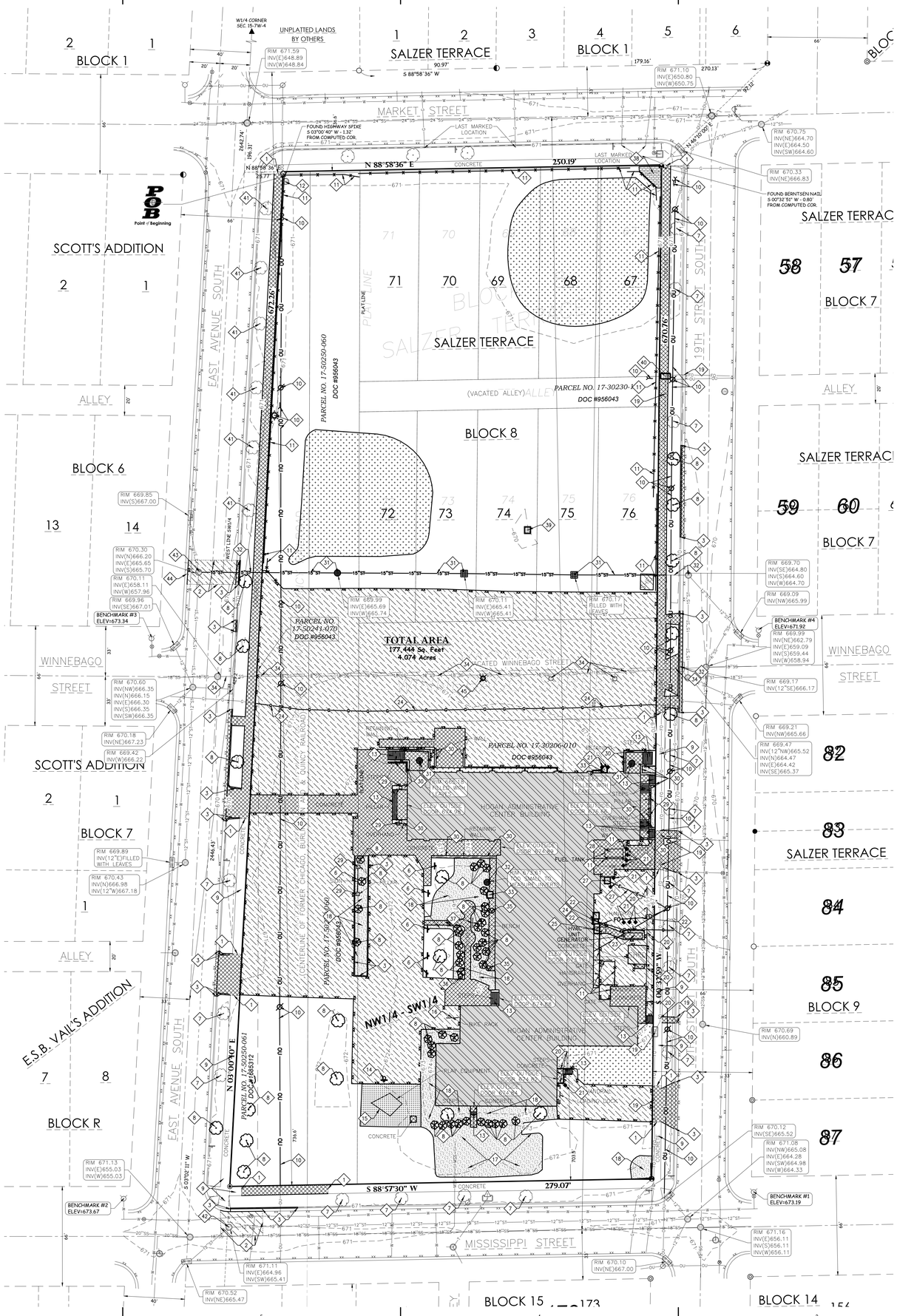
1) Knox Box Required. (preferred to be at the addressed side or front door)
2) Appropriate plans submitted to the State and the La Crosse Fire Department along with fees and permit applications for prior to any work starting:
   Fire alarm

Commercial kitchen suppression system
Fire sprinkler systems
Email all plans to FD (state approved plans for fire alarm and sprinkler systems required for FD).

3) Sprinkler FDC within 100' of nearest hydrant.
   200 psi hydro underground water service line must be performed along with flush test with FD prior to hooking up to sprinkler riser (preferably as soon as the line is covered with dirt).
   6" minimum water service line.

**Parks, Recreation, and Forestry- (Contact-Leah Miller, 789-8672, Dan Trussoni 789-4915)**

1) No comment has been provided at this time on the Landscaping Plan.




## GENERAL NOTES:

1. CONTRACTOR SHALL LOCATE ALL PUBLIC AND PRIVATE UTILITIES PRIOR TO COMMENCEMENT OF WORK.
2. ALL DEMOLITION MATERIALS SHALL BE REMOVED FROM THE SITE AND DISPOSED OF IN A LEGAL MANNER EXCEPT FOR THOSE ITEMS NOTED TO BE SALVAGED, WHICH SHOULD BE TURNED OVER TO THE OWNER.
3. INSTALL AND MAINTAIN ALL REQUIRED EROSION CONTROL MEASURES FOR PERIMETER PROTECTION PRIOR TO THE START OF DEMOLITION/CONSTRUCTION, IN ACCORDANCE WITH THE LOCAL AND STATE GOVERNING AUTHORITIES.
4. BIDDERS SHALL VISIT THE SITE AND REVIEW EXISTING CONDITIONS PRIOR TO THE BID DATE.
5. ANY EXISTING PUBLIC & PRIVATE UTILITIES NOT SHOWN ON THIS DOCUMENT WHICH NEED TO BE REMOVED, RELOCATED, AND/OR ADJUSTED SHALL BE THE RESPONSIBILITY OF THE SITE GRADING CONTRACTOR.
6. COORDINATE ALL PRIVATE & PUBLIC UTILITY REMOVAL, DISCONNECTION, &/OR RECONNECTION WITH RESPECTIVE UTILITY COMPANIES PRIOR TO BEGINNING WORK.
7. COORDINATE UTILITY SERVICE DISCONNECTIONS/OUTAGES WITH OWNER AND ANY IMPACTED NEIGHBORS. MINIMIZE DISRUPTIONS TO THE MAXIMUM EXTENT PRACTICAL.
8. ANY EXISTING UTILITIES NOT EXPRESSLY LABELED FOR DEMOLITION/REMOVAL ON THIS DOCUMENT SHALL BE LEFT IN PLACE AND IN THEIR CURRENT STATE OF OPERATION. CONTACT ENGINEER WHEREVER CLARIFICATION IS NEEDED.
9. STRIP TOPSOIL WITHIN THE PROJECT LIMITS IN ACCORDANCE WITH THE PROJECT MANUAL SPECIFICATIONS.
10. IF STRIPPED TOPSOIL IS STOCKPILED ON SITE, SILT FENCE SHALL BE INSTALLED AROUND THE BASE OF THE STOCKPILE TO PREVENT SEDIMENT TRANSPORT.
11. PRIOR TO PERFORMING WORK WITHIN PUBLIC RIGHT OF WAYS, NOTIFY AND COORDINATE WORK WITH THE LOCAL MUNICIPALITY.
12. MAINTAIN TRAFFIC CIRCULATION TO ALL NEIGHBORING PROPERTIES SHOWN ON THIS DOCUMENT. COORDINATE ALL WORK WITH NEIGHBORING PROPERTY OWNERS.

## KEYNOTES:

1. SAWCUT EXISTING CONCRETE WALK
2. SAWCUT EXISTING ASPHALT PAVEMENT
3. SAWCUT & REMOVE EXISTING CONCRETE CURB & GUTTER
4. SAWCUT EXISTING CONCRETE CURB HEAD ONLY
5. MAINTAIN EXISTING SIGN
6. REMOVE EXISTING SIGN
7. MAINTAIN & PROTECT EXISTING SIGN
8. CLEAR & GRUB EXISTING TREE/VEGETATION
9. MAINTAIN EXISTING CONCRETE WALK
10. REMOVE EXISTING OVERHEAD POWER, POWER POLE, GUY WIRE. COORDINATE REMOVALS WITH UTILITY COMPANY.
11. REMOVE EXISTING FENCE & GATE
12. MAINTAIN EXISTING UTILITY MANHOLE. ADJUST RIM TO FINISH GRADE ELEVATION. SEE SHEET C3.0. COORDINATE ADJUSTMENT WITH UTILITY COMPANY.
13. REMOVE EXISTING CONCRETE STAIRS
14. REMOVE EXISTING BASKETBALL HOOP
15. REMOVE EXISTING PLAYGROUND EQUIMPMENT
16. REMOVE EXISTING BIKE RACK
17. REMOVE EXISTING GARDEN AREA
18. REMOVE EXISTING LANDSCAPE AREA, INCLUDING, NOT LIMITED TO PLANTINGS, MULCH, EDGING, FABRIC, ETC.
19. MAINTAIN EXISTING FIBER OPTIC SERVICE & BOX. ADJUST TOP OF BOX TO FINISH GRADE ELEVATION. SEE SHEET C3.0. COORDINATE ANY ADJUSTMENT WITH UTILITY COMPANY.
20. REMOVE EXISTING HANDRAIL
21. REMOVE EXISTING BOLLARD
22. REMOVE EXISTING FIBER OPTIC SERVICE. COORDINATE REMOVAL WITH UTILITY COMPANY.
23. REMOVE EXISTING GENERATOR
24. REMOVE EXISTING GAS SERVICE AND/OR METER. COORDINATE WITH UTILITY COMPANY.
25. REMOVE EXISTING HVAC UNIT
26. REMOVE EXISTING ELECTRICAL SERVICE. COORDINATE REMOVAL WITH UTILITY COMPANY.
27. REMOVE EXISTING ELECTRICAL TRANSFORMER. COORDINATE REMOVAL WITH UTILITY COMPANY.
28. REMOVE EXISTING TANK
29. REMOVE EXISTING PILLAR
30. REMOVE EXISTING RETAINING WALL
31. REMOVE EXISTING STORM SEWER MANHOLE AND/OR STORM PIPE
32. MAINTAIN EXISTING STORM SEWER MANHOLE AND/OR STORM PIPE
33. REMOVE AC UNIT
34. MAINTAIN EXISTING SANITARY SEWER PIPE AND/OR CLEANOUT
35. REMOVE EXISTING VENT PIPE
36. REMOVE EXISTING BIRD HOUSE
37. REMOVE EXISTING BENCH
38. REMOVE EXISTING BRICK AREA
39. REMOVE EXISTING CABLE BOX– COORDINATE REMOVAL WITH UTILITY COMPANY
40. REMOVE EXISTING FIBER OPTIC BOX– COORDINATE REMOVAL WITH UTILITY COMPANY
41. MAINTAIN & PROTECT EXISTING TREE
42. MAINTAIN EXISTING SANITARY MANHOLE– ADJUST RIM AS NEEDED TO FINISH GRADE ELEVATION
43. MAINTAIN EXISTING STORM MANHOLE– ADJUST RIM AS NEEDED TO FINISH GRADE ELEVATION
44. MAINTAIN EXISTING CONCRETE CURB & GUTTER
45. REMOVE & RELOCATE EXISTING SANITARY CLEANOUT

## DEMOLITION HATCH PATTERNS:

- BITUMINOUS REMOVAL
- CONCRETE REMOVAL
- GRAVEL REMOVAL
- BUILDING REMOVAL

## CIVIL SHEET INDEX:


C1.0 DEMOLITION PLAN
C2.0 LAYOUT PLAN
C3.0 GRADING PLAN
C4.0 EROSION CONTROL PLAN
C5.0 UTILITY PLAN
C6.0 DETAILS
C6.1 DETAILS
C6.2 DETAILS
L1.0 LANDSCAPE PLAN


## BENCHMARK:

ELEVATIONS ARE REFERENCED TO NAVD 88 DATUM.

**BENCHMARK #1**
NORTHWEST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHEAST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND 19TH STREET SOUTH.
ELEVATION = 673.19

**BENCHMARK #2**
NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND EAST AVENUE SOUTH.
ELEVATION = 673.67

**BENCHMARK #3**
NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST CORNER OF THE INTERSECTION OF EAST AVENUE SOUTH AND WINNEBAGO STREET.
ELEVATION = 673.34

**BENCHMARK #4**
BURY BOLT ON HYDRANT, LOCATED AT THE NORTHEAST CORNER OF THE INTERSECTION OF 19TH STREET SOUTH AND WINNEBAGO STREET.
ELEVATION = 671.92

## UTILITY DISCLAIMER:

THE LOCATIONS, SIZES, AND TYPES OF UNDERGROUND PUBLIC AND PRIVATE UTILITIES OR SUBSTRUCTURES SHOW HEREON WERE OBTAINED FROM VISUAL INSPECTION, FIELD MEASUREMENTS, AND/OR AS–BUILT PLANS. SANITARY SEWER AND STORM SEWER PIPE SIZES, INVERTS, DIRECTION, AND LOCATIONS BETWEEN MANHOLES ARE SUPPLEMENTED BY AS–BUILT PLANS AND/OR ESTIMATED BASED ON FIELD OBSERVATIONS. PRIOR TO CONSTRUCTION IN THE VICINITY OF ANY UTILITIES SHOWN HEREON, IT IS RECOMMENDED THAT THE LOCATIONS, DEPTHS, AND SIZES BE FIELD VERIFIED. THE LOCATIONS SHOWN HEREON ARE ONLY APPROXIMATE, WITH POSSIBILITY THAT ADDITIONAL UTILITY LINES NOT DISCOVERED, OR MARKED, DURING THE SEARCH OF RECORDS AND THE FIELD SURVEY MAY EXIST. ANY CONTRACTOR USING THE INFORMATION SHOWN HEREON IS HEREBY FOREWARNED THAT ANY EXCAVATION UPON THIS SITE MAY RESULT IN THE DISCOVERY OF ADDITIONAL UNDERGROUND UTILITIES NOT SHOWN HEREON. IN GENERAL, UNDERGROUND UTILITY LOCATIONS ARE SHOWN FROM UTILITY MARKINGS, BY OTHERS, AND/OR AS–BUILT PLANS, PROVIDED BY OTHERS. POINT OF BEGINNING MAKES NO WARRANTY OF ANY KIND, EXPRESS OR IMPLIED, WITH RESPECT TO THE EXISTING UTILITIES SHOWN HEREON, AND BELIEVES THAT THE INFORMATION CONTAINED HEREIN IS RELIABLE AND GENERALLY ACCURATE FOR THE PURPOSE INTENDED.

0' 30' 60'
1" = 30'


BRAY ARCHITECTS
Milwaukee
829 South 1st Street
Milwaukee, Wisconsin 53204
T: 414.226.0200
Sheboygan • Davenport • Moline
www.brayarch.com


Project Title:
ADDITIONS AND ALTERATIONS TO:
LA CROSSE NEW ELEMENTARY SCHOOL
LA CORSSE SCHOOL DISTRICT
807 EAST AVE. S.
LA CROSSE, WI 54601

REVISIONS:

| | DATE | DESCRIPTION |
|---|---|---|

NOT FOR CONSTRUCTION

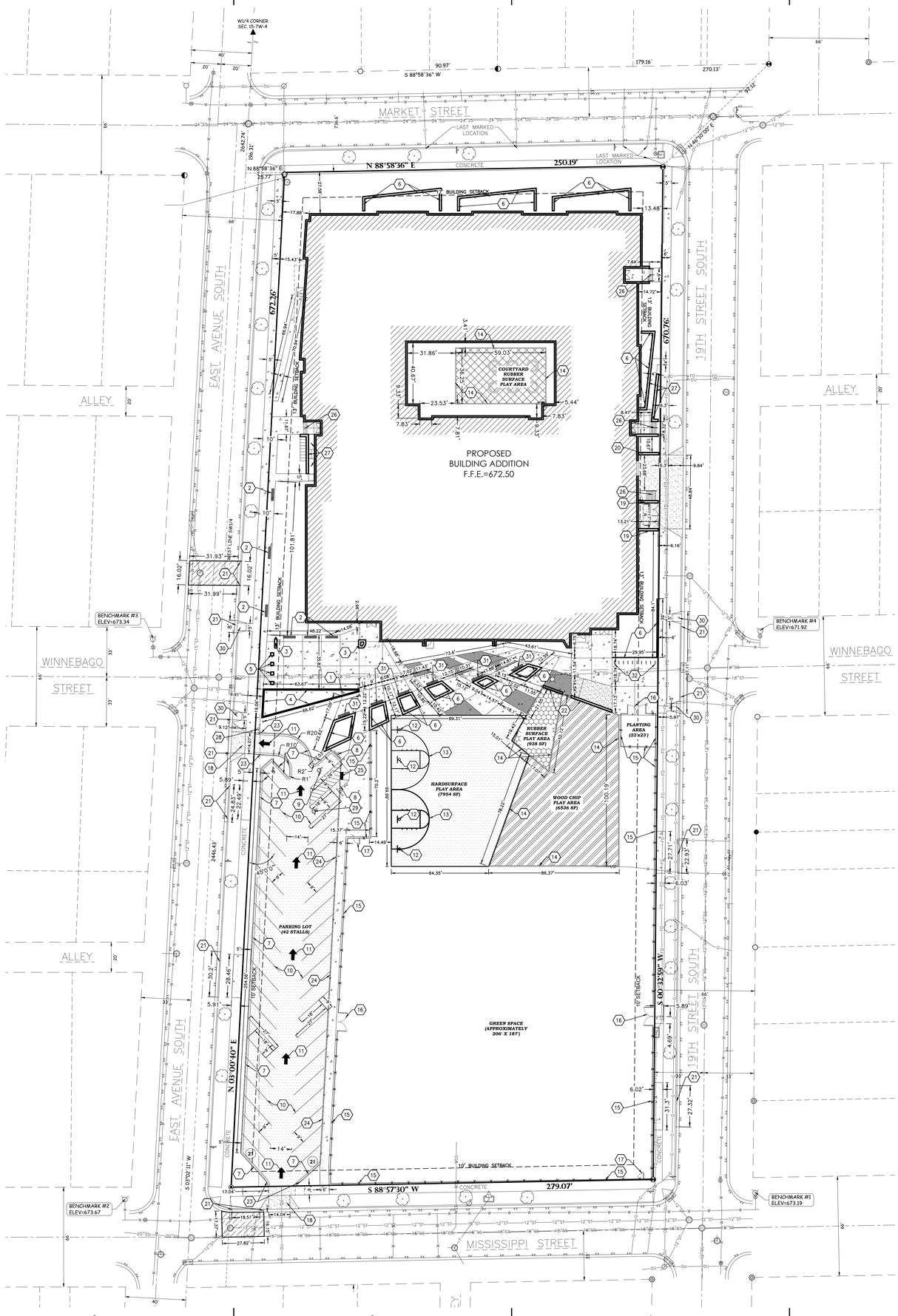
Project Number: 3752

Issued For: 50% CONSTRUCTION DOCUMENTS 08/19/2025

Sheet Title: Demolition Plan

Sheet Number: C1.0

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## GENERAL NOTES:

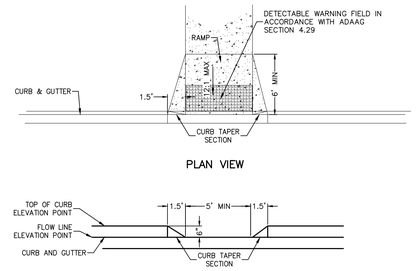
1. CONTRACTOR SHALL LOCATE ALL PUBLIC AND PRIVATE UTILITIES PRIOR TO COMMENCEMENT OF WORK.
2. GRADE, LINE, AND LEVEL TO BE REVIEWED IN THE FIELD BY THE CONSTRUCTION MANAGER.
3. ALL REQUIRED EROSION CONTROL MEASURES SHALL BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH LOCAL MUNICIPAL AND DEPARTMENT OF NATURAL RESOURCES REGULATIONS.
4. SEE SHEET C4.0 FOR ALL REQUIRED EROSION CONTROL ELEMENTS.
5. ANY EXISTING UTILITIES NOT SHOWN ON THIS DOCUMENT WHICH NEED TO BE REMOVED, RELOCATED AND OR ADJUSTED SHALL BE THE RESPONSIBILITY OF THE SITE GRADING CONTRACTOR.
6. VERIFY THE LOCATION OF ALL EXISTING UTILITIES PRIOR TO THE START OF DEMOLITION/CONSTRUCTION.
7. BIDDERS SHALL VISIT THE SITE AND REVIEW EXISTING CONDITIONS PRIOR TO THE BID DATE.
8. PRIOR TO STARTING WORK, VERIFY WITH THE LOCAL AUTHORITIES THAT ALL REQUIRED PERMITS HAVE BEEN ACQUIRED.
9. COORDINATE CONSTRUCTION IN THE RIGHT OF WAY WITH THE LOCAL AUTHORITIES.
10. PROVIDE PROPER BARRICADES, SIGNS, AND TRAFFIC CONTROL TO MAINTAIN THRU TRAFFIC ALONG ADJACENT STREETS IN ACCORDANCE WITH LOCAL MUNICIPAL REQUIREMENTS.
11. SIDEWALK JOINTS SHALL BE INSTALLED AS INDICATED OR AS APPROVED BY THE CONSTRUCTION MANAGER.
12. ALL GENERAL LANDSCAPE AREAS SHALL BE SEEDED, FERTILIZED, AND CRIMP HAY MULCHED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS.

## KEYNOTES:

1. FLAG POLE– TITAN SERIES 25' ANODIZED CLEAR FLAG POLE – IWW25C61 (OR APPROVED ~~EQUAL~~) INSTALL PER MANUFACTURER'S RECOMMENDATIONS — 7 C6.0
2. BENCH – TYPE 1 TF5037 CONCRETE TECH BENCH BY WAUSAU TILE COLOR: A20 WHITE FINISH: WEATHERSTONE(B) OPTION: CAST–IN SKATE STOPPERS
3. CANOPY PILLAR– SEE ARCHITECTURAL PLANS
4. 8" CONCRETE CURB
5. BOLLARD– SEE ARCHITECTURAL PLANS — 5 C6.0
6. 18" HT. PRECAST CONCRETE SEATWALL/PLANTER BY WAUSAU TILE COLOR: A20 WHITE FINISH: WEATHERSTONE(B) OPTION: CAST–IN SKATE STOPPERS
7. 18" CONCRETE CURB & GUTTER
8. ADA PARKING SIGN — 3 C6.0
9. ADA PARKING STALL– STANDARD — 8 C6.0
10. PARKING LOT STRIPING — 2 C6.1
11. DIRECTIONAL ARROW
12. BASKETBALL HOOP– KEEPER GOALS PRODUCT ALL–AMER–IN–GL 60" IN–GROUND BASKETBALL HOOP (OR APPROVED EQUAL) INSTALL PER MANUFACTURER'S RECOMMENDATIONS — 2 C6.1, 1 C6.1
13. PLAYGROUND STRIPING– 4" WIDE LINE, WHITE TRAFFIC PAINT (VERIFY LOCATION & DIMENSIONS W/OWNER PRIOR TO CONSTRUCTION)
14. PLAY AREA CURBING — 9 C6.0, 10 C6.0
15. 6' TALL CHAIN LINK FENCING — 5 C6.1, 6 C6.1
16. 12' SERVICE GATE — 4 C6.1
17. 4' PEDESTRIAN GATE — 3 C6.1
18. DRIVE APRON
19. DUMPSTER ENCLOSURE– SEE ARCHITECTURAL PLANS
20. ARCHITECTURAL WALL– SEE ARCHITECTURAL PLANS
21. CONCRETE CURB & GUTTER– MATCH EXISTING
22. 18" HT. FREE STANDING SEAT WALL BY WAUSAU TILE COLOR: A20 WHITE FINISH: WEATHERSTONE(B) OPTION: CAST–IN SKATE STOPPERS
23. 6" CURB HEAD TAPER SECTION — 5 C6.0
24. THICKENED EDGE WALK — 4 C6.0
25. ADA CURB RAMP 'A' — 6 C6.0
26. CONCRETE STAIRS W/RAILING– SEE ARCHITECTURAL PLANS
27. RAMP RAILING– SEE ARCHITECTURAL PLANS
28. STOP SIGN — 7 C6.1
29. ADA PARKING STALL–VAN ACCESSIBLE — 2 C6.1
30. ADA CURB RAMP 'B' — 1 C2.0
31. THEORY – THICK BEAM BENCH BY LANDSCAPE FORMS LENGTH: 8 FOOT TYPE: THICK BENCH W/SOLID SEAT & 2 SKATE STOPS COLOR: LEMON FINISH: POWDERCOAT
32. KEY BIKE RACK BY LANDSCAPE FORMS COLOR: RED FINISH: POWDERCOAT

## PAVEMENT HATCH PATTERNS:

- PROPOSED STANDARD ASPHALT PAVEMENT — 1 C6.0
- PROPOSED STANDARD CONCRETE PAVEMENT — 2 C6.0
- COLOR CONCRETE TYPE 1 COLOR: 'DARK GREY' BY DAVIS COLORS FINISH: LIGHT SAND BLAST — 2 C6.0
- PROPOSED REINFORCED CONCRETE PAVEMENT — 2 C6.0
- PROPOSED RUBBER SURFACE PLAY AREA (COURTYARD) — 10 C6.0
- PROPOSED WOOD CHIP PLAY AREA — 9 C6.0
- PROPOSED ASPHALT PAVEMENT TO MATCH EXISTING CROSS SECTION
- PROPOSED RUBBER SURFACE PLAY AREA (OUTSIDE OF SCHOOL) — 9 C6.0
- PROPOSED LAWN AREA AT PLAZA




**ADA CURB RAMP 'B'** — 1 C2.0

## BENCHMARK:

ELEVATIONS ARE REFERENCED TO NAVD 88 DATUM.

**BENCHMARK #1**
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ELEVATION = 671.92

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0' 30' 60'
1" = 30'

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Project Title:
ADDITIONS AND ALTERATIONS TO:
LA CROSSE NEW ELEMENTARY SCHOOL
LA CORSSE SCHOOL DISTRICT
807 EAST AVE. S.
LA CROSSE, WI 54601

REVISIONS:

| | DATE | DESCRIPTION |
|---|---|---|

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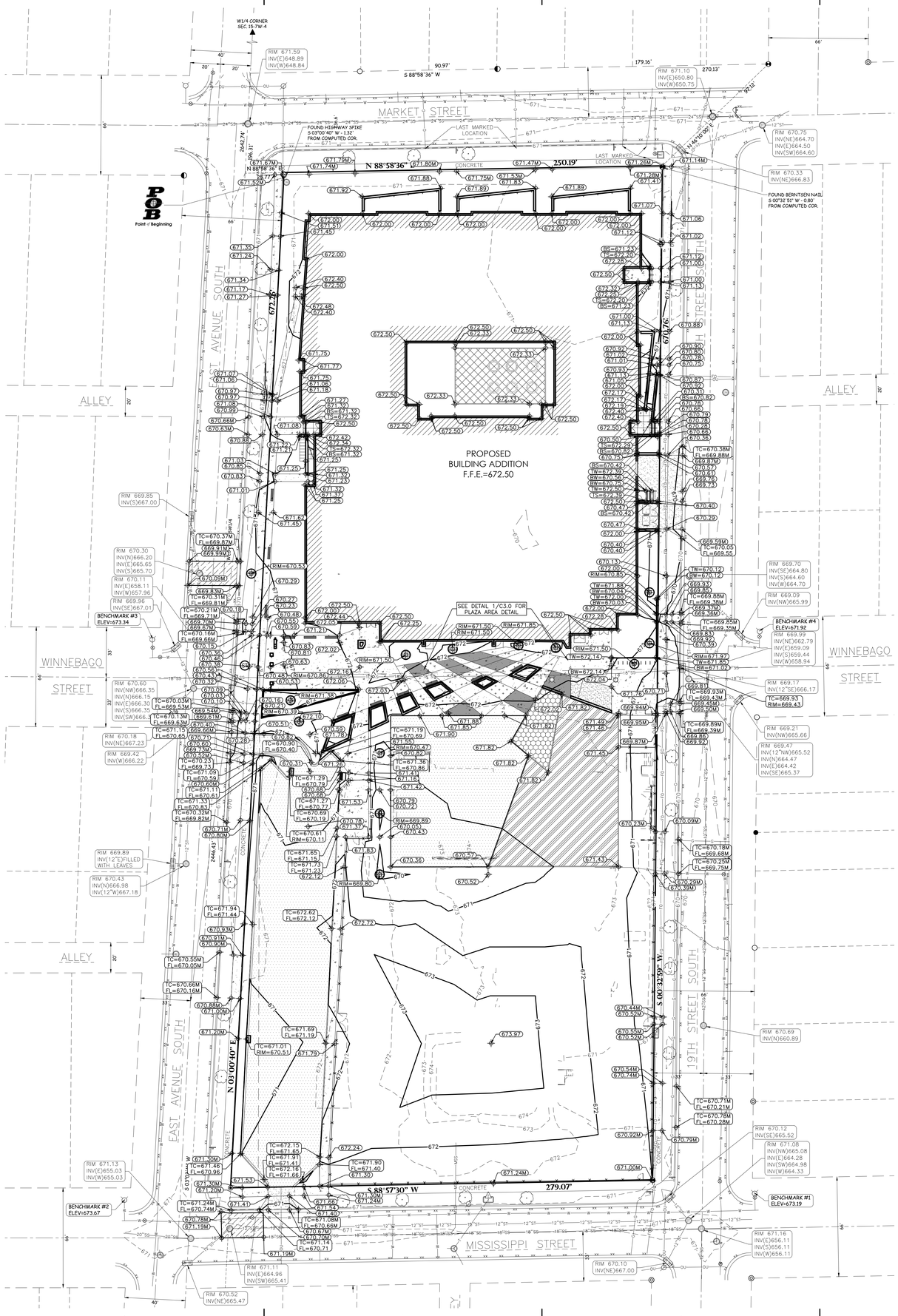
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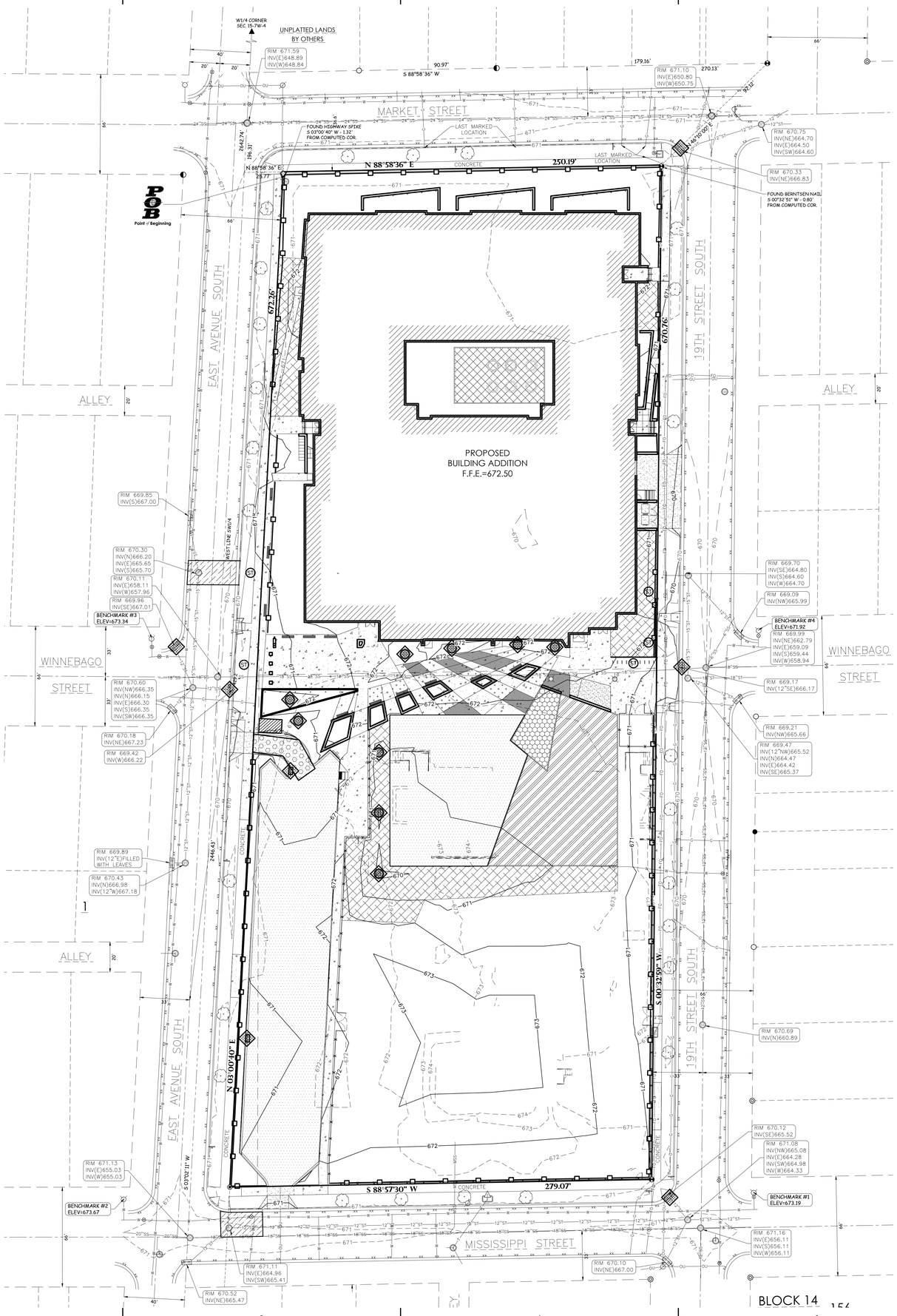
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Sheet Title: Layout Plan

Sheet Number: C2.0

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W1/4 CORNER
SEC 15-7W-4
RIM 671.59
INV(E)648.89
INV(W)648.84
S 88°58'36" W
90.97'
179.16'
270.13'
RIM 671.10
INV(E)650.80
INV(W)650.75
MARKET STREET
FOUND HIGHWAY SPIKE
S 03°00'40" W - 1.32'
FROM COMPUTED COR.
LAST MARKED LOCATION
N 88°58'36" E
RIM 670.75
INV(NE)664.70
INV(E)664.50
INV(SW)664.60
RIM 670.33
INV(NE)666.83
FOUND BERNTSEN NAIL
S 00°32'59" W - 0.80'
FROM COMPUTED COR.
N 88°58'36" E
250.19'
CONCRETE
POB
Point of Beginning
EAST AVENUE SOUTH
19TH STREET SOUTH
ALLEY
PROPOSED
BUILDING ADDITION
F.F.E.=672.50
SEE DETAIL 1/C3.0 FOR
PLAZA AREA DETAIL
WINNEBAGO
STREET
MISSISSIPPI STREET
N 00°00'40" E
S 00°32'59" W
S 88°57'30" W
279.07'
264.74'
244.45'
672.76'
RIM 669.85
INV(S)667.00
RIM 670.30
INV(E)666.20
INV(E)665.65
INV(S)665.70
RIM 670.11
INV(E)658.11
INV(W)657.96
RIM 669.96
INV(SE)667.01
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INV(W)666.22
RIM 669.89
INV(12"E)FILLED
WITH LEAVES
RIM 670.43
INV(N)666.98
INV(12"W)667.18
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669.35M
669.87M
669.95




## GENERAL NOTES:

1. CONTRACTOR SHALL LOCATE ALL PUBLIC AND PRIVATE UTILITIES PRIOR TO COMMENCEMENT OF WORK.
2. NOTIFY THE LOCAL MUNICIPALITY AT LEAST 2 WORKING DAYS PRIOR TO THE START OF SOIL DISTURBING ACTIVITIES.
3. INSTALL ALL TEMPORARY EROSION CONTROL ELEMENTS PRIOR TO THE START OF DEMOLITION/CONSTRUCTION.
4. ALL ACTIVITIES SHALL BE CONDUCTED IN A LOGICAL SEQUENCE TO MINIMIZE THE AMOUNT OF BARE SOIL EXPOSED AT ANY ONE TIME. MAINTAIN EXISTING VEGETATION AS LONG AS POSSIBLE.
5. CRUSHED ROCK DRIVES FOR SEDIMENT TRACKING UTILIZING 3" CRUSHED ROCK SHALL BE MAINTAINED AT ALL CONSTRUCTION ENTRANCES TO THE SITE. THE ROCK DRIVE SHALL BE A MINIMUM OF 12" THICK AND BE A MINIMUM OF 50 FEET IN LENGTH BY THE WIDTH OF THE DRIVEWAY.
6. OFFSITE SEDIMENT DEPOSITS RESULTING FROM STORMWATER RUNOFF SHALL BE CLEANED BY THE END OF THE NEXT WORKDAY. OFFSITE SEDIMENT DEPOSITS RESULTING FROM CONTRACTOR ACTIVITIES, INCLUDING SOIL TRACKING, SHALL BE CLEANED EACH WORKDAY. EXCESSIVE AMOUNTS OF SEDIMENT OR DEBRIS TRACKED ONTO ADJACENT STREETS SHALL BE CLEANED IMMEDIATELY. FINE SEDIMENT ACCUMULATIONS ON ADJACENT STREETS SHALL SWEPT MECHANICALLY OR MANUALLY AT LEAST WEEKLY AND BEFORE IMMINENT RAINFALL.
7. DISTURBED GROUND OUTSIDE OF THE EVERYDAY CONSTRUCTION AREAS, INCLUDING SOIL STOCKPILES, THAT ARE LEFT INACTIVE FOR MORE THAN 7 DAYS SHALL BE TEMPORARILY STABILIZED BY SEEDING/MULCHING OR OTHER APPROVED METHODS.
8. WASTE MATERIAL THAT IS GENERATED ON THE CONSTRUCTION SITE SHALL BE PROPERLY DISPOSED OF AND NOT ALLOWED TO RUN INTO RECEIVING WATERS.
9. EROSION CONTROL DEVICES DESTROYED AS A RESULT OF CONSTRUCTION ACTIVITIES SHALL BE REPAIRED BY THE END OF EACH WORK DAY.
10. INSPECT ALL EROSION CONTROL MEASURES AT LEAST ONCE A WEEK AND AFTER ANY RAINFALL OF 0.5" OR MORE. MAKE NEEDED REPAIRS AND DOCUMENT ALL ACTIVITIES AS PER THE REQUIREMENTS OF THE NOTICE OF INTENT SUBMITTED BY THE PROJECT CIVIL ENGINEER.
11. ALL TEMPORARY EROSION CONTROL ELEMENTS SHALL REMAIN IN PLACE UNTIL A SUFFICIENT GROWTH OF VEGETATION IS ESTABLISHED AND THEN BE REMOVED AS PART OF THE BASE BID.
12. IF SEDIMENT LADEN WATER NEEDS TO BE REMOVED FROM THE SITE, FILTER BAGS OR SCREENING SHALL BE USED IN ACCORDANCE WITH WI DNR TECHNICAL STANDARD 1061 TO PREVENT SEDIMENT DISCHARGE TO THE MAXIMUM EXTENT PRACTICABLE.
13. COORDINATE ALL EARTHWORK ACTIVITIES WITH THE RESPECTIVE TRADES RESPONSIBLE FOR THE INSTALLATION OF GAS, CABLE, TELEPHONE AND ELECTRICAL (INCLUDING MAIN SERVICE, SITE LIGHTING, CONDUITS AND SIGNAGE).
14. INSTALL WisDOT TYPE HR FILTER FABRIC BENEATH ALL RIP RAP.
15. IF BARE SOIL IS EXPOSED DURING THE WINTER MONTHS, STABILIZATION BY MULCHING OR ANIONIC POLYACRYLAMIDE SHALL OCCUR PRIOR TO SNOWFALL OR GROUND FREEZE.
16. SILT FENCE SHALL BE INSTALLED AROUND THE TOPSOIL STOCKPILE.
17. THE CONTRACTOR SHALL PERFORM INSPECTIONS AND MONITORING OF EROSION CONTROL PRACTICES IN ACCORDANCE WITH THE WI DNR "CONSTRUCTION SITE INSPECTION REPORT" FORM 3400-187. THIS FORM CAN BE FOUND IN THE CONSTRUCTION SPECIFICATIONS.

## EROSION CONTROL LEGEND:

EXISTING CONTOUR — — — 888 — — —

PROPOSED CONTOUR ——— 888 ———

PROPOSED SILT FENCE — 9 C6.1

PROPOSED INLET PROTECTION — 10 C6.1

EROSION CONTROL BLANKET — 11 C6.1

ROCK CONSTRUCTION ENTRANCE — 12 C6.1

CONCRETE WASHOUT AREA — 8 C6.1

STORM WATER OVERLAND FLOW DIRECTION →

## EROSION CONTROL SEQUENCING:

1. INSTALL PERIMETER EROSION CONTROL
2. BEGIN DEMOLITION.
3. BEGIN ROUGH GRADING AND UTILITY INSTALLATION
4. DURING GRADING ACTIVITIES EXISTING GRASS AND VEGETATION, TO BE REMOVED, SHALL REMAIN IN PLACE FOR AS LONG AS POSSIBLE, TO AVOID SEDIMENT TRANSPORT.
5. TEMPORARY STABILIZATION ACTIVITY SHALL COMMENCE WHEN LAND DISTURBING CONSTRUCTION ACTIVITIES HAVE TEMPORARILY CEASED AND WILL NOT RESUME FOR A PERIOD EXCEEDING 14 CALENDAR DAYS.
6. FINAL STABILIZATION ACTIVITY SHALL COMMENCE WHEN LAND DISTURBING ACTIVITIES CEASE AND FINAL GRADE HAS BEEN REACHED ON ANY PORTION OF THE SITE.
7. IF DISTURBED AREAS MUST BE LEFT OVER WINTER, AN ANIONIC POLYACRYLAMIDE SHALL BE APPLIED TO ALL DISTURBED AREAS PRIOR TO GROUND FREEZE. SEE SPECIFICATIONS FOR DETAILS.

## BENCHMARK:

ELEVATIONS ARE REFERENCED TO NAVD 88 DATUM.

**BENCHMARK #1**
NORTHWEST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHEAST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND 19TH STREET SOUTH.
ELEVATION = 673.19

**BENCHMARK #2**
NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND EAST AVENUE SOUTH.
ELEVATION = 673.67

**BENCHMARK #3**
NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST CORNER OF THE INTERSECTION OF EAST AVENUE SOUTH AND WINNEBAGO STREET.
ELEVATION = 673.34

**BENCHMARK #4**
BURY BOLT ON HYDRANT, LOCATED AT THE NORTHEAST CORNER OF THE INTERSECTION OF 19TH STREET SOUTH AND WINNEBAGO STREET.
ELEVATION = 671.92

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0' 30' 60'
1" = 30'

N

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Project Title:
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LA CROSSE, WI 54601

REVISIONS:

| | DATE | DESCRIPTION |
|---|---|---|

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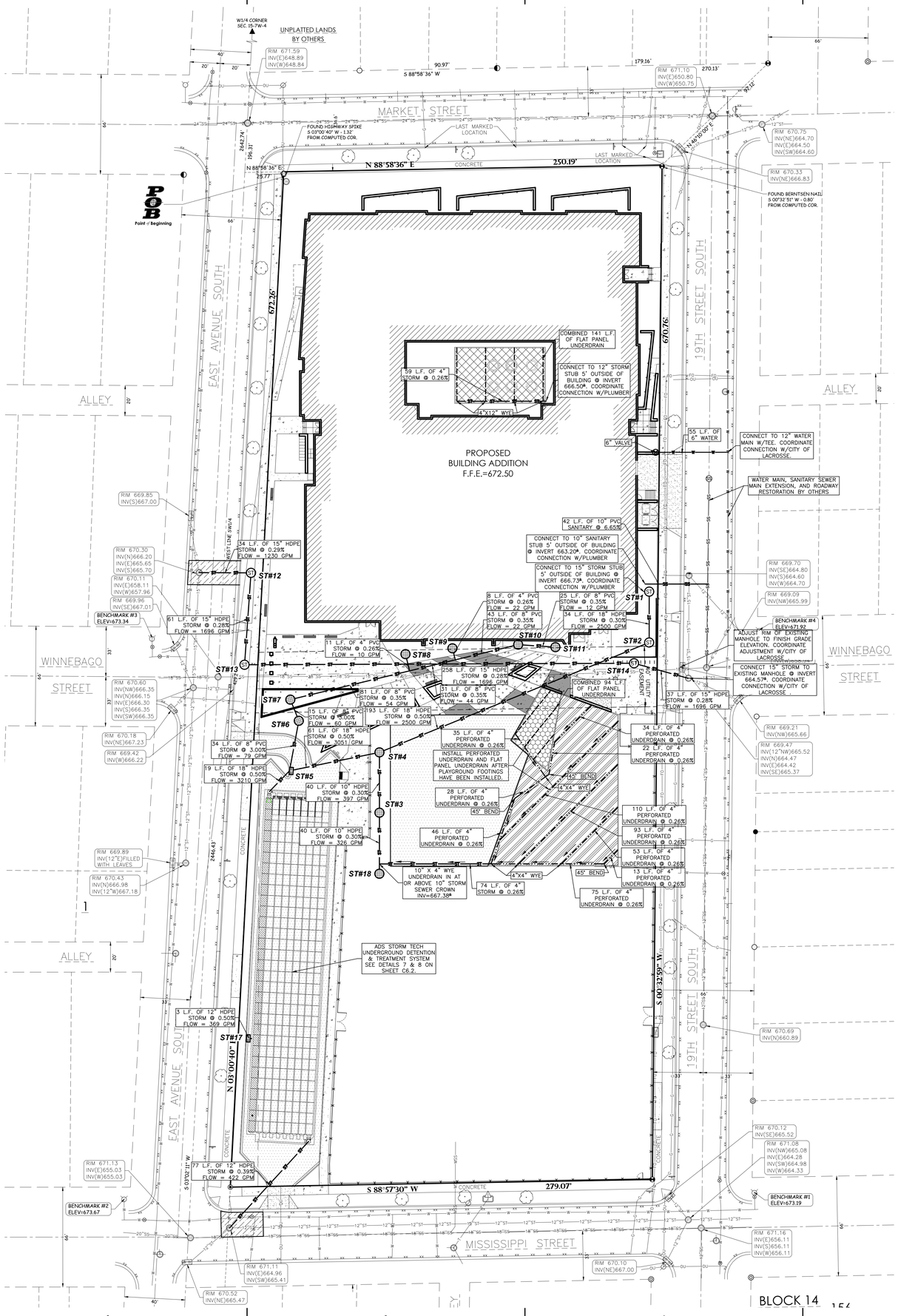
Project Number: 3752

Issued For: 50% CONSTRUCTION DOCUMENTS 08/19/2025

Sheet Title: Erosion Control Plan

Sheet Number: C4.0

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## GENERAL NOTES:

1. CONTRACTOR SHALL LOCATE ALL PUBLIC AND PRIVATE UTILITIES PRIOR TO COMMENCEMENT OF WORK.
2. GRADE, LINE, AND LEVEL SHALL BE REVIEWED IN THE FIELD BY THE CONSTRUCTION MANAGER.
3. ANY EXISTING UTILITIES NOT SHOWN ON THIS DOCUMENT WHICH NEED TO BE REMOVED, RELOCATED AND OR ADJUSTED SHALL BE THE RESPONSIBILITY OF THE SITE GRADING CONTRACTOR.
4. REFER TO THE PROPOSED BUILDING MECHANICAL/PLUMBING PLANS TO VERIFY EXACT CONNECTION LOCATIONS AND SIZES OF PROPOSED SANITARY SEWER AND WATER LATERALS.
5. COORDINATE ALL UTILITY WORK WITH THE RESPECTIVE TRADES RESPONSIBLE FOR THE INSTALLATION OF GAS, CABLE, TELEPHONE AND ELECTRICAL (INCLUDING MAIN SERVICE, SITE LIGHTING, CONDUITS AND SIGNAGE).
6. COORDINATE UTILITY SERVICE DISCONNECTIONS/OUTAGES WITH OWNER AND ANY IMPACTED NEIGHBORS. MINIMIZE DISRUPTIONS TO THE MAXIMUM EXTENT PRACTICAL.
7. COORDINATE ALL WORK WITHIN THE PUBLIC RIGHT OF WAY WITH THE LOCAL MUNICIPALITY.
8. ALL TESTING AND INSPECTION SHALL BE DONE IN ACCORDANCE WITH SPS 382.21.
9. THE PROPOSED WATER MAIN SHALL HAVE A MINIMUM COVER OF 7'-6" TO THE TOP OF PIPE FROM PROPOSED FINISHED GRADE. SEE SHEET C3.0 FOR PROPOSED FINISHED GRADE.
10. THE MUNICIPALITY SHALL BE CONTACTED PRIOR TO ANY EXCAVATION IN THE PUBLIC RIGHT-OF-WAY, AND PRIOR TO CONNECTING SANITARY SEWER AND WATER LATERALS TO THE PUBLIC MAINS.
11. THE CONTRACTOR SHALL HAVE A TRAFFIC CONTROL PLAN APPROVED PRIOR TO WORK COMMENCING.
12. THE MUNICIPALITY SHALL OPERATE ALL EXISTING WATER VALVES, IF NEEDED.
13. FIELD VERIFY INVERT ELEVATION OF THE SANITARY SEWER AND WATER PUBLIC MAIN, AT THE LOCATION OF THE SERVICE LATERAL CONNECTIONS, PRIOR TO CONNECTING THE LATERALS TO THE PUBLIC MAIN.
14. INSTALL WisDOT TYPE HR FILTER FABRIC BENEATH PROPOSED RIP RAP.

## UTILITY LEGEND:

- PROPOSED STORM SEWER
- PROPOSED SANITARY SEWER
- PROPOSED WATER MAIN
- PROPOSED STORM SEWER MANHOLE (ST) — 2 / C6.2
- PROPOSED STORM SEWER INLET — 2 / C6.2
- PROPOSED STORM SEWER CURB INLET — 1 / C6.2
- PROPOSED WATER VALVE — 4 / C6.2
- PROPOSED 4" UNDERDRAIN — 9 / C6.0
- PROPOSED FLAT PANEL UNDERDRAIN — 9 / C6.0, 10 / C6.0
- PROPOSED SANITARY MANHOLE (SN) — 3 / C6.2

## STORM MANHOLE SCHEDULE:

| STRUCTURE # | STRUCTURE DETAILS |
|---|---|
| ST#1 | RIM = 670.85<br>INV (S) = 666.73<br>DEPTH = 4.11'<br><br>48" I.D. PRECAST MANHOLE<br>NEENAH CASTING R-1733-1<br>W/SOLID COVER |
| ST#2 | RIM = 671.97<br>INV (W) = 666.63<br>INV (N) = 666.63<br>DEPTH = 5.34'<br><br>48" I.D. PRECAST MANHOLE<br>NEENAH CASTING R-1733-1<br>W/SOLID COVER |
| ST#3 | RIM = 669.89<br>INV (N) = 666.45<br>INV (S) = 666.45<br>DEPTH = 3.44'<br><br>48" I.D. PRECAST MANHOLE<br>W/NEENAH R-2560-EA CASTING<br>W/BEEHIVE GRATE |
| ST#4 | RIM = 670.47<br>INV (S) = 666.33<br>INV (W) = 665.66<br>INV (E) = 665.66<br>DEPTH = 4.81'<br><br>48" I.D. PRECAST MANHOLE<br>W/NEENAH R-2560-EA CASTING<br>W/BEEHIVE GRATE |
| ST#5 | RIM = 670.11<br>INV (N) = 666.19<br>INV (E) = 665.36<br>INV (SW) = 665.36<br>DEPTH = 4.76'<br><br>2'X3' BOX CATCH BASIN<br>W/NEENAH R-3067 CASTING<br>W/TYPE 'R' GRATE |
| ST#6 | RIM = 670.39<br>INV (N) = 667.20<br>INV (S) = 667.20<br>DEPTH = 3.19'<br><br>48" I.D. PRECAST MANHOLE<br>W/NEENAH R-2560-EA CASTING<br>W/BEEHIVE GRATE |
| ST#7 | RIM = 671.38<br>INV (E) = 667.65<br>INV (S) = 667.65<br>DEPTH = 3.73'<br><br>48" I.D. PRECAST MANHOLE<br>W/NEENAH R-2560-EA CASTING<br>W/BEEHIVE GRATE |
| ST#8 | RIM = 671.50<br>INV (S) = 668.30<br>DEPTH = 3.20'<br><br>48" I.D. PRECAST MANHOLE<br>W/NEENAH R-2560-EA CASTING<br>W/BEEHIVE GRATE |

| STRUCTURE # | STRUCTURE DETAILS |
|---|---|
| ST#9 | RIM = 671.50<br>INV (S) = 668.40<br>DEPTH = 3.10'<br><br>48" I.D. PRECAST MANHOLE<br>W/NEENAH R-2560-EA CASTING<br>W/BEEHIVE GRATE |
| ST#10 | RIM = 671.50<br>INV (E) = 668.20<br>INV (W) = 668.20<br>DEPTH = 3.31'<br><br>48" I.D. PRECAST MANHOLE<br>W/NEENAH R-2560-EA CASTING<br>W/BEEHIVE GRATE |
| ST#11 | RIM = 671.85<br>INV (W) = 668.28<br>DEPTH = 3.57'<br><br>48" I.D. PRECAST MANHOLE<br>W/NEENAH R-2560-EA CASTING<br>W/BEEHIVE GRATE |
| ST#12 | RIM = 670.53<br>INV (S) = 665.55<br>INV (W) = 665.55<br>DEPTH = 4.98'<br><br>48" I.D. PRECAST MANHOLE<br>NEENAH CASTING R-1733-1<br>W/SOLID COVER |
| ST#13 | RIM = 670.28<br>INV (E) = 665.38<br>INV (N) = 665.38<br>DEPTH = 4.90'<br><br>48" I.D. PRECAST MANHOLE<br>NEENAH CASTING R-1733-1<br>W/SOLID COVER |
| ST#14 | RIM = 671.50<br>INV (E) = 664.67<br>INV (W) = 664.67<br>DEPTH = 6.83'<br><br>48" I.D. PRECAST MANHOLE<br>NEENAH CASTING R-1733-1<br>W/SOLID COVER |
| ST#17 | RIM = 670.51<br>INV (E) = 665.28<br>DEPTH = 5.00'<br><br>2'X3' BOX CATCH BASIN<br>W/NEENAH R-3067 CASTING<br>W/TYPE 'R' GRATE |
| ST#18 | RIM = 669.80<br>INV (N) = 666.57<br>DEPTH = 3.23'<br><br>48" I.D. PRECAST MANHOLE<br>W/NEENAH R-2560-EA CASTING<br>W/BEEHIVE GRATE |

## BENCHMARK:

ELEVATIONS ARE REFERENCED TO NAVD 88 DATUM.

**BENCHMARK #1**
NORTHWEST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHEAST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND 19TH STREET SOUTH.
ELEVATION = 673.19

**BENCHMARK #2**
NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND EAST AVENUE SOUTH.
ELEVATION = 673.67

**BENCHMARK #3**
NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST CORNER OF THE INTERSECTION OF EAST AVENUE SOUTH AND WINNEBAGO STREET.
ELEVATION = 673.34

**BENCHMARK #4**
BURY BOLT ON HYDRANT, LOCATED AT THE NORTHEAST CORNER OF THE INTERSECTION OF 19TH STREET SOUTH AND WINNEBAGO STREET.
ELEVATION = 671.92

## UTILITY DISCLAIMER:

THE LOCATIONS, SIZES, AND TYPES OF UNDERGROUND PUBLIC AND PRIVATE UTILITIES OR SUBSTRUCTURES SHOW HEREON WERE OBTAINED FROM VISUAL INSPECTION, FIELD MEASUREMENTS, AND/OR AS-BUILT PLANS. SANITARY SEWER AND STORM SEWER PIPE SIZES, INVERTS, DIRECTION, AND LOCATIONS BETWEEN MANHOLES ARE SUPPLEMENTED BY AS-BUILT PLANS AND/OR ESTIMATED BASED ON FIELD OBSERVATIONS. PRIOR TO CONSTRUCTION IN THE VICINITY OF ANY UTILITIES SHOWN HEREON, IT IS RECOMMENDED THAT THE LOCATIONS, DEPTHS, AND SIZES BE FIELD VERIFIED. THE LOCATIONS SHOWN HEREON ARE ONLY APPROXIMATE, WITH POSSIBILITY THAT ADDITIONAL UTILITY LINES NOT DISCOVERED, OR MARKED, DURING THE SEARCH OF RECORDS AND THE FIELD SURVEY MAY EXIST. ANY CONTRACTOR USING THE INFORMATION SHOWN HEREON IS HEREBY FOREWARNED THAT ANY EXCAVATION UPON THIS SITE MAY RESULT IN THE DISCOVERY OF ADDITIONAL UNDERGROUND UTILITIES NOT SHOWN HEREON. IN GENERAL, UNDERGROUND UTILITY LOCATIONS ARE SHOWN FROM UTILITY MARKINGS, BY OTHERS, AND/OR AS-BUILT PLANS, PROVIDED BY OTHERS. POINT OF BEGINNING MAKES NO WARRANTY OF ANY KIND, EXPRESS OR IMPLIED, WITH RESPECT TO THE EXISTING UTILITIES SHOWN HEREON, AND BELIEVES THAT THE INFORMATION CONTAINED HEREIN IS RELIABLE AND GENERALLY ACCURATE FOR THE PURPOSE INTENDED.

0' 30' 60'
1" = 30'

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LA CORSSE SCHOOL DISTRICT
807 EAST AVE. S.
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REVISIONS:

| | DATE | DESCRIPTION |
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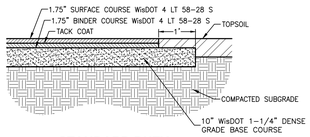
Project Number: 3752

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Sheet Title: Utility Plan

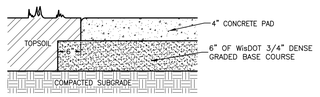
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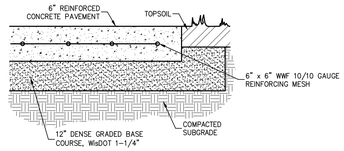



STANDARD DUTY

## ASPHALT PAVEMENT — 1 / C6.0

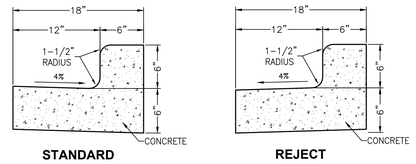



CONCRETE WALK



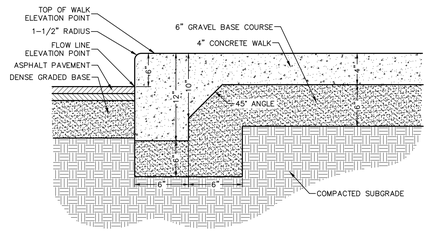

REINFORCED CONCRETE

## CONCRETE PAVEMENT — 2 / C6.0

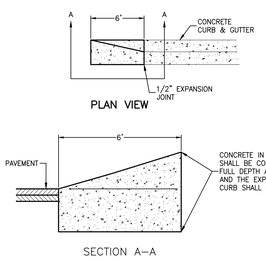



STANDARD REJECT

## CONCRETE CURB & GUTTER — 3 / C6.0




## THICKENED EDGE WALK — 4 / C6.0

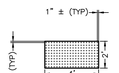



PLAN VIEW




SECTION A–A

## CURB HEAD TAPER SECTION — 5 / C6.0

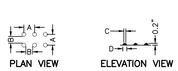



PLAN VIEW

DETECTABLE WARNING FIELD (TYPICAL)

| | MIN. | MAX. |
|---|---|---|
| A | 1.5" | 2.4" |
| B | 0.65" | 1.5" |
| C | * | * |
| D | 0.9" | 1.4" |

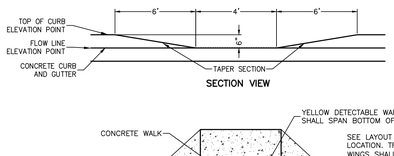
* THE C DIMENSION IS 50% TO 65% OF THE D DIMENSION



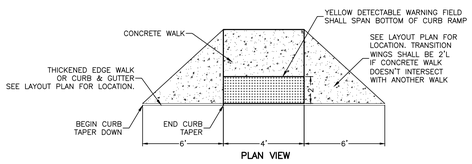

PLAN VIEW ELEVATION VIEW

TRUNCATED DOMES

DETECTABLE WARNING PATTERN DETAIL

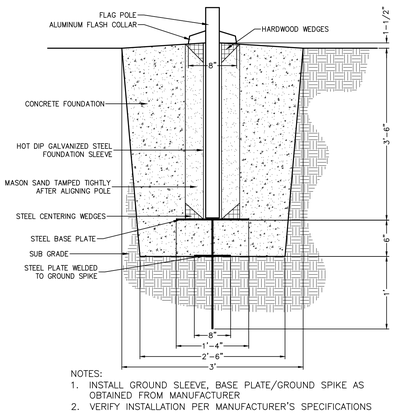



SECTION VIEW




PLAN VIEW

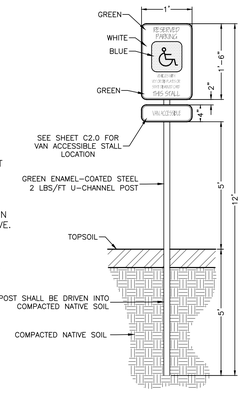
## ADA RAMP — 6 / C6.0



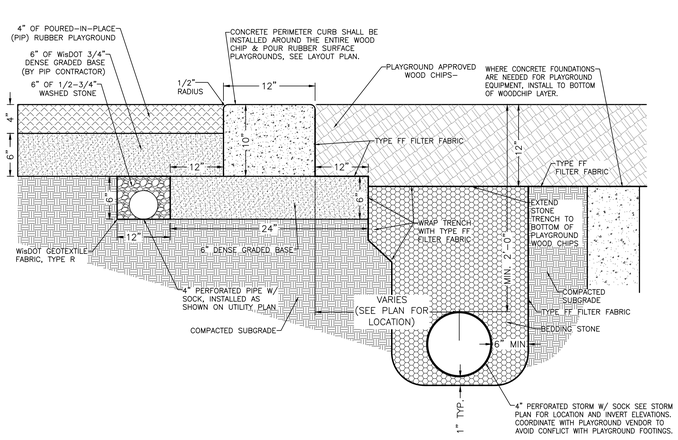

NOTES:
1. INSTALL GROUND SLEEVE, BASE PLATE/GROUND SPIKE AS OBTAINED FROM MANUFACTURER
2. VERIFY INSTALLATION PER MANUFACTURER'S SPECIFICATIONS

## FLAG POLE FOUNDATION — 7 / C6.0

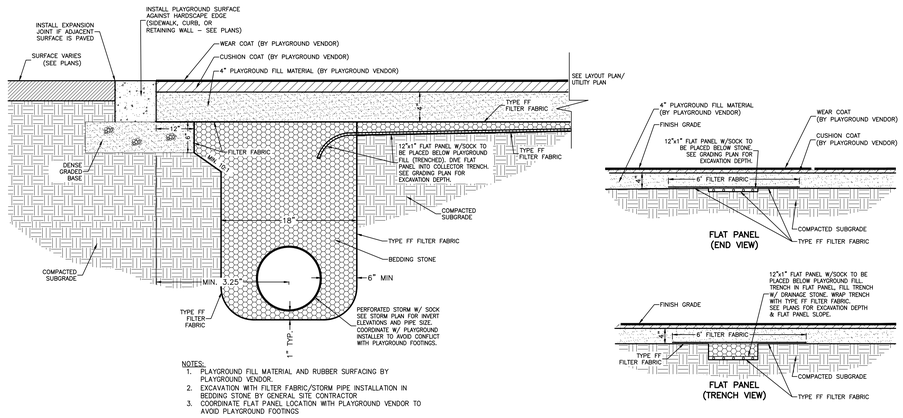
NOTES:
1. IN ACCORDANCE WITH WI TRANS 200.07(2)(a), THE SIGN SHALL CONSIST OF A WHITE RECTANGLE WITH LONGER DIMENSION VERTICAL, HAVING GREEN MESSAGE, AND A BLUE AND WHITE INTERNATIONAL SYMBOL FOR THE BARRIER–FREE ENVIRONMENTS. THE SIGN MAY BE REFLECTIVE OR NON–REFLECTIVE.
2. THE SIGN MUST INCLUDE THE WORDS:
   –RESERVED PARKING
   –VEHICLES WITH VET OR DIS PLATES OR STATE DISABLED CARD THIS STALL




## ADA PARKING SIGN — 8 / C6.0




## PLAY AREA CURBING & DRAINAGE — 9 / C6.0




NOTES:
1. PLAYGROUND FILL MATERIAL AND RUBBER SURFACING BY PLAYGROUND VENDOR.
2. EXCAVATION WITH FILTER FABRIC/STORM PIPE INSTALLATION IN BEDDING STONE BY GENERAL SITE CONTRACTOR
3. COORDINATE FLAT PANEL LOCATION WITH PLAYGROUND VENDOR TO AVOID PLAYGROUND FOOTINGS

## COURTYARD POURED IN PLACE RUBBER PLAY AREA — 10 / C6.0

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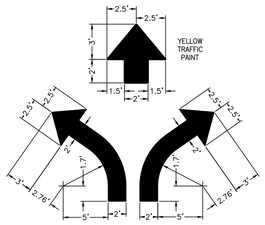
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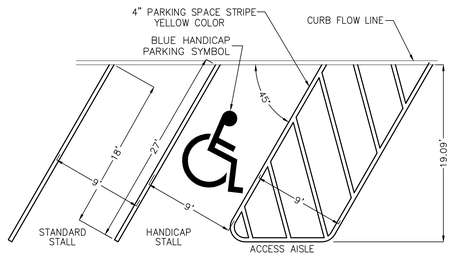
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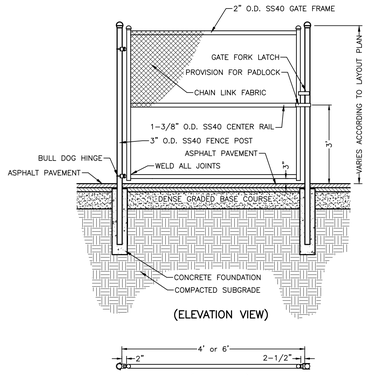
## DIRECTIONAL ARROWS — 1 / C6.1




## PARKING LOT STRIPING — 2 / C6.1

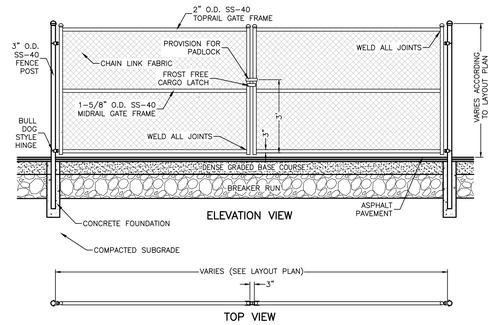



## PEDESTRIAN GATE — 3 / C6.1




NOTES:
1. SEE LAYOUT PLAN FOR GATE LOCATIONS

## SERVICE GATE — 4 / C6.1




NOTES:
1. SEE LAYOUT PLAN FOR GATE LOCATIONS

## CORNER FENCE POST — 5 / C6.1




NOTES:
1. ALL GATES WITH A SWING OF 6' OR LARGER SHALL HAVE A CONCRETE FOUNDATION AS SHOWN ABOVE.
2. SEE LAYOUT PLAN FOR GATE LOCATIONS

## DRIVEN FENCE POST — 6 / C6.1




NOTES:
1. SEE LAYOUT PLAN FOR FENCE LOCATIONS.
2. ALL CHAIN LINK FENCE POSTS SHALL BE DRIVEN.
3. DRIVEN FENCE POSTS INSTALLED WITHIN CONCRETE SHALL BE:
   3.1. CORED, DRIVEN, AND SEALED WITH CONCRETE JOINT SEALANT; OR
   3.2. SET PRIOR TO POURING CONCRETE, WRAPPED WITH EXPANSION FOAM, AND SEALED WITH CONCRETE JOINT SEALANT (SILICONE JOINT SEALANT MEETING ASTM C-920 TYPE S CLASS 25)
   3.3. BEFORE DRIVING A FENCE POST IN A CONCRETE AREA, IF THE FENCE POST IS LOCATED WITHIN 2 FEET OF THE EDGE OF CONCRETE, SAWCUT A CONTROL JOINT FROM THE FENCE POST TO THE EDGE OF CONCRETE.
4. DRIVEN FENCE POSTS INSTALLED WITHIN ASPHALT PAVEMENT SHALL BE CORED, DRIVEN, AND SEALED WITH CONCRETE JOINT SEALANT.

## SIGN INSTALLATION — 7 / C6.1




| POST EMBEDMENT DEPTH | |
|---|---|
| AREA OF SIGN INSTALLATION (SQ. FT.) | D (MIN.) |
| 20 OR LESS | 4' |
| > THAN 20 | 5' |

NOTES:
1. IF APPLICABLE, SIGNS SHALL CONFORM TO MUTCD STANDARDS INCLUDING RETRO REFLECTIVITY STANDARDS.
2. SIGNS SHALL BE EPOXY.
3. SIGNS WIDER THAN 4 FEET OR LARGER THAN 20 SQ. FT. SHALL BE MOUNTED ON MULTIPLE POSTS.

## CONCRETE WASHOUT — 8 / C6.1







NOTES:
1. DO NOT LOCATE AN UNLINED CONCRETE WASHOUT AREA (CWA) WITHIN 400' OF ANY NATURAL DRAINAGE PATHWAY OR WATERBODY. DO NOT LOCATE WITHIN 1,000 OF ANY WELLS OR DRINKING WATER SOURCES. IF SITE CONSTRAINTS MAKE THIS INFEASIBLE, OR IF HIGHLY PERMEABLE SOILS EXIST ON SITE, THE CWA MUST BE INSTALLED WITH AN IMPERMEABLE LINER (16 MIL MIN. THICKNESS) OR SURFACE STORAGE ALTERNATIVES USING PREFABRICATED CONCRETE WASHOUT DEVICES OR A LINED ABOVE GROUND STORAGE SHOULD BE USED.
2. THE CWA SHALL BE INSTALLED PRIOR TO CONCRETE PLACEMENT ON SITE.
3. CWA SHALL INCLUDE A FLAT SUBSURFACE PIT THAT IS AT LEAST 8' BY 8' WITH SLOPES LEADING OUT OF THE SUBSURFACE PIT SHALL BE 3:1. THE PIT SHALL BE AT LEAST 3' DEEP.
4. BERM SURROUNDING SIDES AND BACK OF CWA SHALL HAVE A MINIMUM HEIGHT OF 1'.
5. VEHICLE TRACKING PAD SHALL BE SLOPED 2% TOWARDS CWA PIT.
6. SIGNS SHALL BE PLACED AT THE CONSTUCTION ENTRACE, AT THE CWA, AND ELSEWHERE AS NECESSARY TO CLEARLY INDICATE THE LOCATION OF THE CWA TO OPERATORS OF CONCRETE TRUCKS AND PUMP RIGS.

## SILT FENCE — 9 / C6.1




NOTES:
1. HORIZONTAL BRACE WITH 3"x4" WOODEN FRAME OR EQUIVALENT AT TOP OF POST AS DIRECTED BY THE ENGINEER.
2. TRENCH SHALL BE A MINIMUM OF 4" WIDE BY 6" DEEP TO BURY AND ANCHOR THE GEOTEXTILE FABRIC. FOLD MATERIAL TO FIT TRENCH AND BACKFILL. NEXT, COMPACT THE TRENCH WITH EXCAVATED SOIL.
3. WOOD POSTS SHALL BE A MINIMUM OF 1-1/8" x 1-1/8" IN SIZE AND BE MADE FROM OAK OR HICKORY.
4. ATTACH THE FABRIC TO THE POST WITH WIRE STAPLES OR WOODEN LATH AND NAILS.
5. WOOD POSTS SHALL MAINTAIN A 3' MAXIMUM SPACING WHEN USING STANDARD GEOTEXTILE FABRIC. WHEN WOVEN GEOTEXTILE FABRIC IS USED, THE WOOD POSTS MAY BE SPACED UP TO 8' APART.

## INLET PROTECTION — 10 / C6.1




NOTES:
1. USE TYPE D INLET PROTECTION FOR ALL NON-CURB INLETS
2. USE FLEXSTORM'S CATCH-IT PRODUCT OR APPROVED EQUAL WITH RIGID FRAME.
3. DO NOT BLOCK THE TOP HALF OF THE CURB FACE OPENING

## EROSION CONTROL BLANKET — 11 / C6.1




NOTES:
1. PREPARE SOIL BEFORE INSTALLING MAT, INCLUDING APPLICATION OF LIME, FERTILIZER, AND SEED. NOTE: WHEN USING CELL-O-SEED DO NOT SEED PREPARED AREA. CELL-O-SEED MUST BE INSTALLED WITH PAPER SIDE DOWN.
2. BEGIN AT THE TOP OF THE SLOPE BY ANCHORING THE BLANKET IN A 6" DEEP BY 6" WIDE TRENCH. BACKFILL AND COMPACT THE TRENCH AFTER STAPLING.
3. ROLL THE BLANKETS DOWN THE SLOPE IN THE DIRECTION OF THE WATER FLOW.
4. THE EDGES OF PARALLEL BLANKETS MUST BE STAPLED WITH APPROXIMATELY 2" OVERLAP. WHEN BLANKETS MUST BE SPLICED DOWN THE SLOPE, PLACE BLANKETS END OVER END (SHINGLE STYLE) WITH APPROXIMATELY 6" OVERLAP. STAPLE THROUGH OVERLAPPED AREA APPROXIMATELY 12" APART.
5. EROSION CONTROL MAT SHALL BE LISTED ON THE CURRENT WisDOT PRODUCT ACCEPTABILITY LIST (PAL) AS A CLASS I TYPE A EROSION MAT.

## ROCK CONSTRUCTION ENTRANCE — 12 / C6.1




NOTES:
1. THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHT-OF-WAYS. THIS MAY REQUIRE TOP DRESSING, REPAIR AND/OR CLEANOUT ANY MEASURES USED TO TRAP SEDIMENT.
2. WHEN NECESSARY, WHEELS SHALL BE CLEANED PRIOR TO ENTRANCE ONTO PUBLIC RIGHT-OF-WAY.

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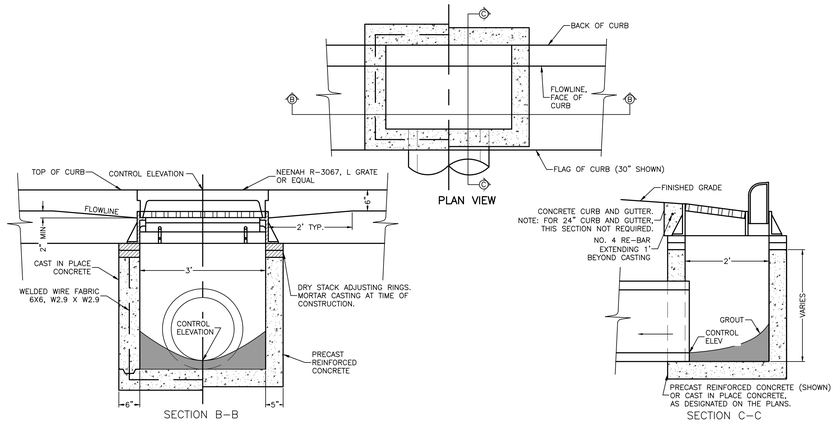
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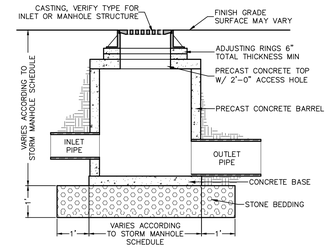
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## STORM CURB INLET

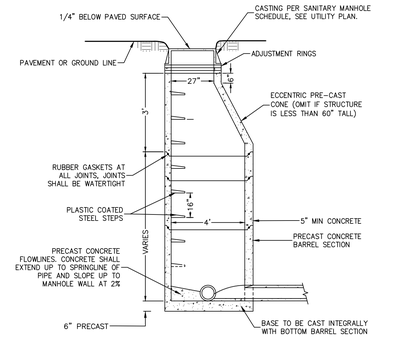
1 C6.2




NOTES:
1. MANHOLES STEPS SHALL BE INSTALLED IN ALL STORM STRUCTURES EXCEEDING A TOTAL HEIGHT OF 5.0'.

## STORM MANHOLE

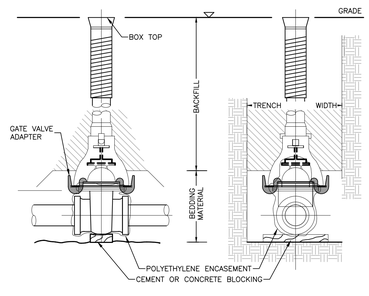
2 C6.2




NOTES:
1. MANHOLES SHALL BE PRECAST REINFORCED CONCRETE, MANUFACTURED TO MEET ASTM C-478.
2. CONNECT PIPES TO MANHOLE WITH WATER-TIGHT RUBBER BOOT CONNECTORS, DESIGNED FOR WASTEWATER APPLICATIONS AND APPLICABLE PIPE MATERIAL. INSTALL PER MANUFACTURER GUIDELINES.

## SANITARY MANHOLE

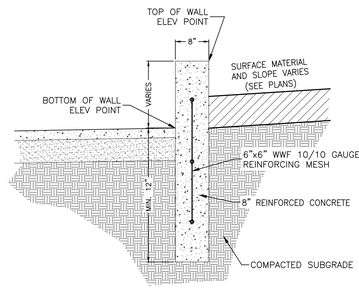
3 C6.2




NOTES:
1. VALVE SHALL BE WATEROUS 2500 SERIES, PER SPSS 601.1.4, OR CITY APPROVED EQUAL.

## WATER VALVE

4 C6.2




## 8" CONCRETE CURB

5 C6.2

| PROPOSED LAYOUT | | PROPOSED ELEVATIONS | | PART TYPE | ITEM ON LAYOUT | DESCRIPTION | *INVERT ABOVE BASE OF CHAMBER | |
|---|---|---|---|---|---|---|---|---|
| 270 | STORMTECH SC-800 CHAMBERS | MAXIMUM ALLOWABLE GRADE (TOP OF PAVEMENT/UNPAVED) | 676.33 | | | | INVERT | MAX FLOW |
| 18 | STORMTECH SC-800 END CAPS | MINIMUM ALLOWABLE GRADE (UNPAVED WITH TRAFFIC) | 673.25 | 36"DSE-CORED END CAP | A | 12" TOP PRE-CORED END CAP, PARTR SC800EPE12TRC / TYP OF ALL 12" TOP CONNECTIONS | 14.40" | |
| 6 | STONE ABOVE (in) | MINIMUM ALLOWABLE GRADE (UNPAVED W/ FINAL ASPHALT) | 669.75 | 24" | B | 24" BOTTOM PREFABRICATED EZ END CAP, PARTR SC800CCEZ / TYP OF ALL 24" BOTTOM | 2.30" | |
| 9 | STONE BELOW (in) | MINIMUM ALLOWABLE GRADE (UNPAVED TRAFFIC PAVEMENT) | 669.38 | PREFABRICATED EZ END CAP | B | CONNECTIONS AND ISOLATOR PLUS ROWS | 2.30" | |
| 40 | STONE VOID | MINIMUM ALLOWABLE GRADE (BASE OF FLEXIBLE PAVEMENT) | 669.13 | | C | INSTALL FLAMP ON 24" ACCESS PIPE / PARTR SCB00024RAMP | | |
| 24099 | INSTALLED SYSTEM VOLUME (CF) (PERIMETER STONE INCLUDED) | TOP OF STONE | 669.13 | | C | INSTALL FLAMP ON 24" ACCESS PIPE / PARTR SCB00024RAMP | | |
| | | TOP OF SC-800 CHAMBER | 668.13 | MANIFOLD | D | 12" x 12" TOP MANIFOLD, ADS N-12 | 14.40" | |
| | (COVER STONE INCLUDED) | BOTTOM OF SC-800 CHAMBER/MANIFOLD INVERT | 667.63 | MANIFOLD | D | 12" x 12" BOTTOM MANIFOLD, ADS N-12 | 2.30" | |
| | (BASE STONE INCLUDED) | 24" x 24" BOTTOM MANIFOLD INVERT | 667.63 | 24" CONNECTION | E | 24" BOTTOM CONNECTION | 2.30" | |
| 10392 | SYSTEM AREA (SF) | 24" x 24" BOTTOM MANIFOLD INVERT | 667.63 | NYLOPLAST (INLET W/ ISO GRATE) | G | 30" DIAMETER (24.00" SUMP MIN) | | 5.9 CFS IN |
| 555.6 | SYSTEM PERIMETER (ft) | 24" BOTTOM CONNECTION INVERT | 668.38 | PLUS ROW) | | | | |
| | | BOTTOM OF SC-800 CHAMBER | 669.13 | NYLOPLAST (OUTLET) | H | 30" DIAMETER (DESIGN BY ENGINEER) | | 14.0 CFS OUT |
| | | UNDERDRAIN INVERT | 665.25 | UNDERDRAIN | I | 6" ADS N-12 DUAL WALL PERFORATED HDPE UNDERDRAIN | | |
| | | BOTTOM OF STONE | 665.25 | INSPECTION PORT | J | 4" SEE DETAIL | | |




**NOTES**
- THE SITE DESIGN ENGINEER MUST REVIEW ELEVATIONS AND IF NECESSARY ADJUST GRADING TO ENSURE THE CHAMBER COVER REQUIREMENTS ARE MET.
- **NOT FOR CONSTRUCTION** THIS LAYOUT IS FOR DIMENSIONAL PURPOSES ONLY TO PROVE CONCEPT & THE REQUIRED STORAGE VOLUME CAN BE ACHIEVED ON SITE.

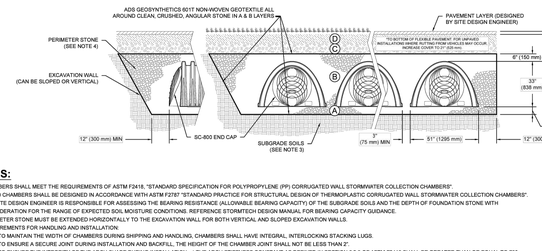
## STORMTECH CHAMBER SYSTEM

7 C6.2

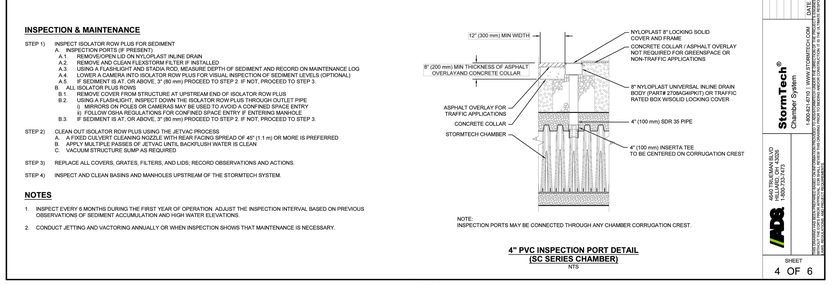
### ACCEPTABLE FILL MATERIALS: STORMTECH SC-800 CHAMBER SYSTEMS

| | MATERIAL LOCATION | DESCRIPTION | AASHTO MATERIAL CLASSIFICATIONS | COMPACTION / DENSITY REQUIREMENT |
|---|---|---|---|---|
| D | **FINAL FILL:** FILL MATERIAL FOR LAYER 'D' STARTS FROM THE TOP OF THE 'C' LAYER TO THE BOTTOM OF FLEXIBLE PAVEMENT OR UNPAVED FINISHED GRADE ABOVE. NOTE THAT PAVEMENT SUBBASE MAY BE PART OF THE 'D' LAYER. | ANY SOIL/ROCK MATERIALS, NATIVE SOILS, OR PER ENGINEER'S PLANS. CHECK PLANS FOR PAVEMENT SUBGRADE REQUIREMENTS. | N/A | PREPARE PER SITE DESIGN ENGINEER'S PLANS. PAVED INSTALLATIONS MAY HAVE STRINGENT MATERIAL AND PREPARATION REQUIREMENTS. |
| C | **INITIAL FILL:** FILL MATERIAL FOR LAYER 'C' STARTS FROM THE TOP OF THE EMBEDMENT STONE ('B' LAYER) TO 15" (375 mm) ABOVE THE TOP OF THE CHAMBER. NOTE THAT PAVEMENT SUBBASE MAY BE A PART OF THE 'C' LAYER. | GRANULAR WELL-GRADED SOIL/AGGREGATE MIXTURES, <35% FINES OR PROCESSED AGGREGATE.<br>MOST PAVEMENT SUBBASE MATERIALS CAN BE USED IN LIEU OF THIS LAYER. | AASHTO M145[4]<br>A-1, A-2-4, A-3<br>OR<br>AASHTO M43[4]<br>3, 357, 4, 467, 5, 56, 57, 6, 67, 68, 7, 78, 8, 89, 9, 10 | BEGIN COMPACTIONS AFTER 12" (300 mm) OF MATERIAL OVER THE CHAMBERS IS REACHED. COMPACT ADDITIONAL LAYERS IN 6" (150 mm) MAX LIFTS TO A MIN. 95% PROCTOR DENSITY FOR WELL GRADED MATERIAL AND 95% RELATIVE DENSITY FOR PROCESSED AGGREGATE MATERIALS. ROLLER GROSS VEHICLE WEIGHT NOT TO EXCEED 12,000 lbs (53 kN). DYNAMIC FORCE NOT TO EXCEED 20,000 lbs (89 kN). |
| B | **EMBEDMENT STONE:** FILL SURROUNDING THE CHAMBERS FROM THE FOUNDATION STONE ('A' LAYER) TO THE 'C' LAYER ABOVE. | CLEAN, CRUSHED, ANGULAR STONE OR RECYCLED CONCRETE[5] | AASHTO M43[4]<br>3, 357, 4, 467, 5, 56, 57 | NO COMPACTION REQUIRED. |
| A | **FOUNDATION STONE:** FILL BELOW CHAMBERS FROM THE SUBGRADE UP TO THE FOOT (BOTTOM) OF THE CHAMBER. | CLEAN, CRUSHED, ANGULAR STONE OR RECYCLED CONCRETE[5] | AASHTO M43[4]<br>3, 357, 4, 467, 5, 56, 57 | PLATE COMPACT OR ROLL TO ACHIEVE A FLAT SURFACE[2,3] |

PLEASE NOTE:
1. THE LISTED AASHTO DESIGNATIONS ARE FOR GRADATIONS ONLY. THE STONE MUST ALSO BE CLEAN, CRUSHED, ANGULAR. FOR EXAMPLE, A SPECIFICATION FOR #4 STONE WOULD STATE: "CLEAN, CRUSHED, ANGULAR NO. 4 (AASHTO M43) STONE".
2. STORMTECH COMPACTION REQUIREMENTS ARE MET FOR 'A' LOCATION MATERIALS WHEN PLACED AND COMPACTED IN 6" (150 mm) (MAX) LIFTS USING TWO FULL COVERAGES WITH A VIBRATORY COMPACTOR.
3. WHERE INFILTRATION SURFACES MAY BE COMPROMISED BY COMPACTION, FOR STANDARD DESIGN LOAD CONDITIONS, A FLAT SURFACE MAY BE ACHIEVED BY RAKING OR DRAGGING WITHOUT COMPACTION EQUIPMENT. FOR SPECIAL LOAD DESIGNS, CONTACT STORMTECH FOR COMPACTION REQUIREMENTS.
4. ONCE LAYER 'C' IS PLACED, ANY SOIL/MATERIAL CAN BE PLACED IN LAYER 'D' UP TO THE FINISHED GRADE. MOST PAVEMENT SUBBASE SOILS CAN BE USED TO REPLACE THE MATERIAL REQUIREMENTS OF LAYER 'C' OR 'D' AT THE SITE DESIGN ENGINEER'S DISCRETION.
5. WHERE RECYCLED CONCRETE AGGREGATE IS USED IN LAYERS 'A' OR 'B' THE MATERIAL SHOULD ALSO MEET THE ACCEPTABILITY CRITERIA OUTLINED IN TECHNICAL NOTE 6.20 "RECYCLED CONCRETE STRUCTURAL BACKFILL".




**NOTES:**
1. CHAMBERS SHALL MEET THE REQUIREMENTS OF ASTM F2418, "STANDARD SPECIFICATION FOR POLYPROPYLENE (PP) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
2. SC-800 CHAMBERS SHALL BE DESIGNED IN ACCORDANCE WITH ASTM F2787 "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
3. THE SITE DESIGN ENGINEER IS RESPONSIBLE FOR ASSESSING THE BEARING RESISTANCE (ALLOWABLE BEARING CAPACITY) OF THE SUBGRADE SOILS AND THE DEPTH OF FOUNDATION STONE WITH CONSIDERATION FOR THE RANGE OF EXPECTED SOIL MOISTURE CONDITIONS. REFERENCE STORMTECH DESIGN MANUAL FOR BEARING CAPACITY GUIDANCE.
4. PERIMETER STONE MUST BE EXTENDED HORIZONTALLY TO THE EXCAVATION WALL FOR BOTH VERTICAL AND SLOPED EXCAVATION WALLS.
5. REQUIREMENTS FOR HANDLING AND INSTALLATION:
   - TO MAINTAIN THE WIDTH OF CHAMBERS DURING SHIPPING AND HANDLING, CHAMBERS SHALL HAVE INTEGRAL, INTERLOCKING STACKING LUGS.
   - TO ENSURE A SECURE JOINT DURING INSTALLATION AND BACKFILL, THE HEIGHT OF THE CHAMBER JOINT SHALL NOT BE LESS THAN 2".
   - TO ENSURE THE INTEGRITY OF THE ARCH SHAPE DURING INSTALLATION, a) THE ARCH STIFFNESS CONSTANT AS DEFINED IN SECTION 6.2.8 OF ASTM F2418 SHALL BE GREATER THAN OR EQUAL TO 750 LBS/FT/%, AND b) TO RESIST CHAMBER DEFORMATION DURING INSTALLATION AT ELEVATED TEMPERATURES (ABOVE 73° F / 23° C), CHAMBERS SHALL BE PRODUCED FROM REFLECTIVE GOLD OR YELLOW COLORS.




**SC-800 ISOLATOR ROW PLUS DETAIL**
NTS

**INSPECTION & MAINTENANCE**

STEP 1) INSPECT ISOLATOR ROW PLUS FOR SEDIMENT
- A. INSPECTION PORTS (IF PRESENT)
  - A.1. REMOVE/OPEN LID ON NYLOPLAST INLINE DRAIN
  - A.2. REMOVE AND CLEAN FLEXSTORM FILTER IF INSTALLED
  - A.3. USING A FLASHLIGHT AND STADIA ROD, MEASURE DEPTH OF SEDIMENT AND RECORD ON MAINTENANCE LOG
  - A.4. LOWER A CAMERA INTO ISOLATOR ROW PLUS FOR VISUAL INSPECTION OF SEDIMENT LEVELS (OPTIONAL)
  - A.5. IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.
- B. ALL ISOLATOR ROWS
  - B.1. REMOVE COVER FROM STRUCTURE AT UPSTREAM END OF ISOLATOR ROW PLUS
  - B.2. USING A FLASHLIGHT, INSPECT DOWN THE ISOLATOR ROW PLUS THROUGH OUTLET PIPE
    - i) MIRRORS ON POLES OR CAMERAS MAY BE USED TO AVOID A CONFINED SPACE ENTRY
    - ii) FOLLOW OSHA REGULATIONS FOR CONFINED SPACE ENTRY IF ENTERING MANHOLE
  - B.3. IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.

STEP 2) CLEAN OUT ISOLATOR ROW PLUS USING THE JETVAC PROCESS
- A. A FIXED CULVERT CLEANING NOZZLE WITH REAR FACING SPREAD OF 45° (1 PLUG) OR MORE IS PREFERRED
- B. APPLY MULTIPLE PASSES OF JETVAC UNTIL BACKFLUSH WATER IS CLEAN
- C. VACUUM STRUCTURE SUMP AS REQUIRED

STEP 3) REPLACE ALL COVERS, GRATES, FILTERS, AND LIDS; RECORD OBSERVATIONS AND ACTIONS.

STEP 4) INSPECT AND CLEAN BASINS AND MANHOLES UPSTREAM OF THE STORMTECH SYSTEM.

**NOTES**
1. INSPECT EVERY 6 MONTHS DURING THE FIRST YEAR OF OPERATION. ADJUST THE INSPECTION INTERVAL BASED ON PREVIOUS OBSERVATIONS OF SEDIMENT ACCUMULATION AND HIGH WATER ELEVATIONS.
2. CONDUCT JETTING AND VACTORING ANNUALLY OR WHEN INSPECTION SHOWS THAT MAINTENANCE IS NECESSARY.



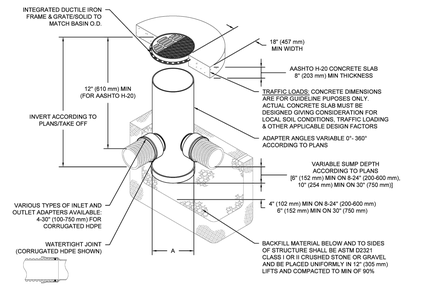

NOTE:
INSPECTION PORTS MAY BE CONNECTED THROUGH ANY CHAMBER CORRUGATION CREST.

**4" PVC INSPECTION PORT DETAIL (SC SERIES CHAMBER)**
NTS

## STORMTECH CHAMBER SYSTEM

8 C6.2

**NYLOPLAST DRAIN BASIN**
NTS




**NOTES**
1. 8-30" (200-750 mm) GRATES/SOLID COVERS SHALL BE DUCTILE IRON PER ASTM A536 GRADE 70-50-05
2. 12-30" (300-750 mm) FRAMES SHALL BE DUCTILE IRON PER ASTM A536 GRADE 70-50-05
3. DRAIN BASIN TO BE CUSTOM MANUFACTURED ACCORDING TO PLAN DETAILS
4. DRAINAGE CONNECTION STUB JOINT TIGHTNESS SHALL CONFORM TO ASTM D3212 FOR CORRUGATED HDPE (ADS & HANCOR DUAL WALL) & SDR 35 PVC
5. FOR COMPLETE DESIGN AND PRODUCT INFORMATION: **WWW.NYLOPLAST-US.COM**
6. TO ORDER CALL: **800-821-6710**

| A | PART # | GRATE/SOLID COVER OPTIONS | | |
|---|---|---|---|---|
| 8" (200 mm) | 2808AG | PEDESTRIAN LIGHT DUTY | STANDARD LIGHT DUTY | SOLID LIGHT DUTY |
| 10" (250 mm) | 2810AG | PEDESTRIAN LIGHT DUTY | STANDARD LIGHT DUTY | SOLID LIGHT DUTY |
| 12" (300 mm) | 2812AG | PEDESTRIAN AASHTO H-10 | STANDARD AASHTO H-20 | SOLID AASHTO H-20 |
| 15" (375 mm) | 2815AG | PEDESTRIAN AASHTO H-10 | STANDARD AASHTO H-20 | SOLID AASHTO H-20 |
| 18" (450 mm) | 2818AG | PEDESTRIAN AASHTO H-10 | STANDARD AASHTO H-20 | SOLID AASHTO H-20 |
| 24" (600 mm) | 2824AG | PEDESTRIAN AASHTO H-10 | STANDARD AASHTO H-20 | SOLID AASHTO H-20 |
| 30" (750 mm) | 2830AG | PEDESTRIAN AASHTO H-10 | STANDARD AASHTO H-20 | SOLID AASHTO H-20 |

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Project Title:
**ADDITIONS AND ALTERATIONS TO:**
**LA CROSSE NEW ELEMENTARY SCHOOL**
**LA CORSSE SCHOOL DISTRICT**
**807 EAST AVE. S.**
**LA CROSSE, WI 54601**

REVISIONS:

| | DATE | DESCRIPTION |
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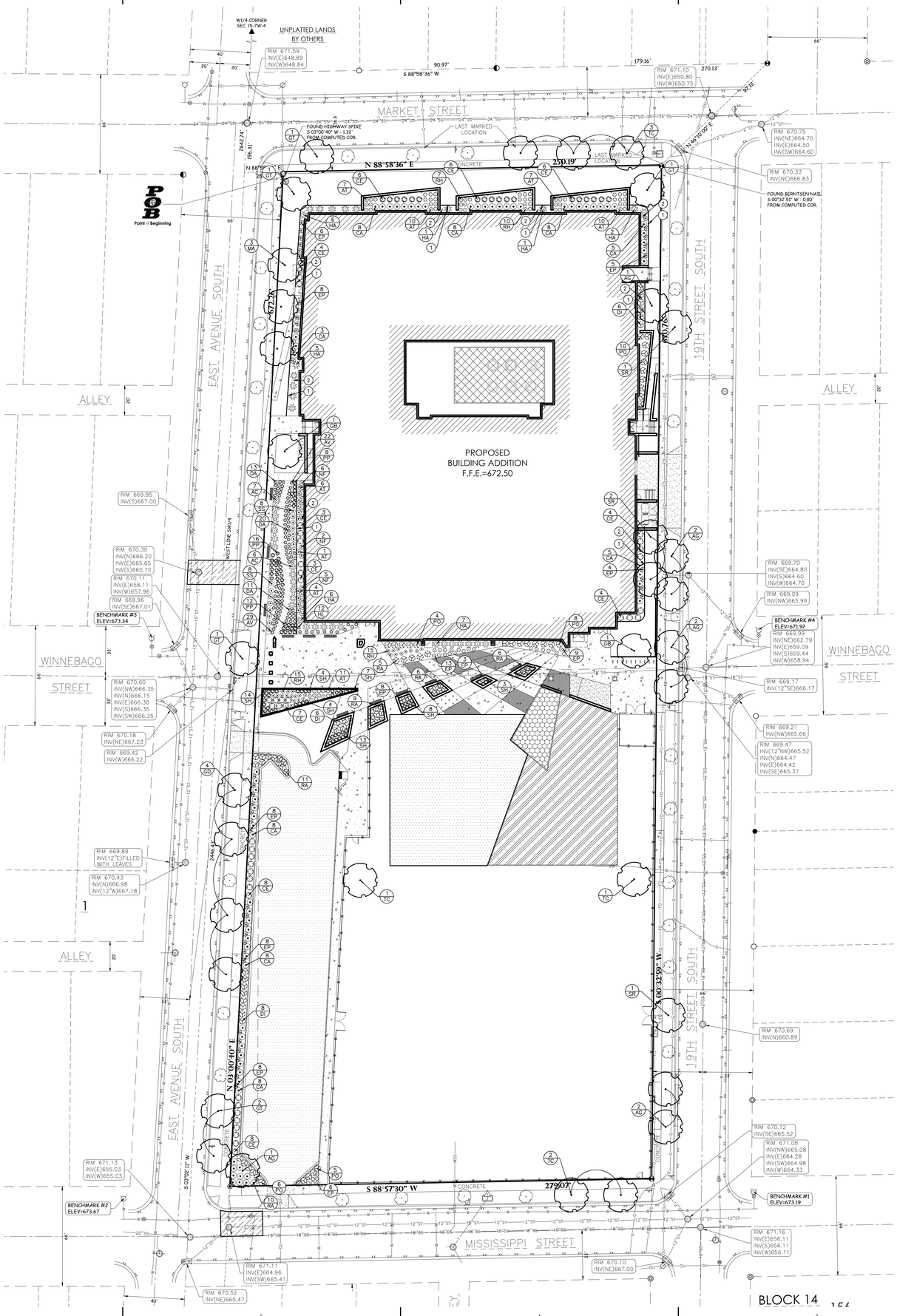
Project Number:
**3752**

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**50% CONSTRUCTION DOCUMENTS**
**08/19/2025**

Sheet Title:
**Details**

Sheet Number:
**C6.2**

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# GENERAL NOTES:

1. CONTACT DIGGER'S HOTLINE 5 WORKING DAYS PRIOR TO THE START OF DEMOLITION/CONSTRUCTION.
2. 6" OF TOPSOIL SHALL BE PROVIDED IN ALL GENERAL LANDSCAPE AREAS. LANDSCAPE CONTRACTOR SHALL VERIFY THAT SPECIFIED PLANTING SOIL DEPTH IS PRESENT PRIOR TO PLANTING.
3. SEED/FERTILIZE/CRIMP HAY MULCH ALL GENERAL LANDSCAPE AREAS DISTURBED DURING CONSTRUCTION EXCEPT ARES LABELED AS SYNTHETIC TURF.
4. ALL PLANT MATERIALS LISTED SHALL MEET THE STANDARDS OF THE AMERICAN NURSERY & LANDSCAPE ASSOCIATION FOR THE SIZES GIVEN.
5. ALL TREES SHALL BE STAKED WITH A MINIMUM OF THREE STAKES.
6. ALL TREES IN THE TURF AREA SHALL HAVE A 5' DIAMETER CIRCLE OF 4" DEPTH SHREDDED HARDWOOD BARK MULCH.
7. CURV-RITE LANDSCAPING EDGING (SERIES 3000 3/16" X 4", WITH MILL FINISH) OR APPROVED EQUAL SHALL BE PLACED AROUND ALL LANDSCAPE BEDS.
8. 3" DEPTH OF SHREDDED HARDWOOD BARK MULCH SHALL BE PLACED IN ALL LANDSCAPE PLANTING BEDS AS NOTED. PROVIDE A SAMPLE TO THE OWNER FOR REVIEW AND APPROVAL PRIOR TO INSTALLATION.
9. 20 YEAR WEED BARRIER FILTER FABRIC SHALL BE PLACED BENEATH ALL LANDSCAPE HARDWOOD BARK MULCH.
10. COORDINATE ALL LANDSCAPE WORK WITH GAS, ELECTRIC, (INCLUDING MAIN SERVICE, SITE LIGHTING, CONDUITS AND SIGNAGE) CABLE AND TELEPHONE CONSTRUCTION AND RESPECTIVE TRADES FOR THE INSTALLATION OF SAID UTILITIES.

# PLANTING SCHEDULE:

| TREES SYMBOLS | BOTANICAL NAME | COMMON NAME | INSTALLATION SIZE | SIZE AT MATURITY | QUANTITY |
|---|---|---|---|---|---|
| AG | AMELANCHIER X GRADIFLORA 'AUTUMN BRILLIANCE' | APPLE SERVICEBERRY | 2" CAL. | 25'TX20'W | 8 |
| GB | GINKGO BILOBA 'PRINCETON SENTRY' (MALE) | GINKGO | 2" CAL. | 50'TX25'W | 2 |
| GD | GYMNOCLADUS DIOICUS 'STATELY MANOR' | KENTUCKY COFFEE TREE | 2" CAL. | 60'TX40'W | 4 |
| GT | GLEDITSIA TRIACANTHOS 'SKYCOLE' | SKYLINE HONEYLOCUST | 2" CAL. | 45'TX35'W | 6 |
| MA | MAACKIA AMURENSIS | AMUR MAACKIA | 2" CAL. | 25'TX20'W | 3 |
| SR | SYRINGA RETICULATA 'IVORY SILK' | IVORY SILK JAPANESE TREE LILAC | 2" CAL. | 25'TX25'W | 4 |
| TC | TILIA CORDATA | LITTLE-LEAF LINDEN | 2" CAL. | 60'TX40'W | 7 |

| SHRUBS SYMBOLS | BOTANICAL NAME | COMMON NAME | INSTALLATION SIZE | SIZE AT MATURITY | QUANTITY |
|---|---|---|---|---|---|
| AT | ASCLEPIAS TUBEROSA | BUTTERFLY WEED | 5 GAL | 2'T-1'W | 56 |
| CE | CEANOTHUS AMERICANUS | NEW JERSEY TEA | 5 GAL | 4'T-4'W | 55 |
| DI | DIERVILLA IONICERA | DWARF BUSH HONEYSUCKLE | 5 GAL | 3'T-3'W | 20 |
| HA | HYDRANGEA ARBORESCENS 'ANNABELLE' | ANNABELLE HYDRANGEA | 5 GAL | 5'T-5'W | 28 |
| NF | NEPETA FAASSENII 'CAT'S MEOW' | CAT'S MEOW CATMINT | 5 GAL | 20"T-3'W | 13 |
| PO | PHYSOCARPUS OPULIFOLIUS 'SMPOTW' | TINY WINE NINEBARK | 5 GAL | 3'T-4'W | 34 |
| RA | RHUS AROMATICA 'GRO-LOW' | FRAGRANT SUMAC | 5 GAL | 2'T-6'W | 47 |

| PERENNIALS AND ORNAMENTAL GRASSES SYMBOLS | BOTANICAL NAME | COMMON NAME | INSTALLATION SIZE | SIZE AT MATURITY | QUANTITY |
|---|---|---|---|---|---|
| CA | CALAMAGROSTIS X ACUTIFLORA 'KARL FOERSTER' | KARL FOERSTER FEATHER REED GRASS | 24" | 5'-7'T | 61 |
| EP | ECHINACEA PURPUREA | PURPLE CONEFLOWER | 12" | 36"T-18"W | 87 |
| HL | HEMEROCALLIS X 'LITTLE GRAPETTE' | LITTLE GRAPETTE DWARF DAYLILY | 12" | 2'T-2'W | 12 |
| RH | RUDBECKIA HIRTA | BLACK-EYED SUSAN | 18" | 24"T-18"W | 41 |
| SH | SPOROBOLUS HETEROLEPIS | PRAIRIE DROPSEED | 12" | 24"T-24"W | 53 |

| DRY BASIN GARDEN PLANTINGS SYMBOLS | BOTANICAL NAME | COMMON NAME | INSTALLATION SIZE | SIZE AT MATURITY | QUANTITY |
|---|---|---|---|---|---|
| AC | AQUILEGIA CANADENSIS | COLUMBINE | 1 GAL | 3'T-2'W | 13 |
| AV | ANEMONE VIRGINIANA | THIMBLEWOOD | 1 GAL | 2'T-1'W | 51 |
| DA | DRYMOCALLIS ARGUTA | PRAIRIE CINQUEFOIL | 1 GAL | 3'T-2'W | 44 |
| PP | PHLOX PILOSA | PRAIRIE PHLOX | 1 GAL | 1'T-1'W | 36 |
| SS | SCHIZACHYRIUM SCOPARIUM | LITTLE BLUESTEM | 1 GAL | 3'T-2'W | 16 |

3'-0" WIDE DRY RIVER ROCK BED
1.5" - 2" MISSISSIPPI RIVER STONE

# (X) KEYNOTES:

1. LANDSCAPE EDGING – SEE GENERAL NOTES
2. LANDSCAPE HARDWOOD MULCH – SEE GENERAL NOTES

# LANDSCAPE REQUIREMENTS:

REQUIREMENT: ONE TREE PLACED IN THE BOULEVARD PER 40 LINEAR FEET OF LOT FRONTAGE.

TOTAL LOT FRONTAGE: 1871 LF = 46.7 | 47 TOTAL TREES REQUIRED

PROPOSED: 38 (17 EXISTING TRESS AND 21 PROPOSED)

REQUIREMENT: NOT LESS THAN TWO TREES AND EIGHT SHRUBS PER 600 SQUARE FEET OF LANDSCAPED AREA.

TOTAL LANDSCAPE AREA: 18,545 SF / 600 = 31 | 62 TOTAL TREE AND 248 SHRUBS REQUIRED

PROPOSED: 251 SHRUBS AND 14 TREES

# BENCHMARK:

ELEVATIONS ARE REFERENCED TO NAVD 88 DATUM.

**BENCHMARK #1**
NORTHWEST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHEAST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND 19TH STREET SOUTH.
ELEVATION = 673.19

**BENCHMARK #2**
NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND EAST AVENUE SOUTH.
ELEVATION = 673.67

**BENCHMARK #3**
NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST CORNER OF THE INTERSECTION OF EAST AVENUE SOUTH AND WINNEBAGO STREET.
ELEVATION = 673.34

**BENCHMARK #4**
BURY BOLT ON HYDRANT, LOCATED AT THE NORTHEAST CORNER OF THE INTERSECTION OF 19TH STREET SOUTH AND WINNEBAGO STREET.
ELEVATION = 671.92

# UTILITY DISCLAIMER:

THE LOCATIONS, SIZES, AND TYPES OF UNDERGROUND PUBLIC AND PRIVATE UTILITIES OR SUBSTRUCTURES SHOW HEREON WERE OBTAINED FROM VISUAL INSPECTION, FIELD MEASUREMENTS, AND/OR AS-BUILT PLANS. SANITARY SEWER AND STORM SEWER PIPE SIZES, INVERTS, DIRECTION, AND LOCATIONS BETWEEN MANHOLES ARE SUPPLEMENTED BY AS-BUILT PLANS AND/OR ESTIMATED BASED ON FIELD OBSERVATIONS. PRIOR TO CONSTRUCTION IN THE VICINITY OF ANY UTILITIES SHOWN HEREON, IT IS RECOMMENDED THAT THE LOCATIONS, DEPTHS, AND SIZES BE FIELD VERIFIED. THE LOCATIONS SHOWN HEREON ARE ONLY APPROXIMATE, WITH POSSIBILITY THAT ADDITIONAL UTILITY LINES NOT DISCOVERED, OR MARKED, DURING THE SEARCH OF RECORDS AND THE FIELD SURVEY MAY EXIST. ANY CONTRACTOR USING THE INFORMATION SHOWN HEREON IS HEREBY FOREWARNED THAT ANY EXCAVATION UPON THIS SITE MAY RESULT IN THE DISCOVERY OF ADDITIONAL UNDERGROUND UTILITIES NOT SHOWN HEREON. IN GENERAL, UNDERGROUND UTILITY LOCATIONS ARE SHOWN FROM UTILITY MARKINGS, BY OTHERS, AND/OR AS-BUILT PLANS, PROVIDED BY OTHERS. POINT OF BEGINNING MAKES NO WARRANTY OF ANY KIND, EXPRESS OR IMPLIED, WITH RESPECT TO THE EXISTING UTILITIES SHOWN HEREON, AND BELIEVES THAT THE INFORMATION CONTAINED HEREIN IS RELIABLE AND GENERALLY ACCURATE FOR THE PURPOSE INTENDED.

0' 30' 60'
1" = 30'


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ADDITIONS AND ALTERATIONS TO:
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LA CORSSE SCHOOL DISTRICT
807 EAST AVE. S.
LA CROSSE, WI 54601

REVISIONS:

| | DATE | DESCRIPTION |
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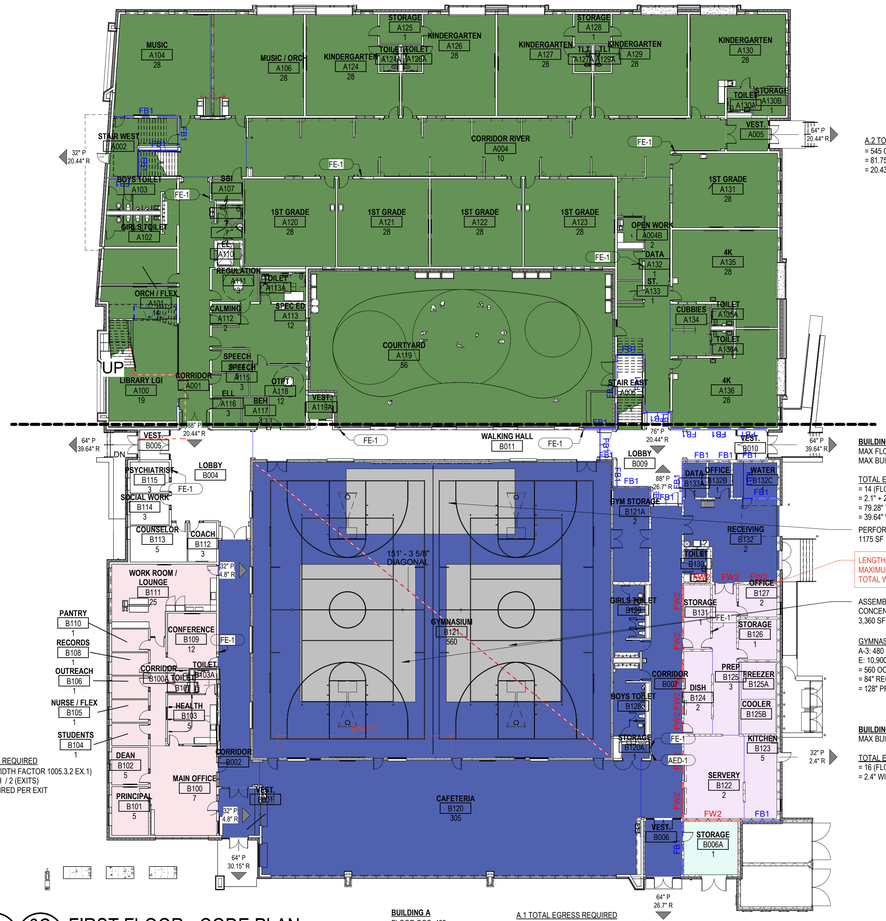
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Issued For: 50% CONSTRUCTION DOCUMENTS 08/19/2025

Sheet Title: Landscape Plan

Sheet Number: L1.0

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A.2 TOTAL EGRESS REQUIRED
= 545 OCC X 0.15 (WIDTH FACTOR 1005.3.2 EX.1)
= 81.75" TOTAL WIDTH / 4 (EXITS)
= 20.4375" WIDTH REQUIRED PER EXIT

**BUILDING A**
MAX FLOOR OCC: 1513
MAX BUILDING OCC: 2280

TOTAL EGRESS REQUIRED
= 14 (FLOOR OCC) X .15 (WIDTH FACTOR 1005.3.2 EX. 1)
= 2.1" + 26.7" (A.1) + 20.44" (2)(A.2) + 9.6" (A.3)
= 79.28" TOTAL WIDTH / 2 (EXITS)
= 39.64" WIDTH REQUIRED PER EXIT
PERFORMANCE AREA
1175 SF = 80 OCC

LENGTH OF FIREWALL: 127'-2"
MAXIMUM ALLOWABLE OPENINGS: 31'-9"
TOTAL WIDTH OF OPENINGS: 31'-0"

ASSEMBLY WITHOUT FIXED SEATS
CONCENTRATED CHAIRS ONLY
3,360 SF = 480 OCC

GYMNASIUM OCCUPANT LOAD BY TYPE
A-3: 480 + 80 = 560 OCC
E: 10,900 SF / 50 = 218 OCC
= 560 OCC X 0.15 (WIDTH FACTOR 1005.3.2 EX.1)
= 84" REQUIRED EXIT WIDTH
= 128" PROVIDED

**BUILDING B**
MAX BUILDING OCC: 16

TOTAL EGRESS REQUIRED
= 16 (FLOOR OCC) X .15 (WIDTH FACTOR 1005.3.2 EX. 1)
= 2.4" WIDTH REQUIRED PER EXIT

A.3 TOTAL EGRESS REQUIRED
= 64 OCC X 0.15 (WIDTH FACTOR 1005.3.2 EX.1)
= 9.6" TOTAL WIDTH / 2 (EXITS)
= 4.8" WIDTH REQUIRED PER EXIT

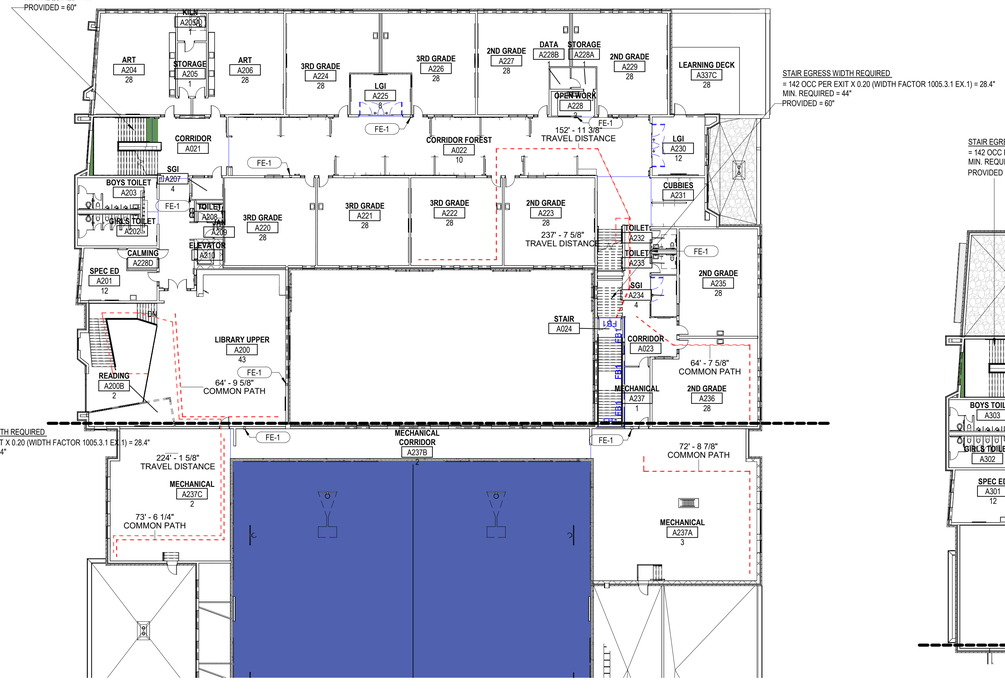
## 6C / G0.1 FIRST FLOOR - CODE PLAN
Scale: 1" = 20'-0"

**BUILDING A**
FLOOR OCC: 483
MAX BUILDING OCC: 2273

TOTAL EGRESS REQUIRED
= 483 (FLOOR OCC) X.0.15 (WIDTH FACTOR 1005.3.2 EX.1)
= 72.45" TOTAL WIDTH / 2 (EXITS)
= 36.225" WIDTH REQUIRED PER EXIT

A.1 TOTAL EGRESS REQUIRED
= 890 OCC X 0.15 (WIDTH FACTOR 1005.3.2 EX.1)
= 133.5" TOTAL WIDTH / 5 (EXITS)
= 26.7" WIDTH REQUIRED PER EXIT

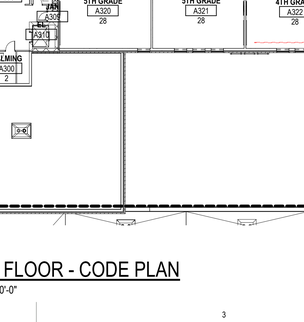
STAIR EGRESS WIDTH REQUIRED
= 142 OCC PER EXIT X 0.20 (WIDTH FACTOR 1005.3.1 EX.1) = 28.4"
MIN. REQUIRED = 44"
PROVIDED = 60"




STAIR EGRESS WIDTH REQUIRED
= 142 OCC PER EXIT X 0.20 (WIDTH FACTOR 1005.3.1 EX.1) = 28.4"
MIN. REQUIRED = 44"

## 6A / G0.1 SECOND FLOOR - CODE PLAN
Scale: 1" = 20'-0"

## 3C / G0.1 STAIR CODE SECTION
Scale: 3/32" = 1'-0"




**SHAFT AND ASSOCIATED FIRE RATING IS NOT REQUIRED PER IBC 1019.3 CONDITION 4.**

THE AREA OF THE VERTICAL OPENINGS BETWEEN STORIES DOES NOT EXCEED TWICE THE HORIZONTAL PROJECTED AREA OF THE STAIRWAY OR RAMP.

VERTICAL OPENING FIRST FLOOR:
9' 6.375" X 10' = **95.3 SF**

VERTICAL OPENING SECOND FLOOR:
19' .75" X 10' = **190.625 SF**

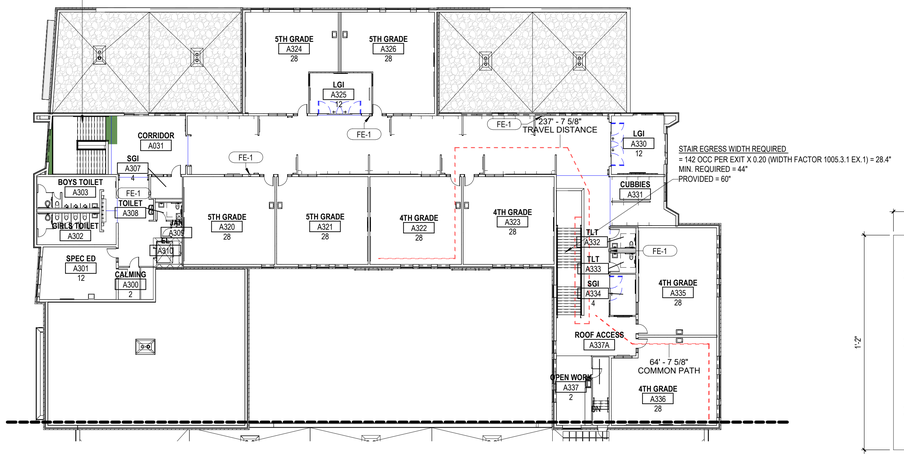
HORIZONTAL PROJECTED AREA:
19' .75" X 17' 11.625" = **342.6 SF**

190.625 SF OF VERTICAL OPENING
IS LESS THAN TWICE THE HORIZONTAL PROJECTED AREA.

STAIR EGRESS WIDTH REQUIRED
= 142 OCC PER EXIT X 0.20 (WIDTH FACTOR 1005.3.1 EX.1) = 28.4"
MIN. REQUIRED = 44"
PROVIDED = 60"

**BUILDING A**
FLOOR OCC: 283
MAX BUILDING OCC: 2273

TOTAL EGRESS REQUIRED
= 283 (FLOOR OCC) X 0.15 (WIDTH FACTOR 1005.3.2 EX.1)
= 42.45" TOTAL WIDTH / 2 (EXITS)
= 21.225" WIDTH REQUIRED PER EXIT




STAIR EGRESS WIDTH REQUIRED
= 142 OCC PER EXIT X 0.20 (WIDTH FACTOR 1005.3.1 EX.1) = 28.4"
MIN. REQUIRED = 44"
PROVIDED = 60"

## 4A / G0.1 THIRD FLOOR - CODE PLAN
Scale: 1" = 20'-0"

## FIRE RESISTIVE & CODE REVIEW LEGEND

**NOTE: THIS SHEET TO BE PRINTED IN COLOR**

WORK IN EXISTING BUILDING
BUILDING ADDITION

ROOM NAME / 101 / 45 — ROOM NAME, ROOM NUMBER, ROOM OCCUPANT LOAD
FE — FIRE EXTINGUISHER
27' R / 32' P — EGRESS DIRECTION, EGRESS WIDTH REQUIRED, EGRESS WIDTH PROVIDE

| | | | | | | |
|---|---|---|---|---|---|---|
| X | X | X | SMOKE TIGHT PARTITION TO DECK | SE1 | SE1 | SE1 | SHAFT ENCLOSURE 1 HOUR |
| CX | CX | CX | SMOKE TIGHT PARTITION TO INTERSTITIAL CEILING | SE2 | SE2 | SE2 | SHAFT ENCLOSURE 2 HOUR |
| PX | PX | PX | SMOKE TIGHT PARTITION TO PLENUM CEILING | SE3 | SE3 | SE3 | SHAFT ENCLOSURE 3 HOUR |
| | | | | SE4 | SE4 | SE4 | SHAFT ENCLOSURE 4 HOUR |
| SB1 | SB1 | SB1 | SMOKE BARRIER 1 HOUR | FB1 | FB1 | FB1 | FIRE BARRIER 1 HOUR |
| SB2 | SB2 | SB2 | SMOKE BARRIER 2 HOUR | FB2 | FB2 | FB2 | FIRE BARRIER 2 HOUR |
| SB3 | SB3 | SB3 | SMOKE BARRIER 3 HOUR | FB3 | FB3 | FB3 | FIRE BARRIER 3 HOUR |
| SB4 | SB4 | SB4 | SMOKE BARRIER 4 HOUR | FB4 | FB4 | FB4 | FIRE BARRIER 4 HOUR |
| FP0.5 | FP0.5 | FP0.5 | FIRE PARTITION 0.5 HOUR | FW1 | FW1 | FW1 | FIRE WALL 1 HOUR |
| FP1 | FP1 | FP1 | FIRE PARTITION 1 HOUR | FW2 | FW2 | FW2 | FIRE WALL 2 HOUR |
| | | | | FW3 | FW3 | FW3 | FIRE WALL 3 HOUR |
| 2HR | 2HR | 2HR | RATED CONSTRUCTION 2 HOUR | FW4 | FW4 | FW4 | FIRE WALL 4 HOUR |

## PROJECT DATA

| | |
|---|---|
| SITE ADDRESS: | 807 EAST AVE S<br>LA CROSSE, WI 54601 |
| COUNTY: | LA CROSSE |
| APPLICABLE CODES: | 2015 INTERNATIONAL BUILDING CODE<br>WISCONSIN COMMERCIAL BUILDING CODE (SPS 361-366) |

**BUILDING #B**
NEW CONSTRUCTION
PRIMARY OCCUPANCY = E
CONSTRUCTION TYPE = IIB
NUMBER OF STORIES = ONE STORY
FULLY SPRINKLERED BUILDING

BUILDING AREA:
MAXIMUM BUILDING HEIGHT PER TABLE 504.3 = 75' W/ SPRINKLER SYSTEM
MAXIMUM NUMBER OF STORIES PER TABLE 504.4 = 3 STORIES W/ SPRINKLER SYSTEM
ALLOWABLE AREA PER TABLE 506.2 = 43,500 SF MULTISTORY & SPRINKLERED
FRONTAGE INCREASE:
TOTAL PERIMETER = 90 LF
BUILDING PERIMETER ON PUBLIC WAY (30' OPEN SPACE) = 72 LF
CALCULATED FRONTAGE AREA INCREASE (80%-25%) = 7,975 SF
TOTAL ALLOWABLE BUILDING AREA BY FLOOR = 51,475 SF
ACTUAL BUILDING AREA
1ST FLOOR = 2,392 SF
TOTAL = 2,392 SF

OCCUPANT LOAD BY TYPE:
'E' OCCUPANCY = 16 OCCUPANTS

EGRESS INFO:
1005.1 - MEANS OF EGRESS SIZING
EGRESS DOOR WIDTH REQUIRED (IBC 1005.3.2 EX #1) = 16 X .15 = 2.4"
EGRESS DOOR WIDTH PROVIDED = 96"
1006.3.1 - MIN. OF (4) EXITS >1000 OCC.
1014.3 - COMMON PATH OF TRAVEL = 75' MAX
1017.2 - EXIT ACCESS TRAVEL DISTANCE = 250' MAX W/ SPRINKLER
1020.1 - CORRIDOR RATING = 0 HR WITH SPRINKLER
1020.2 - CORRIDOR WIDTH = 72" MIN SERVING >100 OCC.
1020.4 EXi#2 - DEAD END CORRIDOR = 50' MAX WITH SPRINKLER

**BUILDING #A**
NEW CONSTRUCTION
PRIMARY OCCUPANCY = E
NON-SEPARATED OCCUPANCY = A-2 (CAFETERIA), A-3 (GYMNASIUM) PER IBC 303.1.3
CONSTRUCTION TYPE = IIB
NUMBER OF STORIES = THREE STORIES
FULLY SPRINKLERED BUILDING

BUILDING AREA:
MAXIMUM BUILDING HEIGHT PER TABLE 504.3 = 75' W/ SPRINKLER SYSTEM
MAXIMUM NUMBER OF STORIES PER TABLE 504.4 = 3 STORIES W/ SPRINKLER SYSTEM
ALLOWABLE AREA PER TABLE 506.2 = 43,500 SF MULTISTORY & SPRINKLERED
FRONTAGE INCREASE:
TOTAL PERIMETER = 933 LF
BUILDING PERIMETER ON PUBLIC WAY (30' OPEN SPACE) = 916 LF
CALCULATED FRONTAGE AREA INCREASE (98.2%-25%) = 10,614 SF
TOTAL ALLOWABLE BUILDING AREA BY FLOOR = 54,114 SF
ACTUAL BUILDING AREA
1ST FLOOR = 52,850 SF
2ND FLOOR = 26,479 SF
3RD FLOOR = 14,717 SF
TOTAL = 94,046 SF

OCCUPANT LOAD BY TYPE:
'E' OCCUPANCY = 1,922 OCCUPANTS
'A' OCCUPANCY = 865 OCCUPANTS

EGRESS INFO:
1005.1 - MEANS OF EGRESS SIZING
EGRESS DOOR WIDTH REQUIRED (IBC 1005.3.2 EX #1) = 1,149 X .15 = 172.4"
EGRESS DOOR WIDTH PROVIDED = 456"
1006.3.1 - MIN. OF (4) EXITS >1000 OCC.
1014.3 - COMMON PATH OF TRAVEL = 75' MAX
1017.2 - EXIT ACCESS TRAVEL DISTANCE = 250' MAX W/ SPRINKLER
1020.1 - CORRIDOR RATING = 0 HR WITH SPRINKLER
1020.2 - CORRIDOR WIDTH = 72" MIN SERVING >100 OCC.
1020.4 EXi#2 - DEAD END CORRIDOR = 50' MAX WITH SPRINKLER

**SANITARY FIXTURE CALCULATIONS**

SCENARIO #1: RESTROOMS FOR TYPICAL SCHOOL DAY BASED ON MAXIMUM EXPECTED STUDENT ENROLLMENT & CURRENT STAFF COUNTS* ('E' OCCUPANCY USE)
TOTAL 'E' OCCUPANTS: 1,938 (969 MEN & 969 WOMEN)

| | WATER CLOSETS | | LAVATORIES | | UNISEX RESTROOMS | DRINK FOUNTAINS | SERVICE SINKS |
|---|---|---|---|---|---|---|---|
| | M (1:50) | W (1:50) | M (1:50) | W (1:50) | | 1:100 | |
| FORMULA | | | | | | | |
| REQUIRED | 20 | 20 | 20 | 20 | 1 | 20 | 1 |
| PROVIDED | 19 WC / 2 U | 19 WC / 2 U | 21 | 21 | 9** | 10/30* | 4 |

*DRINKING FOUNATINS IN EACH CLASSROOM
**RESTROOMS IN K/4K CLASSROOMS

SCENARIO #2: RESTROOMS FOR A GYMNASIUM AND CAFETERIA ('A-3' OCCUPANCY USE)
TOTAL 'A-3' OCCUPANTS: 860 (430 MEN & 430 WOMEN)

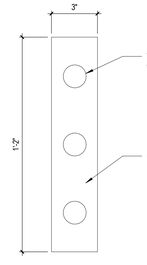
| | WATER CLOSETS | | LAVATORIES | | UNISEX RESTROOMS | DRINK FOUNTAINS | SERVICE SINKS |
|---|---|---|---|---|---|---|---|
| | M (1:125) | W (1:65) | M (1:200) | W (1:200) | | 1:500 | |
| FORMULA | | | | | | | |
| REQUIRED | 4 | 7 | 3 | 3 | | 2 | |
| PROVIDED | 5 | 5 WC /2 U | 3 | 3 | | 4 | |

SCENARIO #1 (FULL SCHOOL) AND SCENARIO #2 (GYMNASIUM EVENT) ARE NOT EXPECTED TO TAKE PLACE AT THE SAME TIME. BOTH SCENARIO #1 AND SCENARIO #2 CAN BE MEET BY PROVIDED FIXTURES.

** UNISEX TOILETS CAN BE USED TOWARD MENS OR WOMENS REQUIRED FIXTURE TOTAL (MEN'S AND WOMEN'S TOTAL FIXTURES SHOWN ABOVE DO NOT INCLUDE ANY UNISEX FIXTURES)

*** (23) WALL MOUNTED DRINK FOUNTAINS AND (8) DRINK FOUNTAINS PROVIDED AT CLASSROOM SINKS

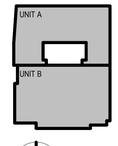
**** SPS 362.2902 - 66% OF WATER CLOSETS (WC) CAN BE URINALS (U) IN BOTH 'E' AND 'A' OCCUPANCIES




THREE (3) 1 1/2" DIAMETER REFLECTIVE CIRCLES, WHITE IN COLOR, SPACED EQUALLY VERTICALLY AND HORIZONTALLY

1/4" THICK METAL PLATE, BLUE IN COLOR.

## 2A / G0.1 EXTERIOR FIRE WALL SIGN
Scale: 3" = 1'-0"




KEY PLAN

BRAY ARCHITECTS
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829 South 1st Street
Milwaukee, Wisconsin 53204
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Sheboygan • Davenport
Moline • Saint Paul
www.brayarch.com

Project Title:
New Building for
New Elementary School
La Crosse School District
807 East Ave S
La Crosse, WI 54601

REVISIONS:

| # | DATE | DESCRIPTION |
|---|---|---|

NOT FOR CONSTRUCTION

Project Number: 3752

Issued For: 50% CD REVIEW
08/19/2025

Sheet Title: CODE REVIEW

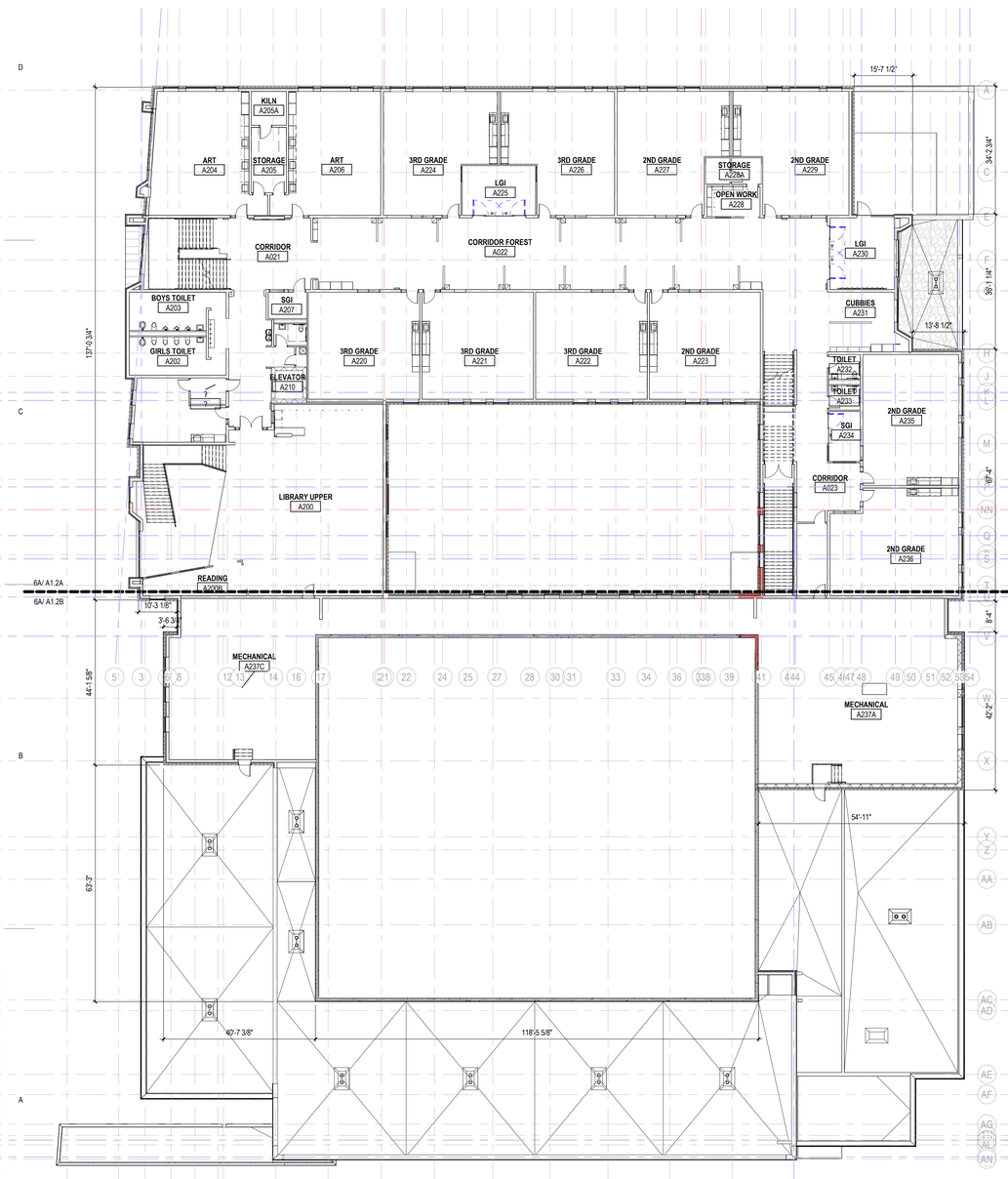
Sheet Number: G0.1

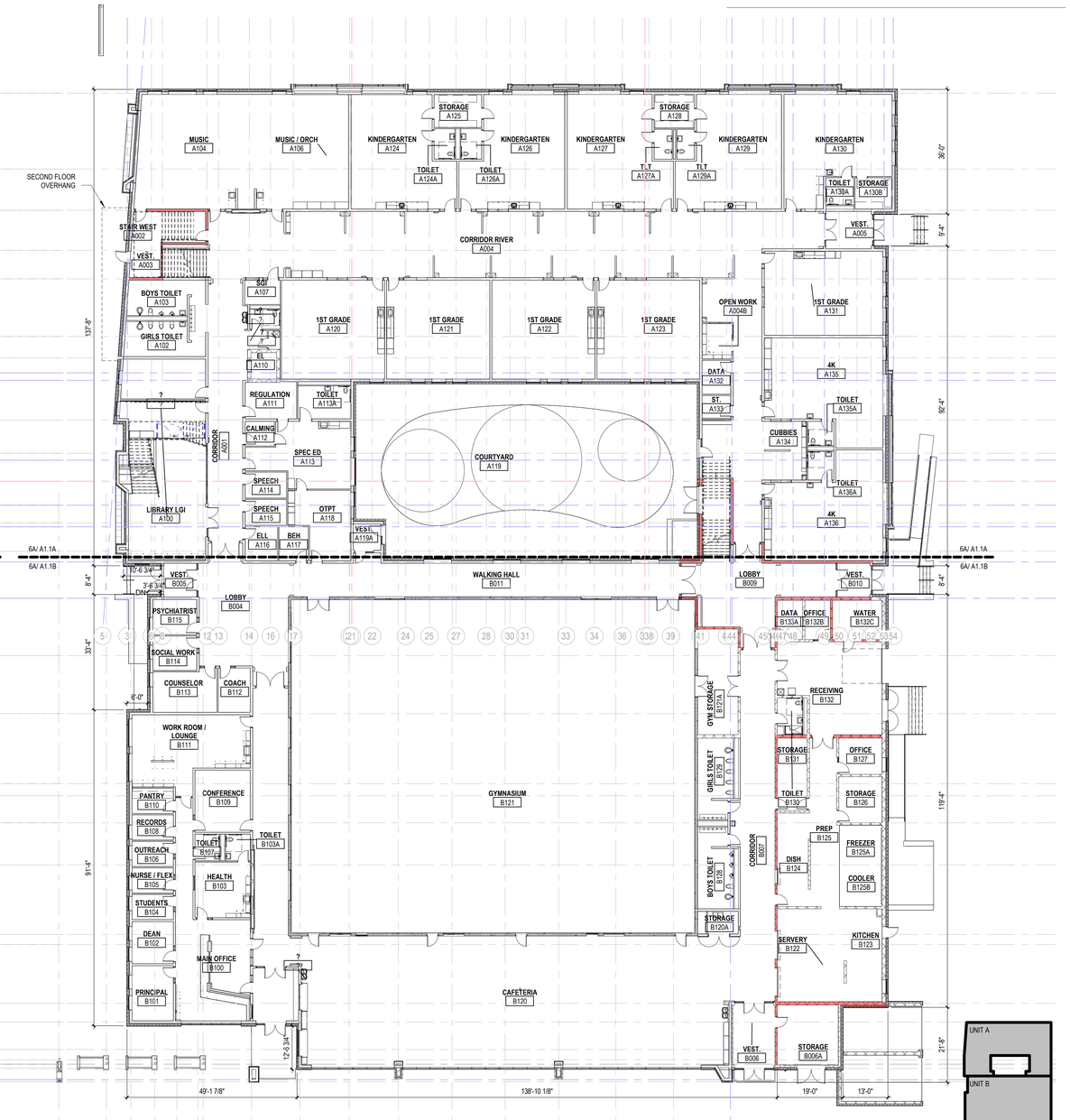
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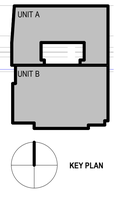
Autodesk Docs://3752 - LaCrosse School 2024 Referendum/3752_LCSD_New Elementary School.rvt
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# GENERAL NOTES

1. DO NOT SCALE FROM DRAWINGS.
2. REFER TO SHEET AG1.1 & AG1.2 FOR WALL TYPES. REFER TO SHEET G0.1 FOR RATED WALL LOCATIONS.
3. REFER TO SHEET AG1.0 & A6.0 FOR TYPICAL MOUNTING HEIGHTS. REFER TO INTERIOR ELEVATION SHEETS FOR ROOM SPECIFIC FIXTURE MOUNTING HEIGHTS.
4. REFER TO SHEET A9.0B FOR FLOOR TRANSITION DETAILS.
5. REFER TO SHEET A9.0B FOR TYPICAL FLOOR/BASE CONDITIONS.
6. DIMENSIONS FOR INTERIOR STUD WALLS ARE TO FACE OF WALL. EXTERIOR STUD WALLS ARE DIMENSIONED TO INTERIOR FACE OF STUDS. CMU WALLS ARE DIMENSIONED TO FACE OF WALL.
7. IDENTIFY ALL RATED WALLS AND PARTITIONS WITH PAINTED LETTERING THAT READS "FIRE AND/OR SMOKE BARRIER - PROTECT ALL OPENINGS" ABOVE CEILINGS. LETTERING TO BE LOCATED WITHIN 15 FEET OF THE END OF EACH WALL AND AT INTERVALS NOT EXCEEDING 30'-0" O.C. HORIZONTALLY, BOTH SIDES OF WALL. LETTERING TO BE NOT LESS THAN 3" IN HEIGHT WITH A 3/8" MINIMUM STROKE IN A CONTRASTING COLOR. SEE SHEET G0.1 PLANS FOR LOCATION OF RATED WALLS.
8. REFER TO STRUCTURAL DRAWINGS FOR FIREPROOFING LOCATIONS. PROVIDE INTUMESCENT PAINT AT EXPOSED STEEL COLUMNS AND AT THE BOTTOM FLANGE OF STEEL LINTELS. PROVIDE SPRAYED FIRE-RESISTIVE MATERIAL AT STEEL BEAMS, ROOF DECK AND ROOF JOISTS UNLESS NOTED OTHERWISE.
9. REFER TO REFLECTED CEILING PLANS & CLERESTORY PLANS FOR LOCATION OF ALL WINDOW TREATMENTS.




6A / A1.00 SECOND FLOOR - OVERALL PLAN
Scale: 1/16" = 1'-0"

3A / A1.00 FIRST FLOOR - OVERALL PLAN
Scale: 1/16" = 1'-0"

Autodesk Docs://3752 - LaCrosse School 2004 Referendum/3752_LCSD_New Elementary School.rvt
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Project Title:
New Building for
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La Crosse School District
807 East Ave S
La Crosse, WI 54601

REVISIONS:

| # | DATE | DESCRIPTION |
|---|---|---|

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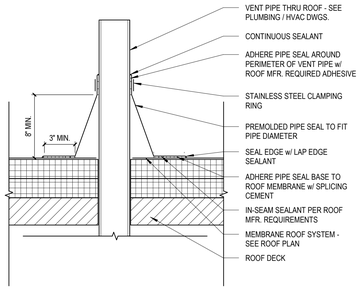
Project Number: 3752

Issued For: 50% CD REVIEW
08/19/2025

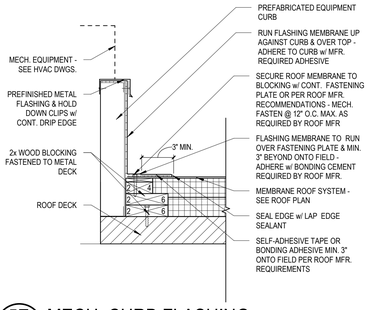
Sheet Title: OVERALL FLOOR PLANS

Sheet Number: A1.00

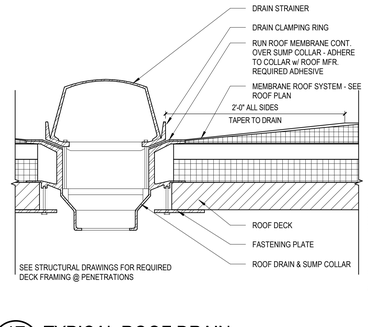
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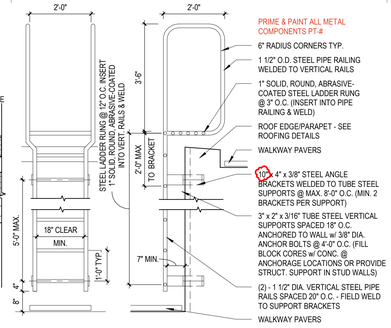

6E A1.01 VENT PIPE FLASHING
Scale: 1 1/2" = 1'-0"




5E A1.01 MECH. CURB FLASHING
Scale: 1 1/2" = 1'-0"

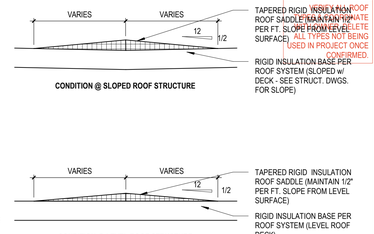



4E A1.01 TYPICAL ROOF DRAIN
Scale: 1 1/2" = 1'-0"

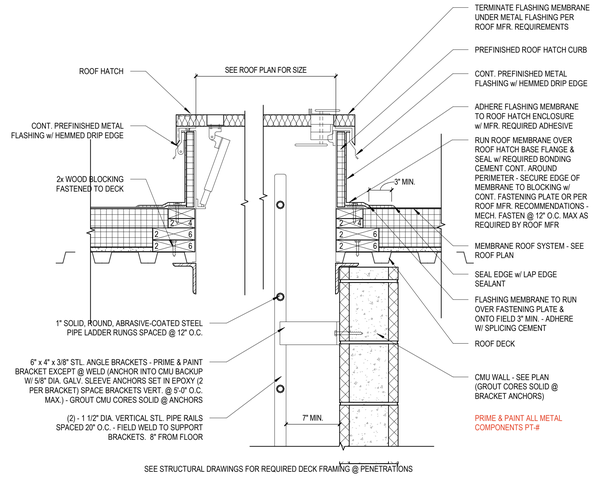



3E A1.01 ROOF LADDER
Scale: 1/2" = 1'-0"

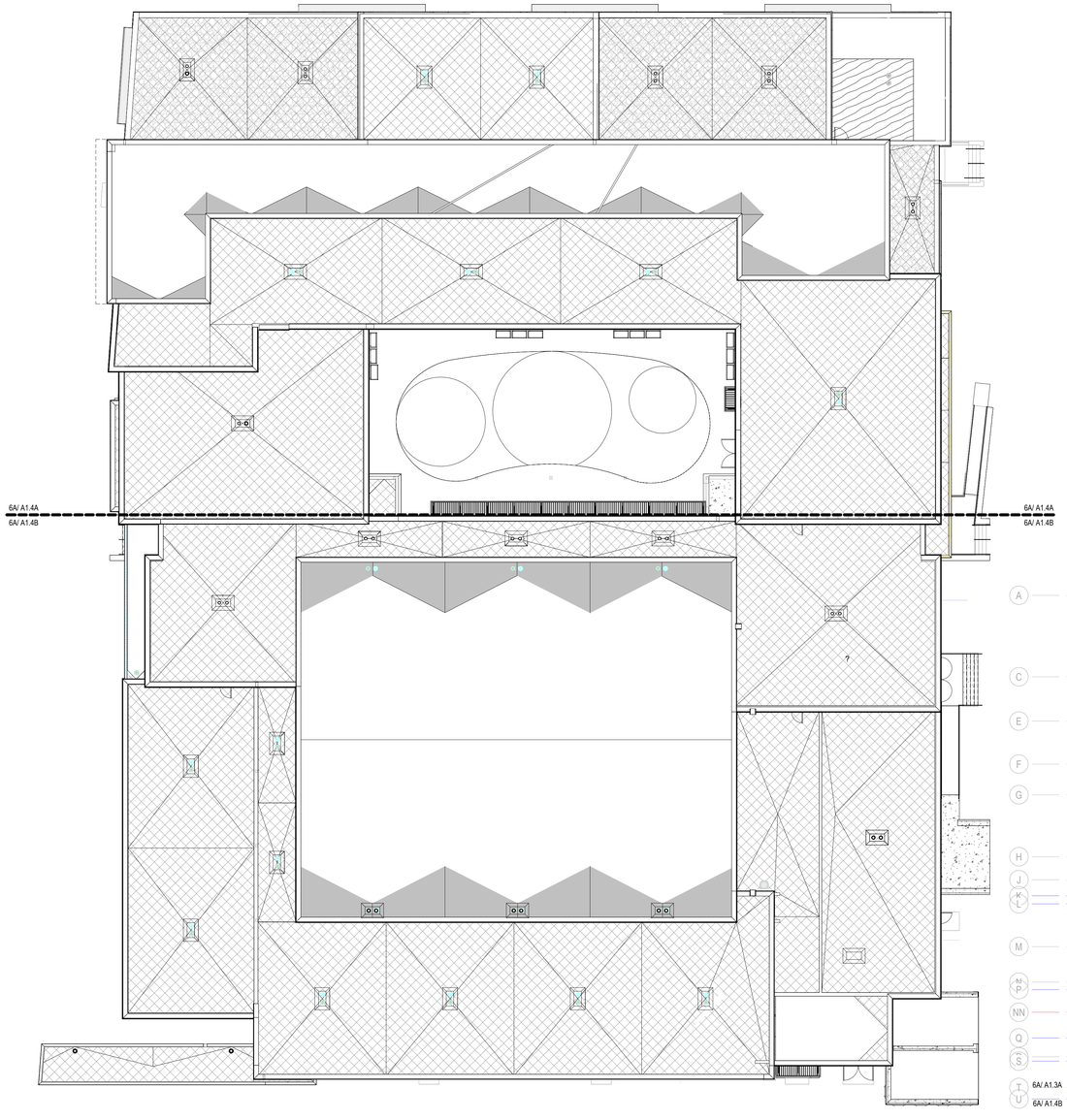
USER NOTE:
Verify bracket depth with wall thickness



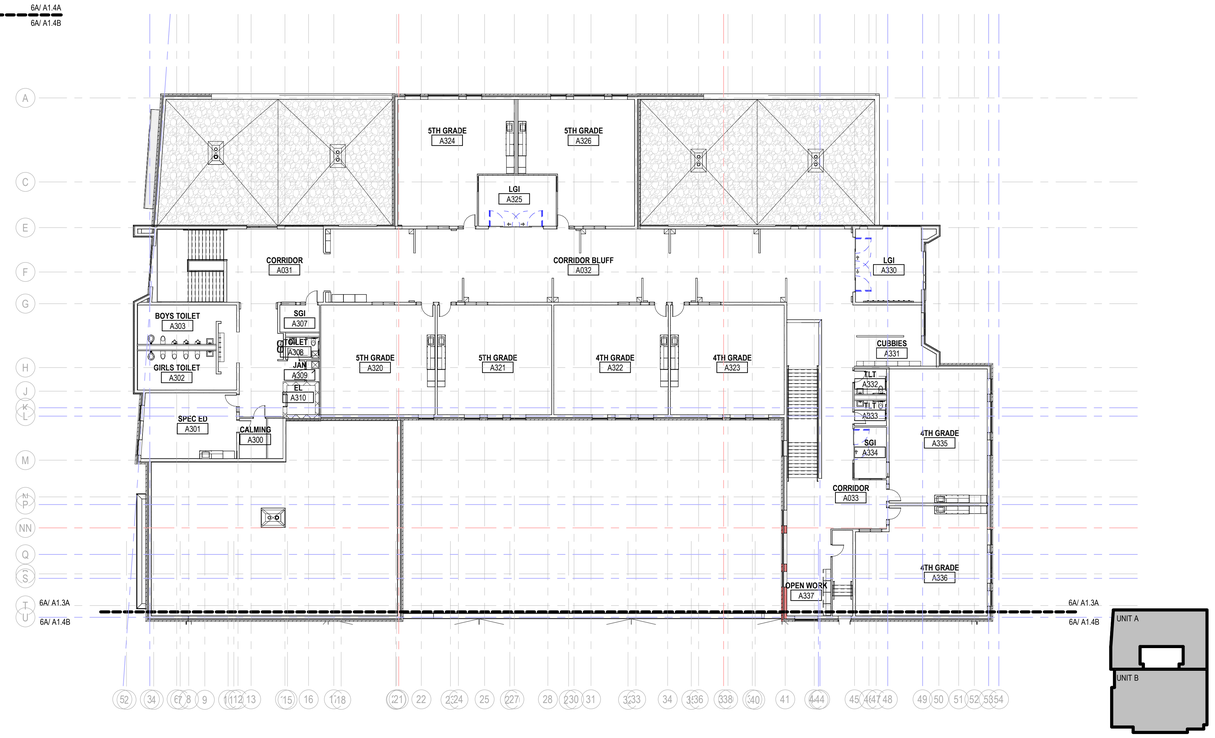

2E A1.01 TYP. ROOF SADDLE
Scale: 3/4" = 1'-0"



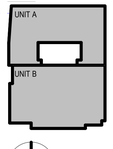

3C A1.01 TYPICAL ROOF HATCH
Scale: 1 1/2" = 1'-0"




6A A1.01 ROOF PLAN - OVERALL PLAN
Scale: 1/16" = 1'-0"




3A A1.01 THIRD FLOOR - OVERALL PLAN
Scale: 1/16" = 1'-0"




KEY PLAN

## ROOF PLAN LEGEND & NOTES

ROOF TYPE 1 (RT-1):

- EPDM SHEET
- COVERBOARD
- 5" INSULATION DEPTH (MINIMUM) WITH A MINIMUM OF 2 LAYERS; STAGGER JOINTS; TAPERED INSULATION AS SHOWN ON PLAN
- THERMAL BARRIER AT SPRAY APPLIED FIRE PROOFING LOCATIONS ONLY - SEE STRUCTURAL
- SLOPED STRUCTURE WITH METAL ROOF DECK, SEE STRUCTURAL FOR SIZE & TYPE

ROOF TYPE 2 (RT-2):

- EPDM SHEET
- COVERBOARD
- TAPERED INSULATION, 1/4" PER FOOT MINIMUM SLOPE, 5" DEPTH (MINIMUM) WITH A MINIMUM OF 2 LAYERS; STAGGER JOINTS
- THERMAL BARRIER AT SPRAY APPLIED FIRE PROOFING LOCATIONS ONLY - SEE STRUCTURAL
- FLAT STRUCTURE WITH METAL ROOF DECK, SEE STRUCTURAL FOR SIZE & TYPE

ROOF TYPE 3 (RT-3):

- EPDM SHEET
- COVERBOARD
- 5" INSULATION DEPTH (MINIMUM) WITH A MINIMUM OF 2 LAYERS; STAGGER JOINTS; TAPERED INSULATION AS SHOWN ON PLAN
- THERMAL BARRIER
- SLOPED STRUCTURE WITH ACOUSTICAL METAL ROOF DECK, SEE STRUCTURAL FOR SIZE & TYPE

ROOF TYPE 4 (RT-4):

- EPDM SHEET
- COVERBOARD
- TAPERED INSULATION, 1/4" PER FOOT MINIMUM SLOPE, 5" DEPTH (MINIMUM) WITH A MINIMUM OF 2 LAYERS; STAGGER JOINTS
- THERMAL BARRIER
- FLAT STRUCTURE WITH ACOUSTICAL METAL ROOF DECK, SEE STRUCTURAL FOR SIZE & TYPE

ROOF TYPE 5 (RT-5):

- BALLASTED ROOF
- COVERBOARD
- TAPERED INSULATION, 1/4" PER FOOT MINIMUM SLOPE, 5" DEPTH (MINIMUM) WITH A MINIMUM OF 2 LAYERS; STAGGER JOINTS
- THERMAL BARRIER
- FLAT STRUCTURE WITH ACOUSTICAL METAL ROOF DECK, SEE STRUCTURAL FOR SIZE & TYPE

+8" DENOTES APPROXIMATE THICKNESS OF TAPERED INSULATION ABOVE THE 5" MIN. AT DRAINS, SHOWN FOR REFERENCE ONLY. MAINTAIN CONTINUOUS MINIMUM SLOPE TOWARDS DRAIN AS DESCRIBED IN ROOF TYPES

DENOTES WALKWAY PAVERS

DENOTES ROOF CRICKETS; MAINTAIN 1/2" PER FOOT SLOPE TO DRAINS

DENOTES TAPERED INSULATION; MAINTAIN 1/4" PER FOOT SLOPE TO DRAINS

DENOTES GRAVEL BALLAST

1. ALL ROOF SLOPES TO BE MINIMUM 1/4" PER FOOT UNLESS NOTED OTHERWISE.
2. ALL ROOF TOP CURBS TO BE SADDLED WITH MINIMUM 1/2" PER FOOT PITCH.
3. NOT ALL ROOF PENETRATIONS ARE SHOWN. VERIFY PENETRATIONS WITH ALL BUILDING TRADES.
4. SEE HVAC DRAWINGS FOR SIZES, QUANTITIES, LOCATIONS AND TYPES OF HVAC PENETRATIONS AND ROOF TOP MOUNTED EQUIPMENT.
5. SEE DETAILS E/ A1.01FOR VENT PIPE FLASHING DETAIL. VERIFY ALL LOCATIONS AND QUANTITIES W/ PLUMBING CONTRACTOR.
6. SEE DETAIL 5E/ A1.01 FOR CURB FLASHING DETAIL.
7. SEE DETAIL 4E/ A1.01 FOR TYPICAL ROOF DRAIN DETAIL.
8. SEE DETAIL 3E/ A1.01 FOR ROOF LADDER.
9. SEE DETAIL 2E/ A1.01 FOR TYPICAL ROOF SADDLES.
10. SEE DETAIL 11/ A5.6 FOR TYPICAL THROUGH-WALL SCUPPER DETAIL.

WALKWAY PAVERS ROOF PLAN TO INDICATE LOCATE ALL TYPES NOT BEING USED IN PROJECT ONCE CONFIRMED.

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New Elementary School
La Crosse School District
807 East Ave S
La Crosse, WI 54601

REVISIONS:

| # | DATE | DESCRIPTION |
|---|---|---|
| 1 | Date 1 | Revision 1 |

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Project Number: 3752

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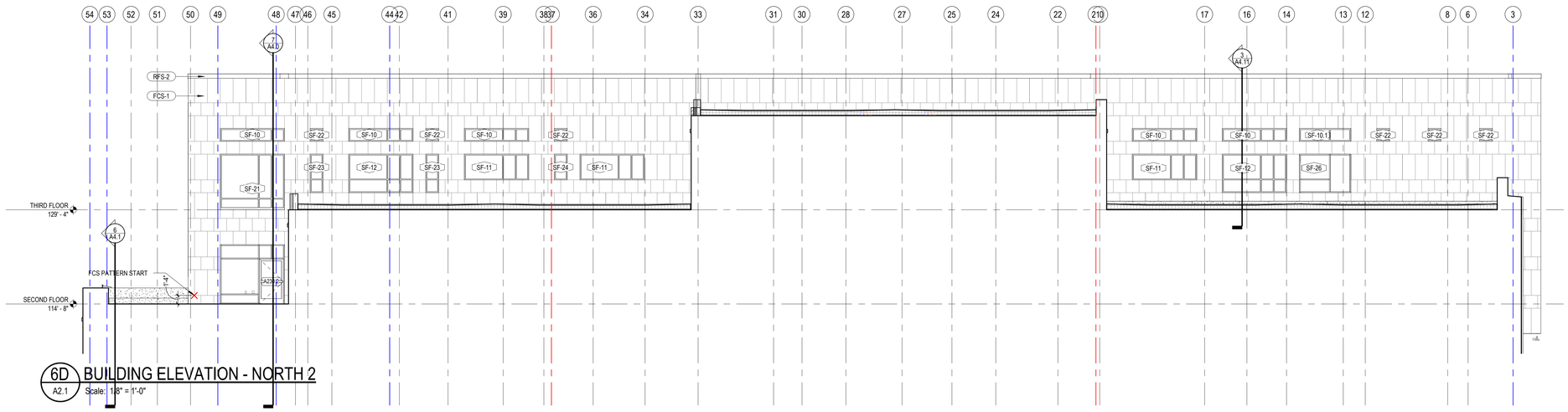
Sheet Title: OVERALL FLOOR PLANS

Sheet Number: A1.01

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## 6D BUILDING ELEVATION - NORTH 2

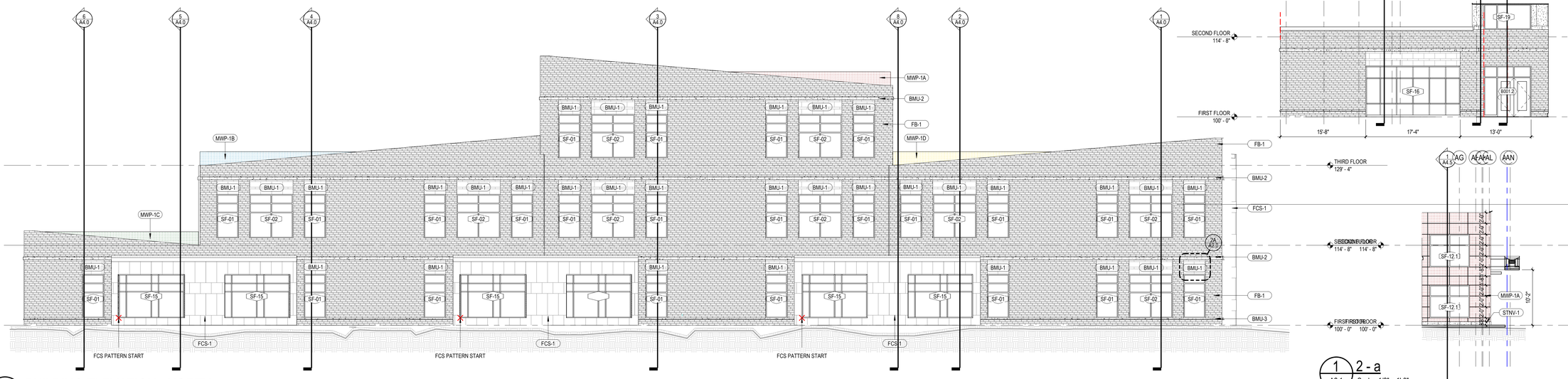
A2.1 Scale: 1/8" = 1'-0"

## EXTERIOR MATERIAL LEGEND

- MWP-1A - METAL WALL PANEL, WNX RED
- MWP-1B - METAL WALL PANEL, AMOX BLUE
- MWP-1C - METAL WALL PANEL, BGN GREEN
- MWP-1D - METAL WALL PANEL, EYL YELLOW
- MWP-2 - METAL WALL PANEL, RAL 7047
- FB-1 - FACE BRICK
- BMU-1 - BURNISHED MASONRY UNITS
- FCS-1 - FIBER CEMENT REVEAL PANEL SIDING, PATINA SEE 2B/ A2.0 FOR PATTERN
- STNV-1 - ANCHORED STONE VENEER

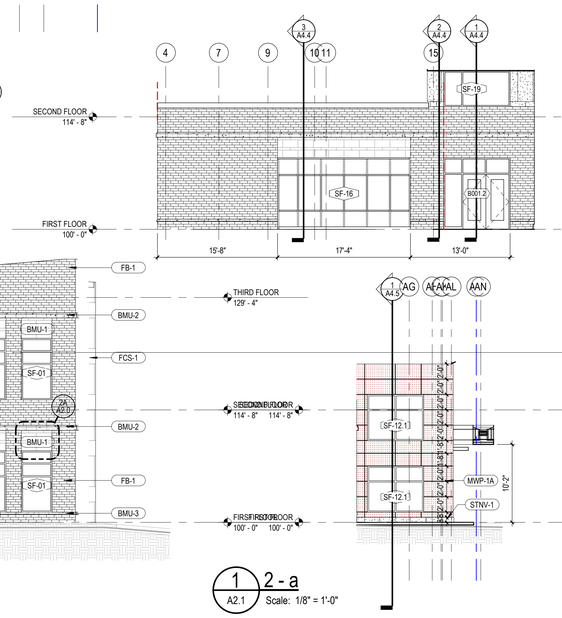
## KEYNOTE LEGEND

| | |
|---|---|
| BMU-1 | BURNISHED MASONRY UNITS |
| BMU-2 | BURNISHED MASONRY UNITS, 6" TO 4" ACCENT TRIM |
| BMU-3 | BURNISHED MASONRY UNITS, 4" BULLNOSE ACCENT TRIM |
| DS-1 | |
| FB-1 | FACE BRICK |
| FCS-1 | FIBER CEMENT REVEAL PANEL SIDING, PATINA |
| MWP-1A | METAL WALL PANEL, WNX RED |
| MWP-1B | METAL WALL PANEL, AMOX BLUE |
| MWP-1C | METAL WALL PANEL, BGN GREEN |
| MWP-1D | METAL WALL PANEL, EYL YELLOW |
| MWP-2 | METAL WALL PANEL, RAL 7047 |
| RFS-1 | FLASHING |
| RFS-2 | ROOF-EDGE FASCIA, COLOR TO MATCH MWP-2 |
| RFS-2B | ROOF-EDGE FASCIA, COLOR TO MATCH MWP-1B |
| STNV-1 | ANCHORED STONE VENEER |



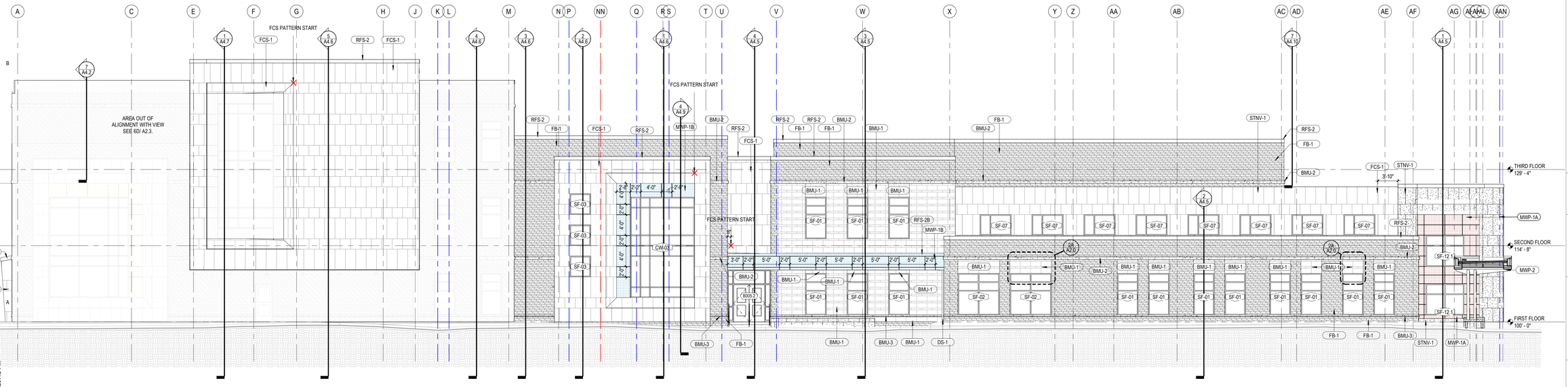

## 6C BUILDING ELEVATION - NORTH

A2.1 Scale: 1/8" = 1'-0"




## 1 2 - a

A2.1 Scale: 1/8" = 1'-0"




## 6A BUILDING ELEVATION - WEST

A2.1 Scale: 1/8" = 1'-0"

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Sheet Title:
EXTERIOR ELEVATIONS

Sheet Number:
A2.1

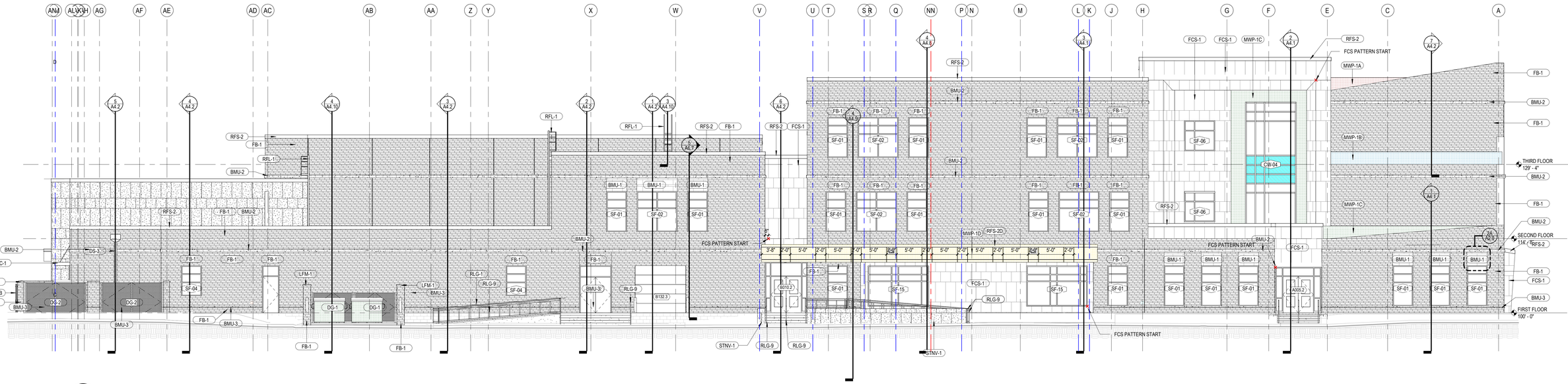
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## KEYNOTE LEGEND

| | |
|---|---|
| ASC-1 | ALUMINUM SUNSHADE CANOPY |
| BMU-1 | BURNISHED MASONRY UNITS |
| BMU-2 | BURNISHED MASONRY UNITS, 6" TO 4" ACCENT TRIM |
| BMU-3 | BURNISHED MASONRY UNITS, 4" BULLNOSE ACCENT TRIM |
| DG-1 | DUMPSTER ENCLOSURE GATE 72" |
| DG-2 | DUMPSTER ENCLOSURE GATE 96" |
| DS-2 | |
| DS-3 | |
| FB-1 | FACE BRICK |
| FCS-1 | FIBER CEMENT REVEAL PANEL SIDING, PATINA |
| HS-1 | |
| LFM-1 | |
| MTS-1B | |
| MWP-1A | METAL WALL PANEL, WNX RED |
| MWP-1B | METAL WALL PANEL, AMOX BLUE |
| MWP-1C | METAL WALL PANEL, BGN GREEN |
| MWP-1D | METAL WALL PANEL, EYL YELLOW |
| MWP-2 | METAL WALL PANEL, RAL 7047 |
| RFL-1 | STEEL LADDER (ROOF) |
| RFS-2 | ROOF-EDGE FASCIA, COLOR TO MATCH MWP-2 |
| RFS-2D | ROOF-EDGE FASCIA, COLOR TO MATCH MWP-1D |
| RLG-1 | PIPE AND TUBE METAL HANDRAIL |
| RLG-9 | |
| STNV-1 | ANCHORED STONE VENEER |

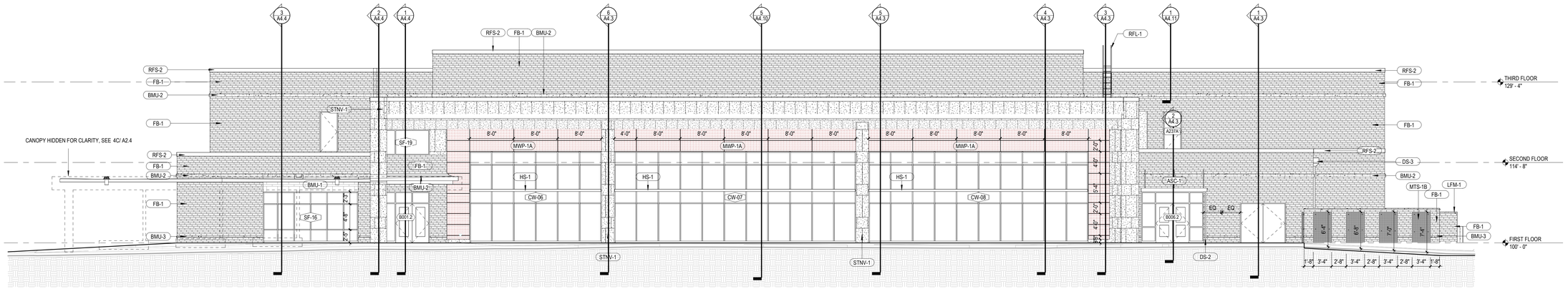
## EXTERIOR MATERIAL LEGEND

- MWP-1A - METAL WALL PANEL, WNX RED
- MWP-1B - METAL WALL PANEL, AMOX BLUE
- MWP-1C - METAL WALL PANEL, BGN GREEN
- MWP-1D - METAL WALL PANEL, EYL YELLOW
- MWP-2 - METAL WALL PANEL, RAL 7047
- FB-1 - FACE BRICK
- BMU-1 - BURNISHED MASONRY UNITS
- FCS-1 - FIBER CEMENT REVEAL PANEL SIDING, PATINA SEE 2B/ A2.0 FOR PATTERN
- STNV-1 - ANCHORED STONE VENEER




## 6C BUILDING ELEVATION - EAST

A2.2 Scale: 1/8" = 1'-0"




## 6A BUILDING ELEVATION - SOUTH

A2.2 Scale: 1/8" = 1'-0"


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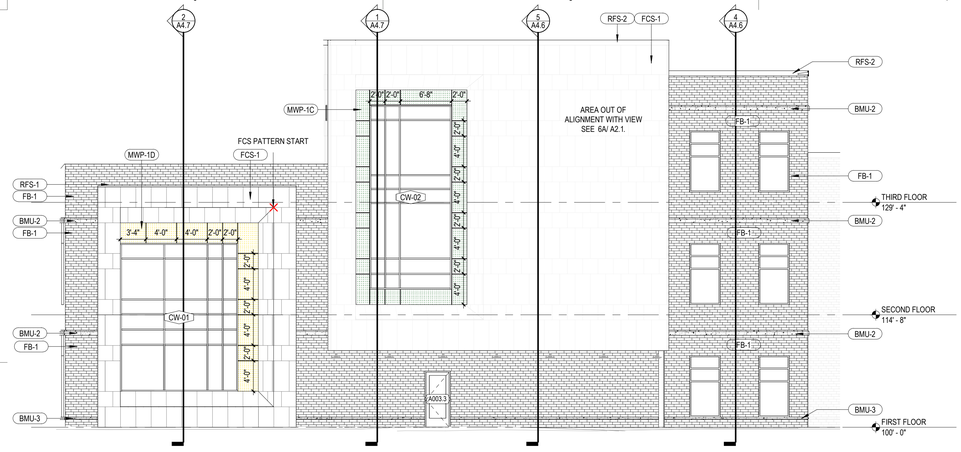
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Sheet Title: **EXTERIOR ELEVATIONS**

Sheet Number: **A2.2**

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## 6D BUILDING ELEVATION - 4 DEGREES NORTH OF WEST

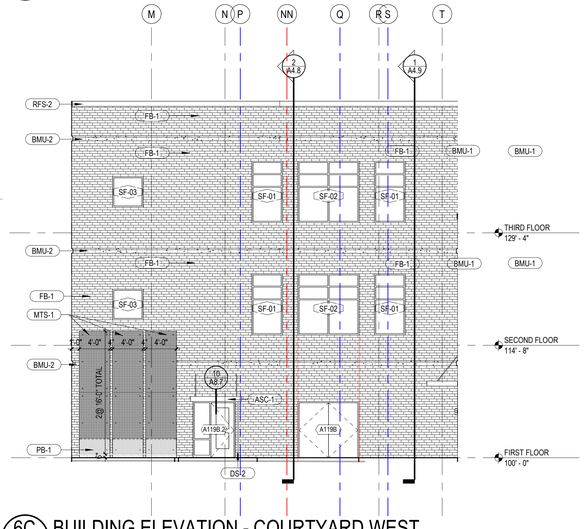
A2.3 Scale: 1/8" = 1'-0"

## EXTERIOR MATERIAL LEGEND

- MWP-1A - METAL WALL PANEL, WNX RED
- MWP-1B - METAL WALL PANEL, AMOX BLUE
- MWP-1C - METAL WALL PANEL, BGN GREEN
- MWP-1D - METAL WALL PANEL, EYL YELLOW
- MWP-2 - METAL WALL PANEL, RAL 7047
- FB-1 - FACE BRICK
- BMU-1 - BURNISHED MASONRY UNITS
- FCS-1 - FIBER CEMENT REVEAL PANEL SIDING, PATINA SEE 2B/ A2.0 FOR PATTERN
- STNV-1 - ANCHORED STONE VENEER

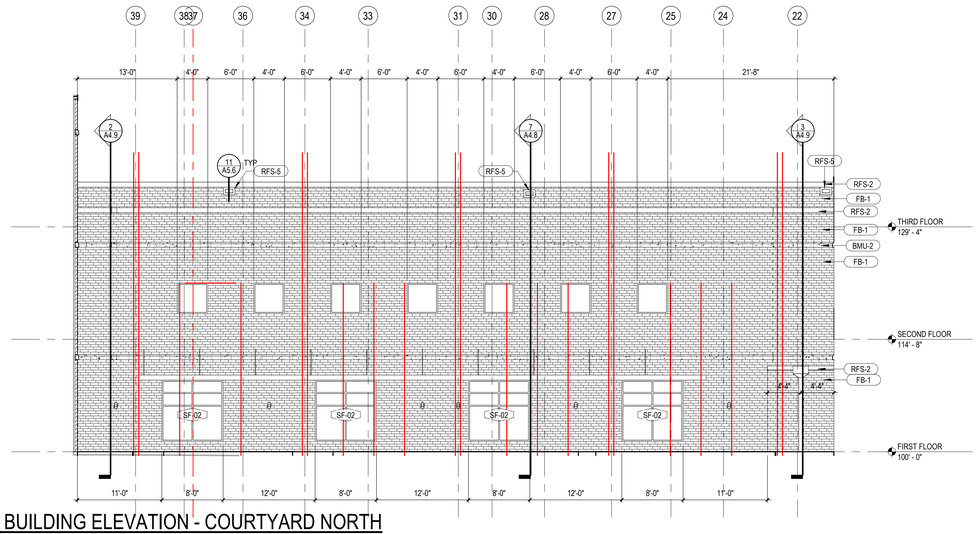
## KEYNOTE LEGEND

| | |
|---|---|
| ASC-1 | ALUMINUM SUNSHADE CANOPY |
| BMU-1 | BURNISHED MASONRY UNITS |
| BMU-2 | BURNISHED MASONRY UNITS, 6" TO 4" ACCENT TRIM |
| BMU-3 | BURNISHED MASONRY UNITS, 4" BULLNOSE ACCENT TRIM |
| DS-2 | |
| FB-1 | FACE BRICK |
| FCS-1 | FIBER CEMENT REVEAL PANEL SIDING, PATINA |
| MTS-1 | |
| MWP-1A | METAL WALL PANEL, WNX RED |
| MWP-1B | METAL WALL PANEL, AMOX BLUE |
| MWP-1C | METAL WALL PANEL, BGN GREEN |
| MWP-1D | METAL WALL PANEL, EYL YELLOW |
| MWP-2 | METAL WALL PANEL, RAL 7047 |
| PB-1 | |
| RFS-1 | FLASHING |
| RFS-2 | ROOF-EDGE FASCIA, COLOR TO MATCH MWP-2 |
| RFS-5 | THRU WALL SCUPPER UNIT |
| STNV-1 | ANCHORED STONE VENEER |



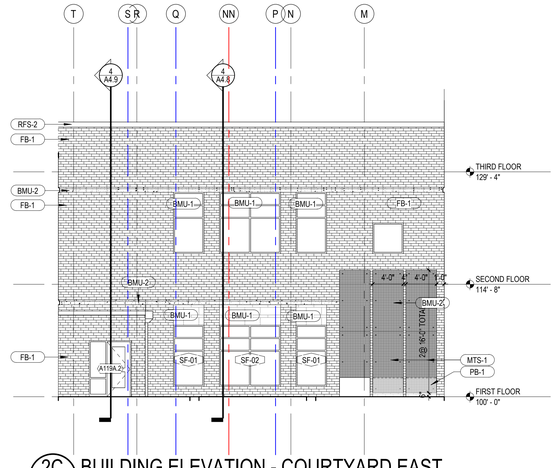

## 6C BUILDING ELEVATION - COURTYARD WEST

A2.3 Scale: 1/8" = 1'-0"



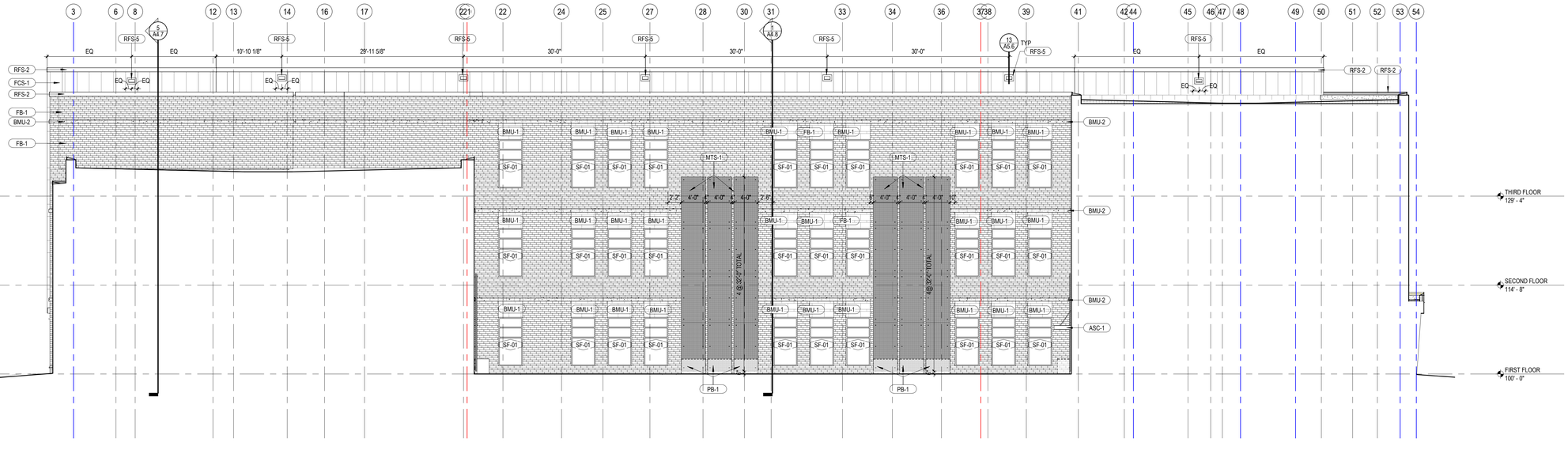

## 4C BUILDING ELEVATION - COURTYARD NORTH

A2.3 Scale: 1/8" = 1'-0"




## 2C BUILDING ELEVATION - COURTYARD EAST

A2.3 Scale: 1/8" = 1'-0"




## 6A BUILDING ELEVATION - COURTYARD SOUTH

A2.3 Scale: 1/8" = 1'-0"

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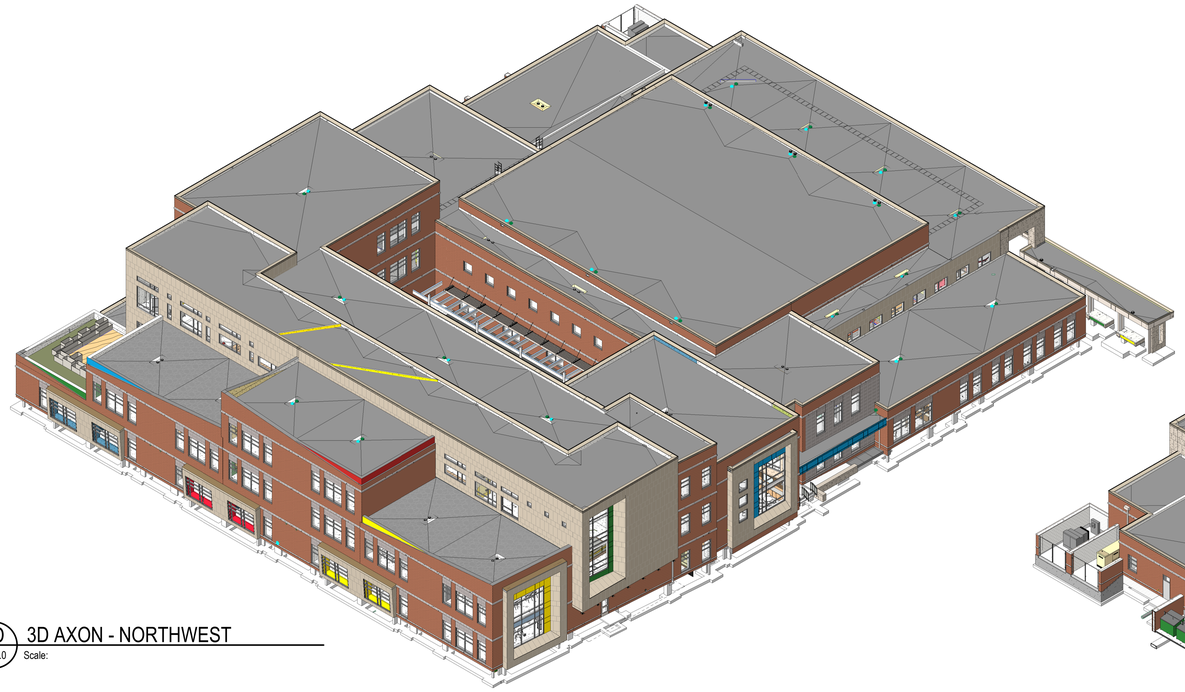
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Sheet Title: **EXTERIOR ELEVATIONS**

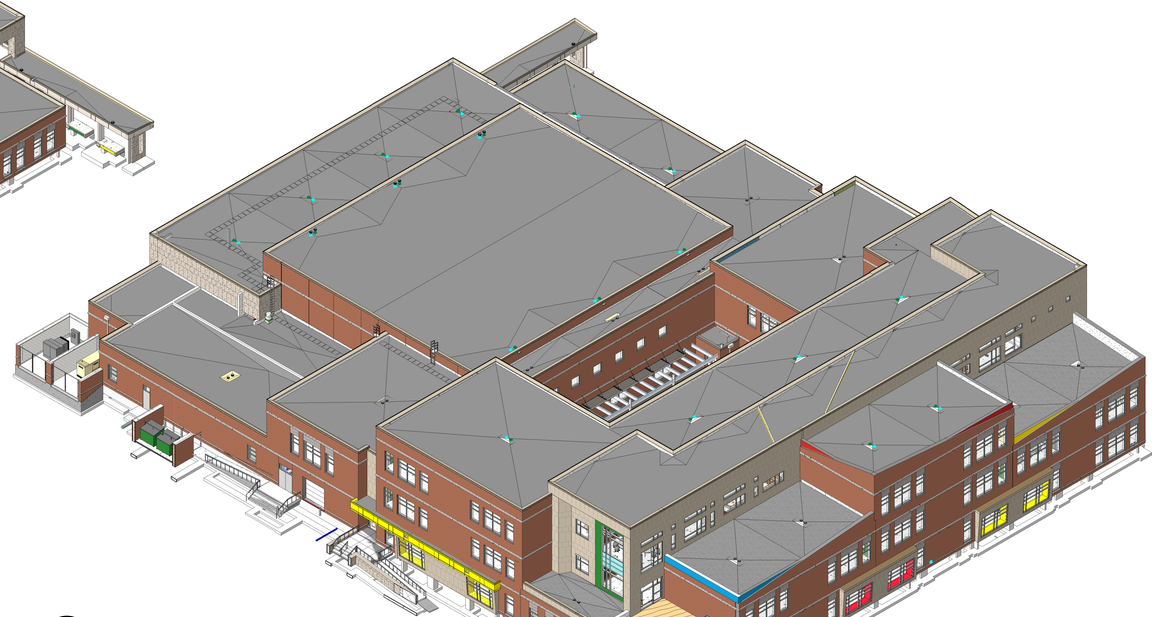
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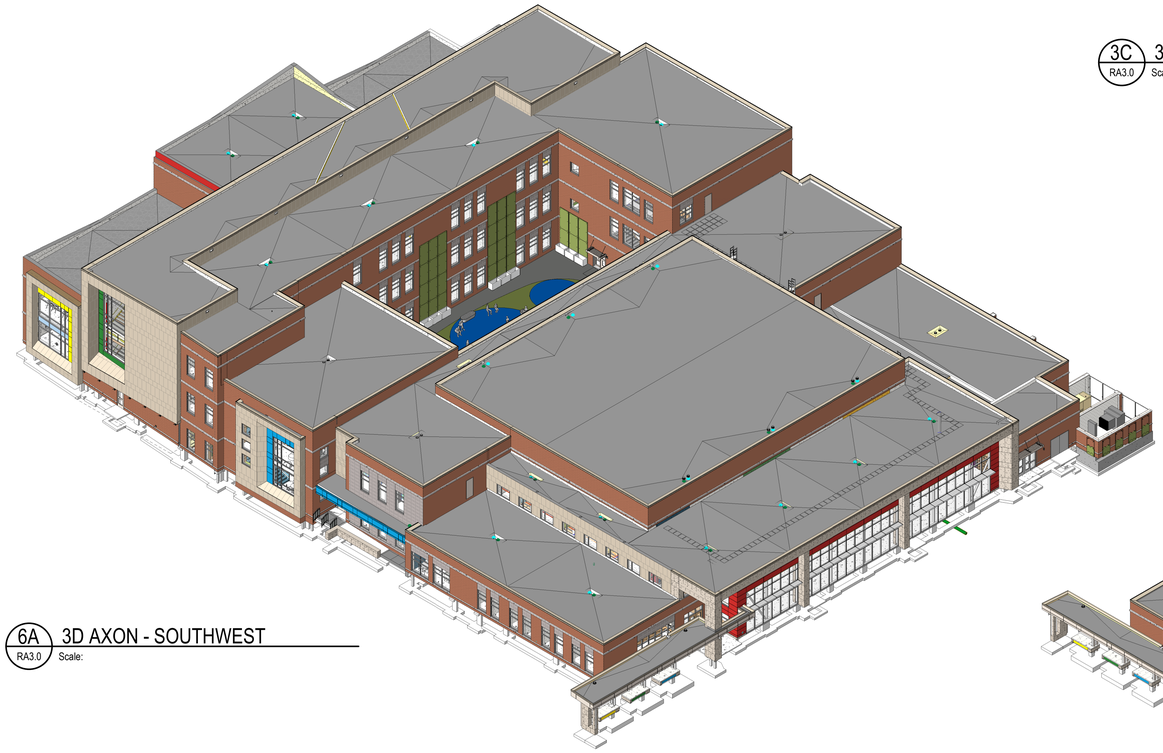
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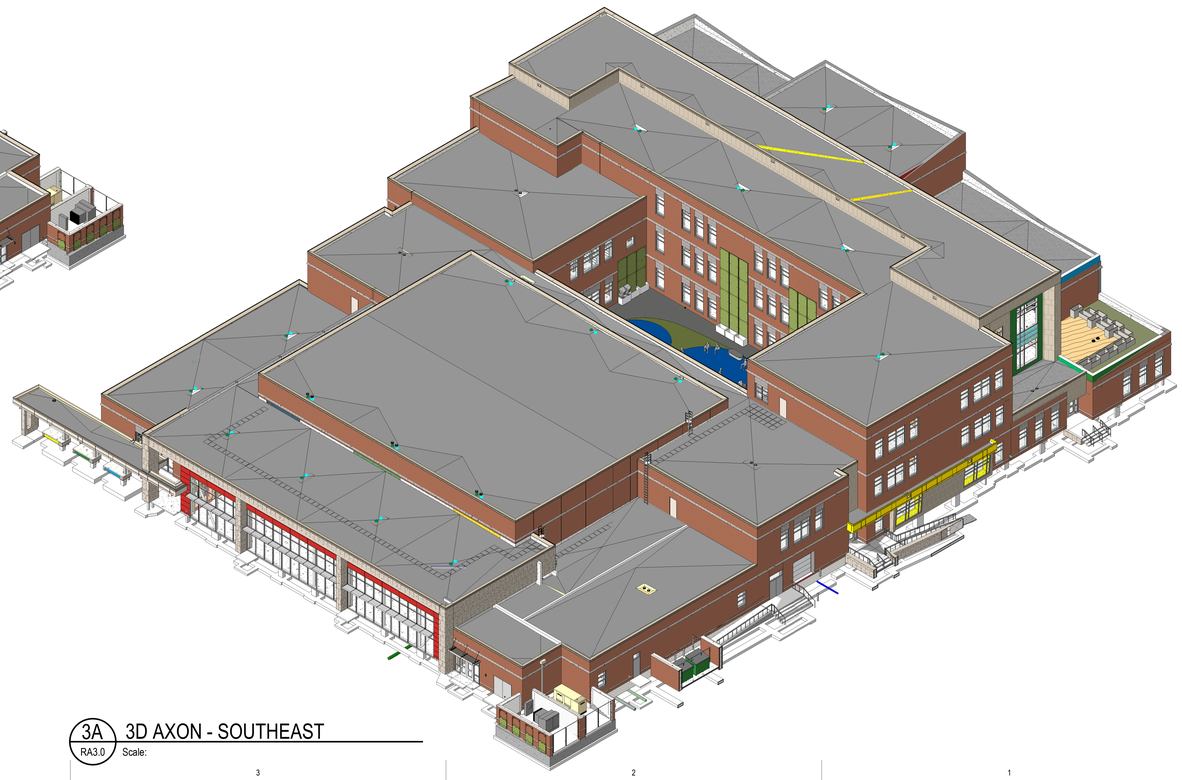
6D 3D AXON - NORTHWEST
RA3.0 Scale:



3C 3D AXON - NORTHEAST
RA3.0 Scale:



6A 3D AXON - SOUTHWEST
RA3.0 Scale:



3A 3D AXON - SOUTHEAST
RA3.0 Scale:

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NOTE: THESE IMAGES ARE FOR REFERENCE ONLY. FURNITURE FOR REFERENCE ONLY, AND NOT PART OF SCOPE.

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3D VIEWS

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RA3.0

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LA CROSSE
WISCONSIN

Department of Planning and Development
400 LA CROSSE ST, LA CROSSE, WI 54601 | P: (608) 789-7512

# Memorandum

To: **DESIGN REVIEW COMMITTEE**
**JOE LEDVINA, LA CROSSE SCHOOL DISTRICT**

From: **TIM ACKLIN, PLANNING AND DEVELOPMENT DEPARTMENT**

Date: **May 30, 2025**

**Re: DESIGN REVIEW PROJECT**
**NEW ELEMENTARY SCHOOL- 807 EAST AVE.**

Design Review Committee Members:
Linnea Miller, Police Department
Tim Acklin, Planning & Development Department
Matt Gallager, Engineering Department
Yuri Nasonovs, Engineering Department
Eddie Young, Building & Inspections Department
Jason Riley, Building & Inspections Department
Bee Xiong, Fire Department- Community Risk Management
Brian Asp, Utilities Department
Leah Miller, Parks, Recreation, and Forestry Department
Jamie Hassemer, Engineering Department
Stephanie Sward, Engineering Department
Cullen Haldeman, Engineering Department

On May 15, 2025, plans were submitted to the Design Review Committee for review of the project located at 807 East Ave. (New Elementary School) The following comments/feedback have been provided.

All revised plans in accordance with this memo must be submitted to the Planning and Development Department for review, unless otherwise stated. No permits will be issued for this project by the Division of Fire Prevention and Building Safety until they receive written confirmation/approval from the Planning and Development Department.

**Requirements Prior to Issuance of a Demolition or Footing & Foundation Permit**

1) Approval of a Certified Survey Map. (If applicable)
2) Combination of parcels for project site. (If applicable)
3) Approval of Final Plans from the Engineering Department.
4) Approval of Final Plans from the Utility/Water Department.
5) Approval of Final Plans from the Division of Fire Prevention and Building Safety (Inspections)

**Requirements Prior to Issuance of a Building Permit**

1) Approval of Final Plans from the Planning and Development Department.

**Requirements Prior to Issuance of an Occupancy Permit**

1) A Letter of Credit in the estimated cost amount of the proposed landscaping has been submitted to the Planning and Development Department to guarantee the proper installation and growth of all landscape improvements proposed in the approved Landscape Plan **OR** all proposed landscaping in the approved Landscape Plan has been installed.
2) A stamped letter of substantial completion from the design engineer of the project within 10 days of completion.
3) Field review and approval of the completed stormwater management facility by the City's Utilities Department.

**Engineering Department (Traffic) - (Contact-Matt Gallager-789-7392, Stephanie Sward-789-8171, Jamie Hassemer- 789-8182, Cullen Haldeman- 789-8185)**

1) Parking stalls on East Ave S & 19th St S cannot be designated for a particular use. Parking in public right of way is open for anyone to use.
2) Curb ramp north of Winnebago Street on East Avenue is not allowed. Midblock curb ramps need to have a matching one on the other side of the street and approved by Engineering & Board of Public Works.
3) Parking blocks will be required on the east side of the parking lot if the sidewalk is not raised above the parking lot.
4) Provide a rectangular dimension for the parking stalls including length & width.
5) Provide dimensions for the two Accessible parking stalls and the access aisle.
6) An Accessible parking space needs to be designated van accessible.
7) What is the angle of the parking stalls? **45 degrees.** May need larger drive aisle to match parking lot standards.
8) Any replacement of curb, pavement, sidewalk, curb ramp, driveways need to follow City Standard Specifications and Detail Drawings.
9) Excavation permit will be required by your contractor before any work can be performed.
10) A temporary traffic control plan will be required for the work in East Avenue & 19th Street
11) If there is a photometric plan, please submit. (Jamie Hassemer)
12) No double parking lane. Follow up with Engineering on this.
13) No curb bump ins on 19th/East Ave. Market, East Ave and 19th need to be no parking on school days.

14) Comments on how storm utility is arranged. Sanitary main and water mains need to meet City policy. Brian Asp and Yuri may have additional comments on this.

**Building and Inspections Department**
**(Contact-Eddie Young- 789-7582, Jason Riley- 789-7585)**

1) Lots to be combined to create 1 parcel.
2) Demo permit
3) Require State approved Building, plumbing and HVAC plans.
4) Submit a separate permit for any proposed signs.
5) With 13' setback, dumpster enclosure and architectural wall need to meet this setback if attached to the building. If enclosure is not attached to the building may be considered a fence and would need fence permit. Planter walls are fine since not attached and landscape item.
6) No plumbing permits until Utilities issues are satisfied.

**Police Department- Linnea Miller-789-7205**

1) No concerns at this time.

**Planning Department-(Contact-Tim Acklin-789-7391)**

1) Parking closer to the street (East Ave) than the building. Will need an exception from the Common Council. **(Post mtg comment. NO exception is required. Municipal Code allows for administrative if site conditions offer no practical alternative)**
2) Provide exterior materials on elevations.
3) Must submit a Landscaping Plan
4) Must submit a Photometric Plan including light specifications and cutsheets.
5) Provide calculation of percent of first floor windows to total façade on all street facing facades. (East, Market, 19th). (The ground/first floor must comprise of windows and doors that equal 20% of the entire façade.)

**Utilities Department- (Brian Asp-789-3897)**

1) Sanitary sewer needs to be extended to the north from the intersection of Winnebago to eliminate long lateral, work with the utility to coordinate work and payment.
2) Watermain needs to be extended to Market St.
3) Storm sewer on East Ave needs to stay in the ROW and make a 90 degree turn into the easement
4) Sanitary Sewer Connection Fee – work with utilities office to calculate
5) Filed easement is required for extension of Winnebago St.
6) Horizontal and Vertical separation must be maintained between utilities per city standard specs – show dimensions on plans

**Engineering Department (Stormwater) - (Contact-Yuri Nasonovs-789-7594)**

1) Mostly covered by Brian and Matt.
2) Need water quality management letter. Requirements are on the website.
3) Stormwater management permit per Sec. 105 of code.
4) Since over 1 acre of disturbance, get DNR permit.
5) Fees associated with water quality management letter and stormwater management permit. May be waived by Board of Public Works.

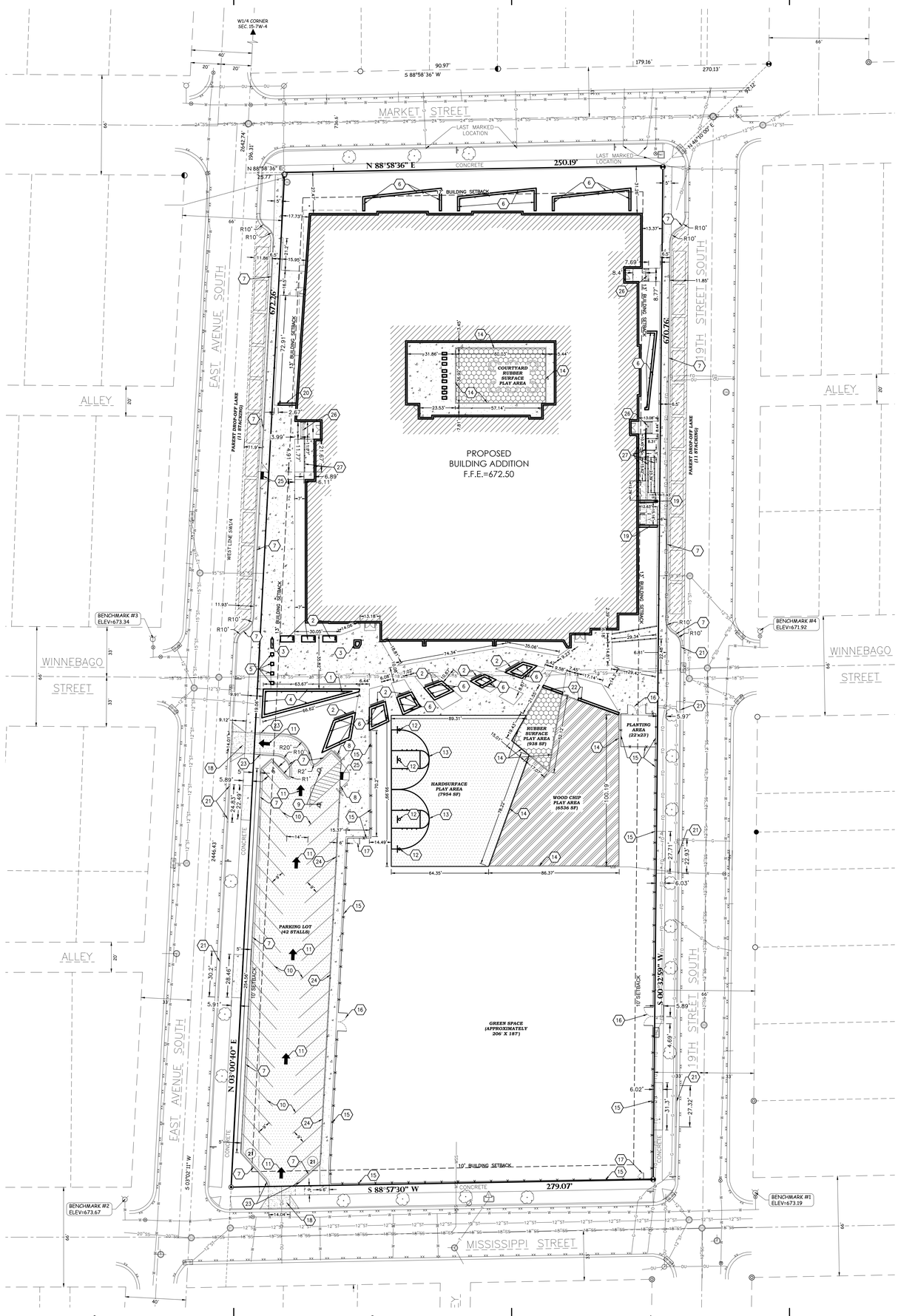
6) Submit plans and models for review in accordance with Chapter 105.

**Fire Department- (Contact- Bee Xiong 789-7260)**

1) Knox Box Required
2) Appropriate plans submitted to the State and the La Crosse Fire Department along with fees and permit applications for prior to any work starting:
   Fire alarm
   Commercial kitchen suppression system
   Fire sprinkler systems
   Email all plans to FD (state approved plans for fire alarm and sprinkler systems required for FD).
3) Sprinkler FDC within 100' of nearest hydrant.
   200 psi hydro underground water service line must be performed along with flush test with FD prior to hooking up to sprinkler riser (preferably as soon as the line is covered with dirt).
   6" minimum water service line.

**Parks, Recreation, and Forestry- (Contact-Leah Miller, 789-8672, Dan Trussoni 789-4915)**

1) Requesting a landscaping plan




## GENERAL NOTES:

1. CONTRACTOR SHALL LOCATE ALL PUBLIC AND PRIVATE UTILITIES PRIOR TO COMMENCEMENT OF WORK.
2. GRADE, LINE, AND LEVEL TO BE REVIEWED IN THE FIELD BY THE CONSTRUCTION MANAGER.
3. ALL REQUIRED EROSION CONTROL MEASURES SHALL BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH LOCAL MUNICIPAL AND DEPARTMENT OF NATURAL RESOURCES REGULATIONS.
4. SEE SHEET C4.0 FOR ALL REQUIRED EROSION CONTROL ELEMENTS.
5. ANY EXISTING UTILITIES NOT SHOWN ON THIS DOCUMENT WHICH NEED TO BE REMOVED, RELOCATED AND OR ADJUSTED SHALL BE THE RESPONSIBILITY OF THE SITE GRADING CONTRACTOR.
6. VERIFY THE LOCATION OF ALL EXISTING UTILITIES PRIOR TO THE START OF DEMOLITION/CONSTRUCTION.
7. BIDDERS SHALL VISIT THE SITE AND REVIEW EXISTING CONDITIONS PRIOR TO THE BID DATE.
8. PRIOR TO STARTING WORK, VERIFY WITH THE LOCAL AUTHORITIES THAT ALL REQUIRED PERMITS HAVE BEEN ACQUIRED.
9. COORDINATE CONSTRUCTION IN THE RIGHT OF WAY WITH THE LOCAL AUTHORITIES.
10. PROVIDE PROPER BARRICADES, SIGNS, AND TRAFFIC CONTROL TO MAINTAIN THRU TRAFFIC ALONG ADJACENT STREETS IN ACCORDANCE WITH LOCAL MUNICIPAL REQUIREMENTS.
11. SIDEWALK JOINTS SHALL BE INSTALLED AS INDICATED OR AS APPROVED BY THE CONSTRUCTION MANAGER.
12. ALL GENERAL LANDSCAPE AREAS SHALL BE SEEDED, FERTILIZED, AND CRIMP HAY MULCHED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS.

## (XX) KEYNOTES:

1. FLAG POLE
2. BENCH
3. CANOPY PILLAR– SEE ARCHITECTURAL PLANS
4. 8" CONCRETE CURB
5. BOLLARD– SEE ARCHITECTURAL PLANS
6. 18" PLANTER
7. 18" CONCRETE CURB & GUTTER
8. ADA PARKING SIGN
9. ADA PARKING STALL
10. PARKING LOT STRIPING
11. DIRECTIONAL ARROW
12. BASKETBALL HOOP
13. PLAYGROUND STRIPING
14. PLAY AREA CURBING
15. 6' TALL CHAIN LINK FENCING
16. 12' SERVICE GATE
17. 4' PEDESTRIAN GATE
18. DRIVE APRON
19. DUMPSTER ENCLOSURE– SEE ARCHITECTURAL PLANS
20. ARCHITECTURAL WALL– SEE ARCHITECTURAL PLANS
21. 6" CONCRETE CURB HEAD– MATCH EXISTING
22. 18" SEAT WALL
23. 6" CURB HEAD TAPER SECTION
24. THICKENED EDGE WALK
25. ADA CURB RAMP
26. CONCRETE STAIRS W/RAILING
27. RAMP RAILING

## PAVEMENT HATCH PATTERNS:

- PROPOSED STANDARD ASPHALT PAVEMENT
- PROPOSED STANDARD CONCRETE PAVEMENT
- PROPOSED REINFORCED CONCRETE PAVEMENT
- PROPOSED RUBBER SURFACE PLAY AREA
- PROPOSED WOOD CHIP PLAY AREA
- PROPOSED PERVIOUS PAVEMENT

## BENCHMARK:

ELEVATIONS ARE REFERENCED TO NAVD 88 DATUM.

**BENCHMARK #1**
NORTHWEST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHEAST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND 19TH STREET SOUTH.
ELEVATION = 673.19

**BENCHMARK #2**
NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND EAST AVENUE SOUTH.
ELEVATION = 673.67

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NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST CORNER OF THE INTERSECTION OF EAST AVENUE SOUTH AND WINNEBAGO STREET.
ELEVATION = 673.34

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BURY BOLT ON HYDRANT, LOCATED AT THE NORTHEAST CORNER OF THE INTERSECTION OF 19TH STREET SOUTH AND WINNEBAGO STREET.
ELEVATION = 671.92

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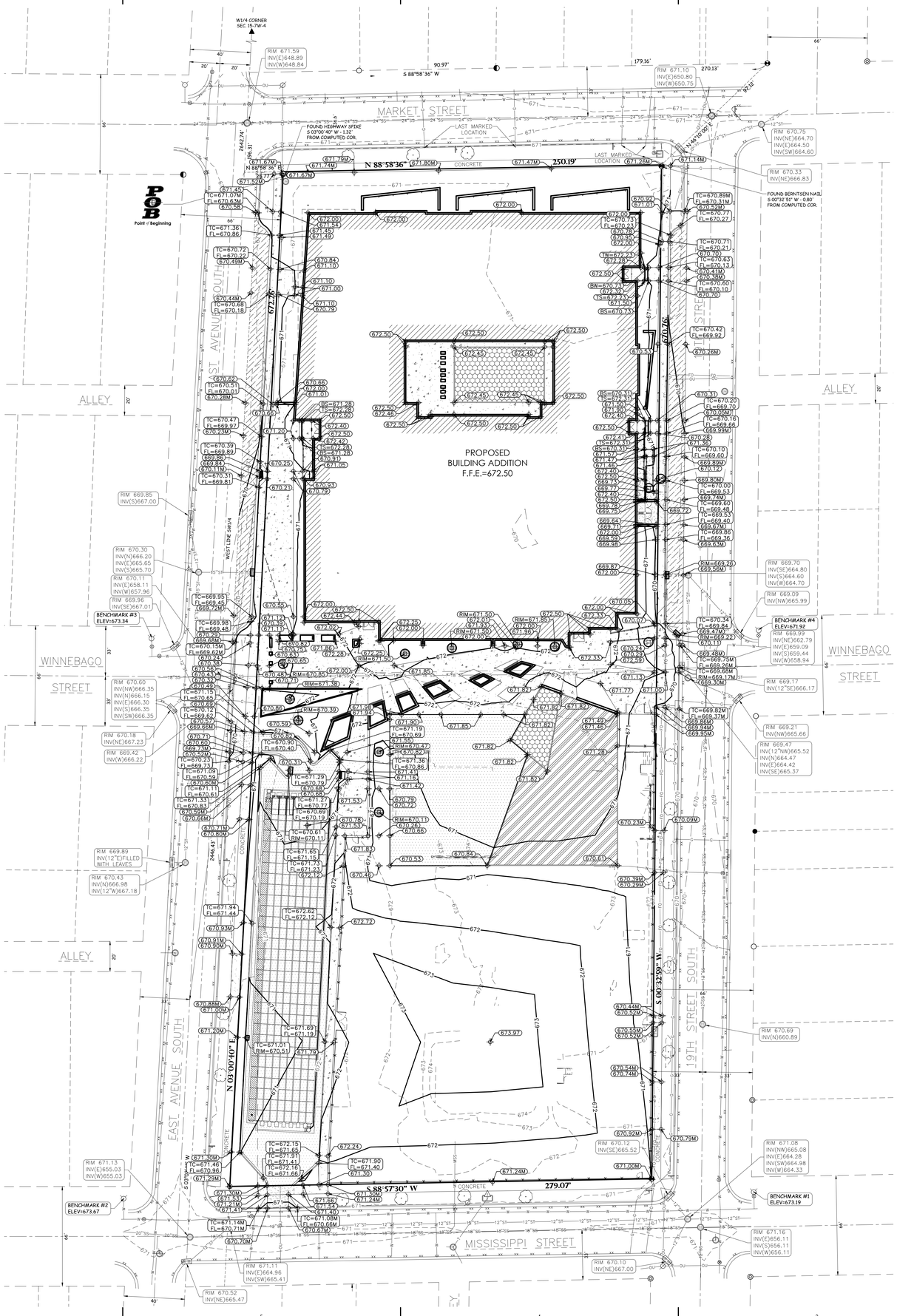
Project Number:
3752

Issued For:
PROGRESS SET
05/15/2025

Sheet Title:
Layout Plan

Sheet Number:
C2.0

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## GENERAL NOTES:

1. CONTRACTOR SHALL LOCATE ALL PUBLIC AND PRIVATE UTILITIES PRIOR TO COMMENCEMENT OF WORK.
2. THE PROPOSED SITE PLAN FINISH FLOOR ELEVATION OF 672.50 EQUALS THE PROPOSED BUILDING ARCHITECTURAL FINISH FLOOR ELEVATION OF 100.00'.
3. GRADE, LINE, AND LEVEL SHALL BE REVIEWED IN THE FIELD BY THE CONSTRUCTION MANAGER.
4. INSTALL AND MAINTAIN ALL REQUIRED EROSION CONTROL MEASURES IN ACCORDANCE WITH LOCAL MUNICIPAL AND DEPARTMENT OF NATURAL RESOURCES REGULATIONS.
5. 6" OF TOPSOIL SHALL BE PROVIDED IN ALL GENERAL LAWN AREAS AND 12" SHALL BE PROVIDED IN ALL PLANTING BED AREAS.
6. SEE SHEET C4.0 FOR ALL REQUIRED EROSION CONTROL ELEMENTS.
7. ANY EXISTING UTILITIES NOT SHOWN ON THIS DOCUMENT WHICH NEED TO BE REMOVED, RELOCATED, AND/OR ADJUSTED SHALL BE THE RESPONSIBILITY OF THE SITE GRADING CONTRACTOR.
8. COORDINATE ALL EARTHWORK ACTIVITIES WITH THE RESPECTIVE TRADES RESPONSIBLE FOR THE INSTALLATION OF GAS, CABLE, TELEPHONE AND ELECTRICAL (INCLUDING MAIN SERVICE, SITE LIGHTING, CONDUITS AND SIGNAGE).
9. INSTALL WisDOT TYPE HR FILTER FABRIC BENEATH ALL RIP RAP.
10. EXCESS TOPSOIL SHALL BE REMOVED FROM SITE, UNLESS OTHERWISE DIRECTED BY THE OWNER. COORDINATE WITH OWNER FOR LOCATION OF STOCKPILE IF THE OWNER CHOOSES TO SALVAGE EXCESS TOPSOIL FOR FUTURE USE. SILT FENCE SHALL BE PLACED AROUND STOCKPILE.
11. ALL TESTING AND INSPECTION SHALL BE DONE IN ACCORDANCE WITH SPS 382.21.
12. THE LOCAL MUNICIPALITY SHALL BE CONTACTED PRIOR TO ANY EXCAVATION IN THE PUBLIC RIGHT-OF-WAY.
13. THE CONTRACTOR SHALL HAVE A TRAFFIC CONTROL PLAN APPROVED PRIOR TO WORK COMMENCING.
14. THE LOCAL MUNICIPALITY SHALL OPERATE ALL EXISTING WATER VALVES IF NEEDED.
15. GRADES AT BUILDING EDGE SHALL BE 6" BELOW FINISHED FLOOR ELEVATION EXCEPT AT DOOR WAY ENTRANCES OR UNLESS OTHERWISE NOTED.

## GRADING LEGEND:

| | |
|---|---|
| EXISTING CONTOUR | — — — 712 — — — |
| PROPOSED CONTOUR | 712 |
| PROPOSED SPOT ELEVATION | 892.26 |
| PROPOSED RIM ELEVATION | RIM=893.56 |
| PROPOSED TOP OF CURB ELEVATION | TC=893.56 |
| PROPOSED FLOW LINE ELEVATION | FL=893.56 |
| PROPOSED TOP OF STAIR ELEVATION | TS=893.56 |
| PROPOSED BOTTOM OF STAIR ELEVATION | BS=893.56 |
| PROPOSED TOP OF WALL ELEVATION | TW=893.56 |
| PROPOSED BOTTOM OF WALL ELEVATION | BW=893.56 |
| PROPOSED MATCH ELEVATION (CONTRACTOR TO VERIFY) | 892.05M |
| PROPOSED STORM SEWER MANHOLE | ST |
| PROPOSED STORM SEWER CLEANOUT | |
| PROPOSED STORM SEWER INLET | |
| PROPOSED STORM SEWER CURB INLET | |
| PROPOSED SANITARY SEWER CLEANOUT | |
| PROPOSED DOWNSPOUT CONNECTION | |

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ELEVATIONS ARE REFERENCED TO NAVD 88 DATUM.

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ELEVATION = 673.19

**BENCHMARK #2**
NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST QUADRANT OF THE INTERSECTION OF MISSISSIPPI STREET AND EAST AVENUE SOUTH.
ELEVATION = 673.67

**BENCHMARK #3**
NORTHEAST FLANGE BOLT ON HYDRANT, LOCATED AT THE NORTHWEST CORNER OF THE INTERSECTION OF EAST AVENUE SOUTH AND WINNEBAGO STREET.
ELEVATION = 673.34

**BENCHMARK #4**
BURY BOLT ON HYDRANT, LOCATED AT THE NORTHEAST CORNER OF THE INTERSECTION OF 19TH STREET SOUTH AND WINNEBAGO STREET.
ELEVATION = 671.92

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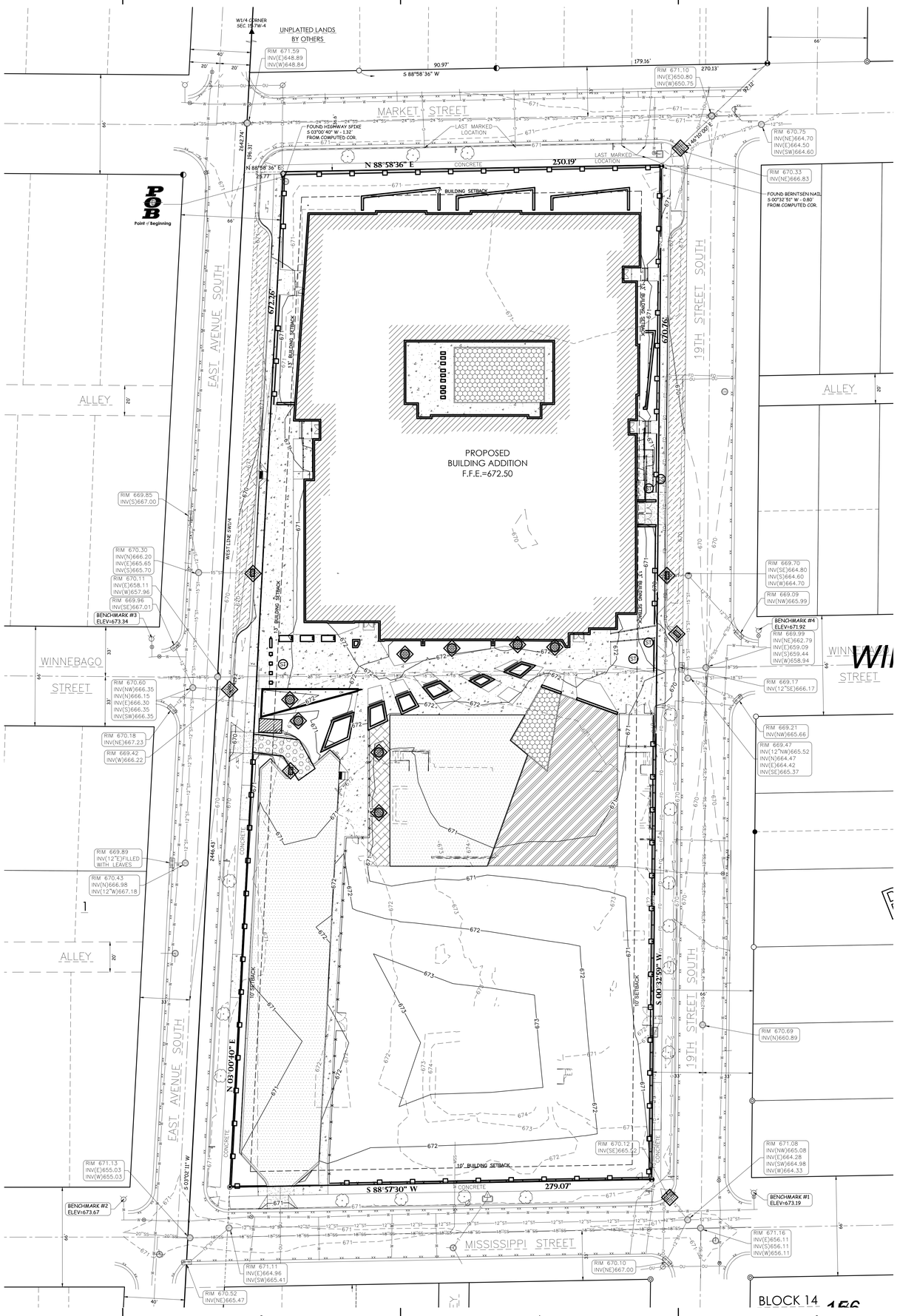
Project Number: 3752

Issued For: PROGRESS SET 05/15/2025

Sheet Title: Grading Plan

Sheet Number: C3.0

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## GENERAL NOTES:

1. CONTRACTOR SHALL LOCATE ALL PUBLIC AND PRIVATE UTILITIES PRIOR TO COMMENCEMENT OF WORK.
2. NOTIFY THE LOCAL MUNICIPALITY AT LEAST 2 WORKING DAYS PRIOR TO THE START OF SOIL DISTURBING ACTIVITIES.
3. INSTALL ALL TEMPORARY EROSION CONTROL ELEMENTS PRIOR TO THE START OF DEMOLITION/CONSTRUCTION.
4. ALL ACTIVITIES SHALL BE CONDUCTED IN A LOGICAL SEQUENCE TO MINIMIZE THE AMOUNT OF BARE SOIL EXPOSED AT ANY ONE TIME. MAINTAIN EXISTING VEGETATION AS LONG AS POSSIBLE.
5. CRUSHED ROCK DRIVES FOR SEDIMENT TRACKING UTILIZING 3" CRUSHED ROCK SHALL BE MAINTAINED AT ALL CONSTRUCTION ENTRANCES TO THE SITE. THE ROCK DRIVE SHALL BE A MINIMUM OF 12" THICK AND BE A MINIMUM OF 50 FEET IN LENGTH BY THE WIDTH OF THE DRIVEWAY.
6. OFFSITE SEDIMENT DEPOSITS RESULTING FROM STORMWATER RUNOFF SHALL BE CLEANED BY THE END OF THE NEXT WORKDAY. OFFSITE SEDIMENT DEPOSITS RESULTING FROM CONTRACTOR ACTIVITIES, INCLUDING SOIL TRACKING, SHALL BE CLEANED EACH WORKDAY. EXCESSIVE AMOUNTS OF SEDIMENT OR DEBRIS TRACKED ONTO ADJACENT STREETS SHALL BE CLEANED IMMEDIATELY. FINE SEDIMENT ACCUMULATIONS ON ADJACENT STREETS SHALL SWEPT MECHANICALLY OR MANUALLY AT LEAST WEEKLY AND BEFORE IMMINENT RAINFALL.
7. DISTURBED GROUND OUTSIDE OF THE EVERYDAY CONSTRUCTION AREAS, INCLUDING SOIL STOCKPILES, THAT ARE LEFT INACTIVE FOR MORE THAN 7 DAYS SHALL BE TEMPORARILY STABILIZED BY SEEDING/MULCHING OR OTHER APPROVED METHODS.
8. WASTE MATERIAL THAT IS GENERATED ON THE CONSTRUCTION SITE SHALL BE PROPERLY DISPOSED OF AND NOT ALLOWED TO RUN INTO RECEIVING WATERS.
9. EROSION CONTROL DEVICES DESTROYED AS A RESULT OF CONSTRUCTION ACTIVITIES SHALL BE REPAIRED BY THE END OF EACH WORK DAY.
10. INSPECT ALL EROSION CONTROL MEASURES AT LEAST ONCE A WEEK AND AFTER ANY RAINFALL OF 0.5" OR MORE. MAKE NEEDED REPAIRS AND DOCUMENT ALL ACTIVITIES AS PER THE REQUIREMENTS OF THE NOTICE OF INTENT SUBMITTED BY THE PROJECT CIVIL ENGINEER.
11. ALL TEMPORARY EROSION CONTROL ELEMENTS SHALL REMAIN IN PLACE UNTIL A SUFFICIENT GROWTH OF VEGETATION IS ESTABLISHED AND THEN BE REMOVED AS PART OF THE BASE BID.
12. IF SEDIMENT LADEN WATER NEEDS TO BE REMOVED FROM THE SITE, FILTER BAGS OR SCREENING SHALL BE USED IN ACCORDANCE WITH WI DNR TECHNICAL STANDARD 1061 TO PREVENT SEDIMENT DISCHARGE TO THE MAXIMUM EXTENT PRACTICABLE.
13. COORDINATE ALL EARTHWORK ACTIVITIES WITH THE RESPECTIVE TRADES RESPONSIBLE FOR THE INSTALLATION OF GAS, CABLE, TELEPHONE AND ELECTRICAL (INCLUDING MAIN SERVICE, SITE LIGHTING, CONDUITS AND SIGNAGE).
14. INSTALL WisDOT TYPE HR FILTER FABRIC BENEATH ALL RIP RAP.
15. IF BARE SOIL IS EXPOSED DURING THE WINTER MONTHS, STABILIZATION BY MULCHING OR ANIONIC POLYACRYLAMIDE SHALL OCCUR PRIOR TO SNOWFALL OR GROUND FREEZE.
16. SILT FENCE SHALL BE INSTALLED AROUND THE TOPSOIL STOCKPILE.
17. THE CONTRACTOR SHALL PERFORM INSPECTIONS AND MONITORING OF EROSION CONTROL PRACTICES IN ACCORDANCE WITH THE WI DNR "CONSTRUCTION SITE INSPECTION REPORT" FORM 3400-187. THIS FORM CAN BE FOUND IN THE CONSTRUCTION SPECIFICATIONS.

## EROSION CONTROL LEGEND:

EXISTING CONTOUR — — — 888 — — —

PROPOSED CONTOUR ——— 888 ———

PROPOSED SILT FENCE

PROPOSED INLET PROTECTION

EROSION CONTROL BLANKET

ROCK CONSTRUCTION ENTRANCE

CONCRETE WASHOUT AREA

STORM WATER OVERLAND FLOW DIRECTION

## EROSION CONTROL SEQUENCING:

1. INSTALL PERIMETER EROSION CONTROL
2. BEGIN DEMOLITION.
3. BEGIN ROUGH GRADING AND UTILITY INSTALLATION
4. DURING GRADING ACTIVITIES EXISTING GRASS AND VEGETATION, TO BE REMOVED, SHALL REMAIN IN PLACE FOR AS LONG AS POSSIBLE, TO AVOID SEDIMENT TRANSPORT.
5. TEMPORARY STABILIZATION ACTIVITY SHALL COMMENCE WHEN LAND DISTURBING CONSTRUCTION ACTIVITIES HAVE TEMPORARILY CEASED AND WILL NOT RESUME FOR A PERIOD EXCEEDING 14 CALENDAR DAYS.
6. FINAL STABILIZATION ACTIVITY SHALL COMMENCE WHEN LAND DISTURBING ACTIVITIES CEASE AND FINAL GRADE HAS BEEN REACHED ON ANY PORTION OF THE SITE.
7. IF DISTURBED AREAS MUST BE LEFT OVER WINTER, AN ANIONIC POLYACRYLAMIDE SHALL BE APPLIED TO ALL DISTURBED AREAS PRIOR TO GROUND FREEZE. SEE SPECIFICATIONS FOR DETAILS.

## BENCHMARK:

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ELEVATION = 673.19

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ELEVATION = 673.67

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ELEVATION = 673.34

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BURY BOLT ON HYDRANT, LOCATED AT THE NORTHEAST CORNER OF THE INTERSECTION OF 19TH STREET SOUTH AND WINNEBAGO STREET.
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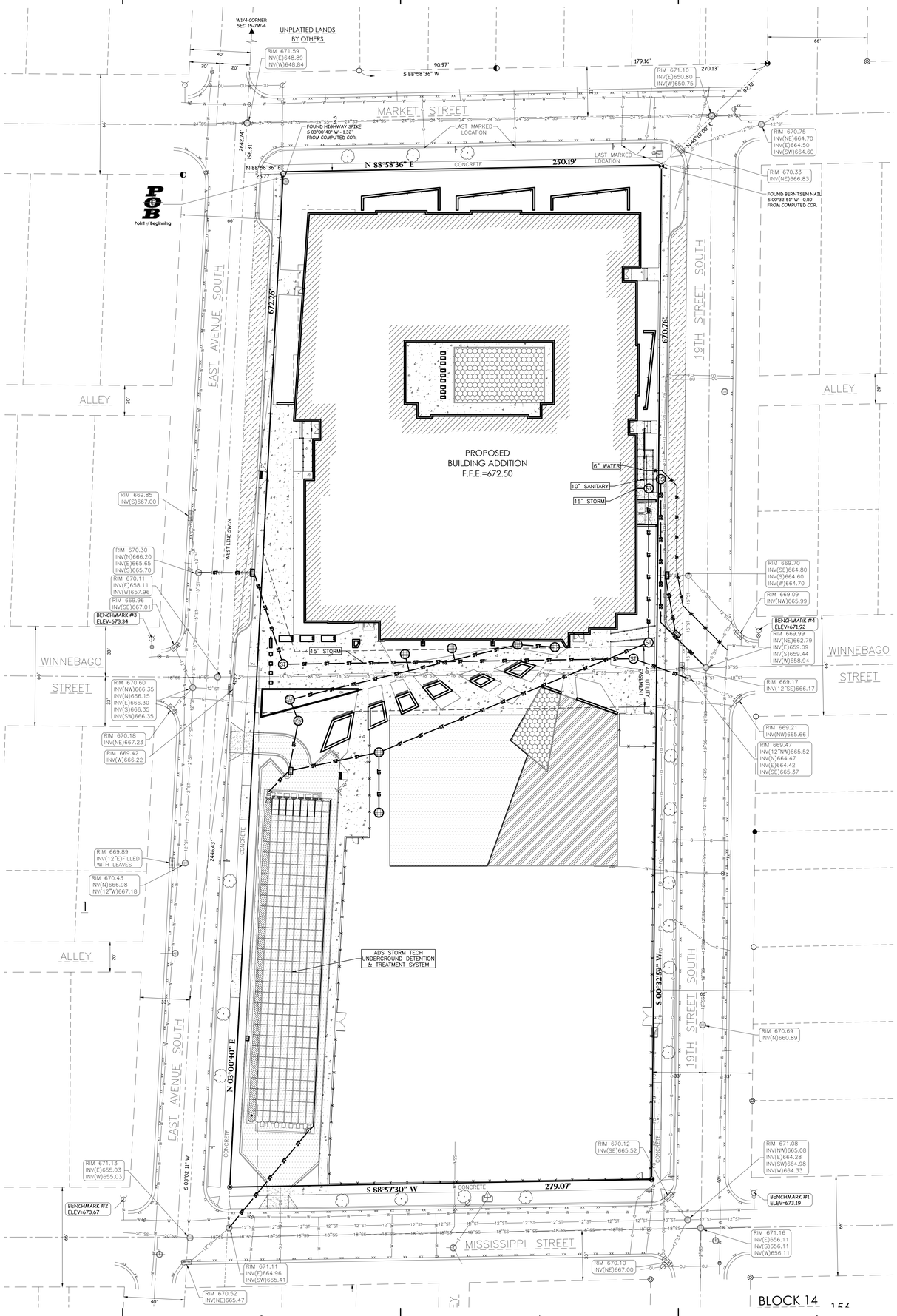
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Issued For: PROGRESS SET 05/15/2025

Sheet Title: Erosion Control Plan

Sheet Number: C4.0

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3. ANY EXISTING UTILITIES NOT SHOWN ON THIS DOCUMENT WHICH NEED TO BE REMOVED, RELOCATED AND OR ADJUSTED SHALL BE THE RESPONSIBILITY OF THE SITE GRADING CONTRACTOR.
4. REFER TO THE PROPOSED BUILDING MECHANICAL/PLUMBING PLANS TO VERIFY EXACT CONNECTION LOCATIONS AND SIZES OF PROPOSED SANITARY SEWER AND WATER LATERALS.
5. COORDINATE ALL UTILITY WORK WITH THE RESPECTIVE TRADES RESPONSIBLE FOR THE INSTALLATION OF GAS, CABLE, TELEPHONE AND ELECTRICAL (INCLUDING MAIN SERVICE, SITE LIGHTING, CONDUITS AND SIGNAGE).
6. COORDINATE UTILITY SERVICE DISCONNECTIONS/OUTAGES WITH OWNER AND ANY IMPACTED NEIGHBORS. MINIMIZE DISRUPTIONS TO THE MAXIMUM EXTENT PRACTICAL.
7. COORDINATE ALL WORK WITHIN THE PUBLIC RIGHT OF WAY WITH THE LOCAL MUNICIPALITY.
8. ALL TESTING AND INSPECTION SHALL BE DONE IN ACCORDANCE WITH SPS 382.21.
9. THE PROPOSED WATER MAIN SHALL HAVE A MINIMUM COVER OF 7'-6" TO THE TOP OF PIPE FROM PROPOSED FINISHED GRADE. SEE SHEET C3.0 FOR PROPOSED FINISHED GRADE.
10. THE MUNICIPALITY SHALL BE CONTACTED PRIOR TO ANY EXCAVATION IN THE PUBLIC RIGHT-OF-WAY, AND PRIOR TO CONNECTING SANITARY SEWER AND WATER LATERALS TO THE PUBLIC MAINS.
11. THE CONTRACTOR SHALL HAVE A TRAFFIC CONTROL PLAN APPROVED PRIOR TO WORK COMMENCING.
12. THE MUNICIPALITY SHALL OPERATE ALL EXISTING WATER VALVES, IF NEEDED.
13. FIELD VERIFY INVERT ELEVATION OF THE SANITARY SEWER AND WATER PUBLIC MAIN, AT THE LOCATION OF THE SERVICE LATERAL CONNECTIONS, PRIOR TO CONNECTING THE LATERALS TO THE PUBLIC MAIN.
14. INSTALL WisDOT TYPE HR FILTER FABRIC BENEATH PROPOSED RIP RAP.

## UTILITY LEGEND:

- PROPOSED STORM SEWER
- PROPOSED SANITARY SEWER
- PROPOSED WATER MAIN
- PROPOSED STORM SEWER MANHOLE
- PROPOSED STORM SEWER INLET
- PROPOSED STORM SEWER CURB INLET

## BENCHMARK:

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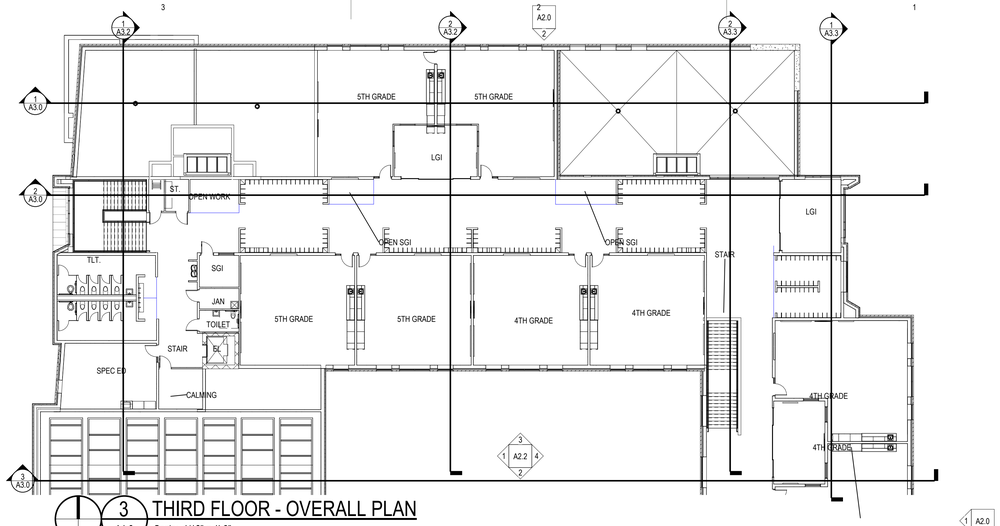
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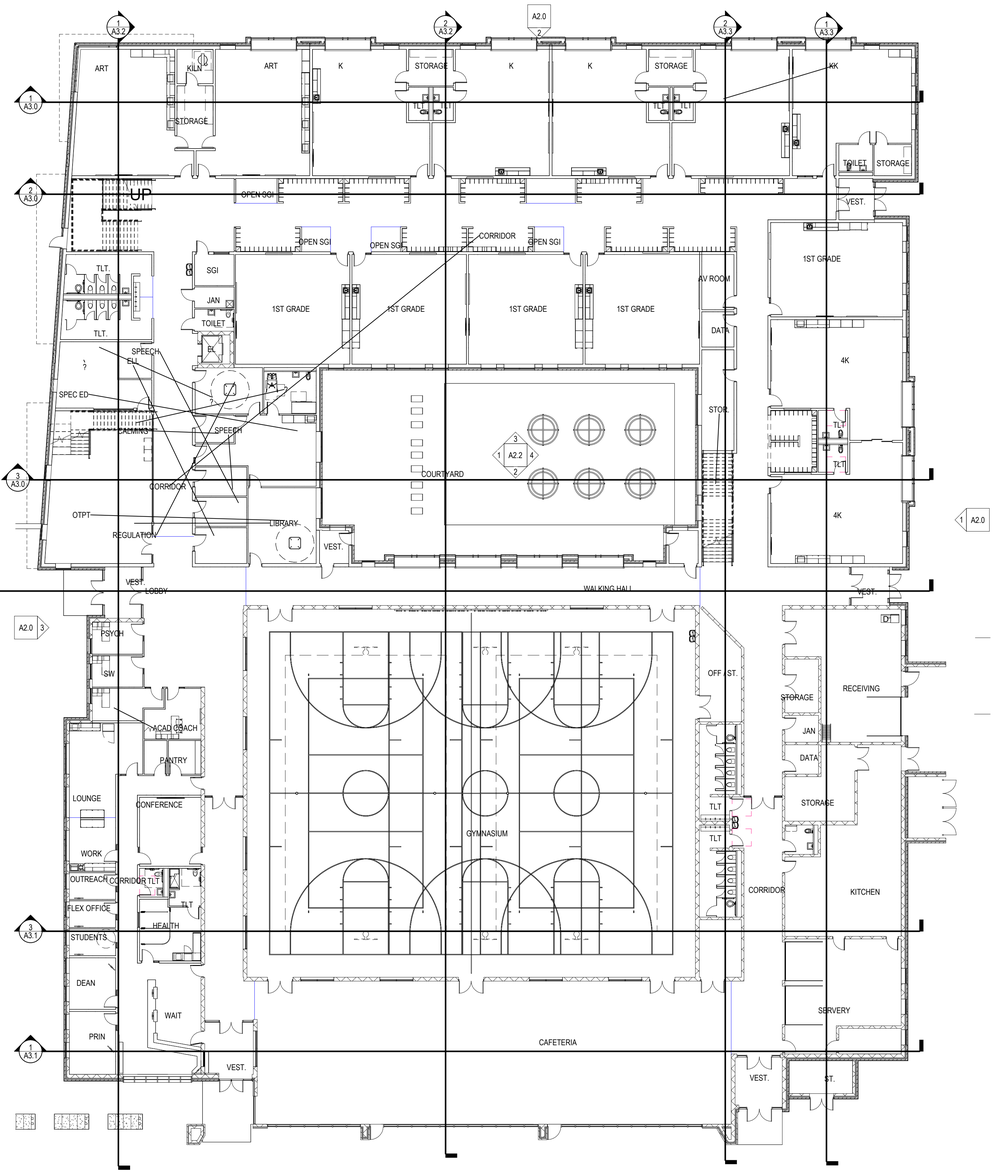
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Sheet Number: C5.0

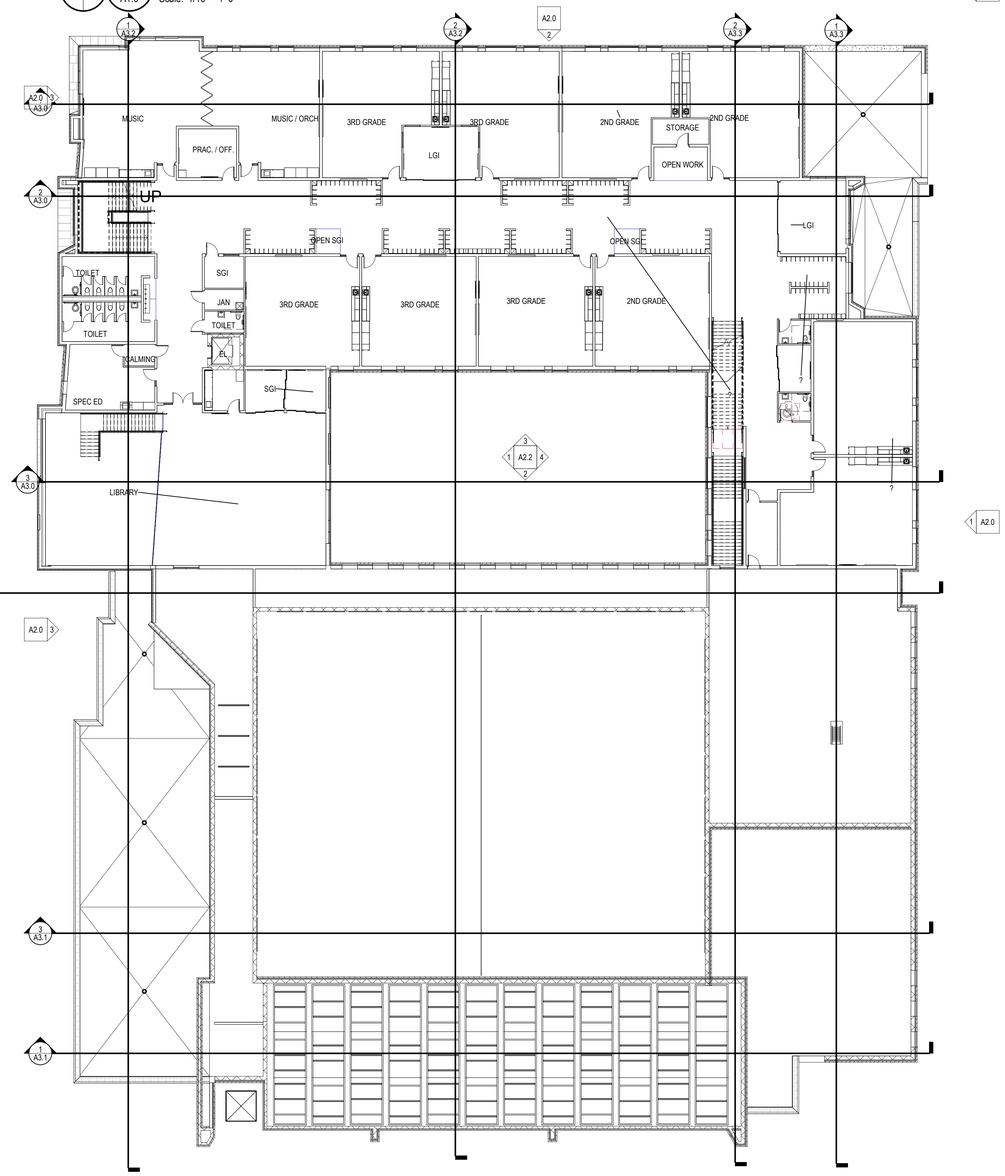
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3 THIRD FLOOR - OVERALL PLAN
A1.0 Scale: 1/16" = 1'-0"




1 FIRST FLOOR - OVERALL PLAN
A1.0 Scale: 1/16" = 1'-0"




2 SECOND FLOOR - OVERALL PLAN
A1.0 Scale: 1/16" = 1'-0"

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**New Elementary School**
**La Crosse School District**
**807 East Ave S**
**La Crosse, WI 54601**

REVISIONS:

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Project Number:
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Sheet Title:
**OVERALL FLOOR PLANS**

Sheet Number:
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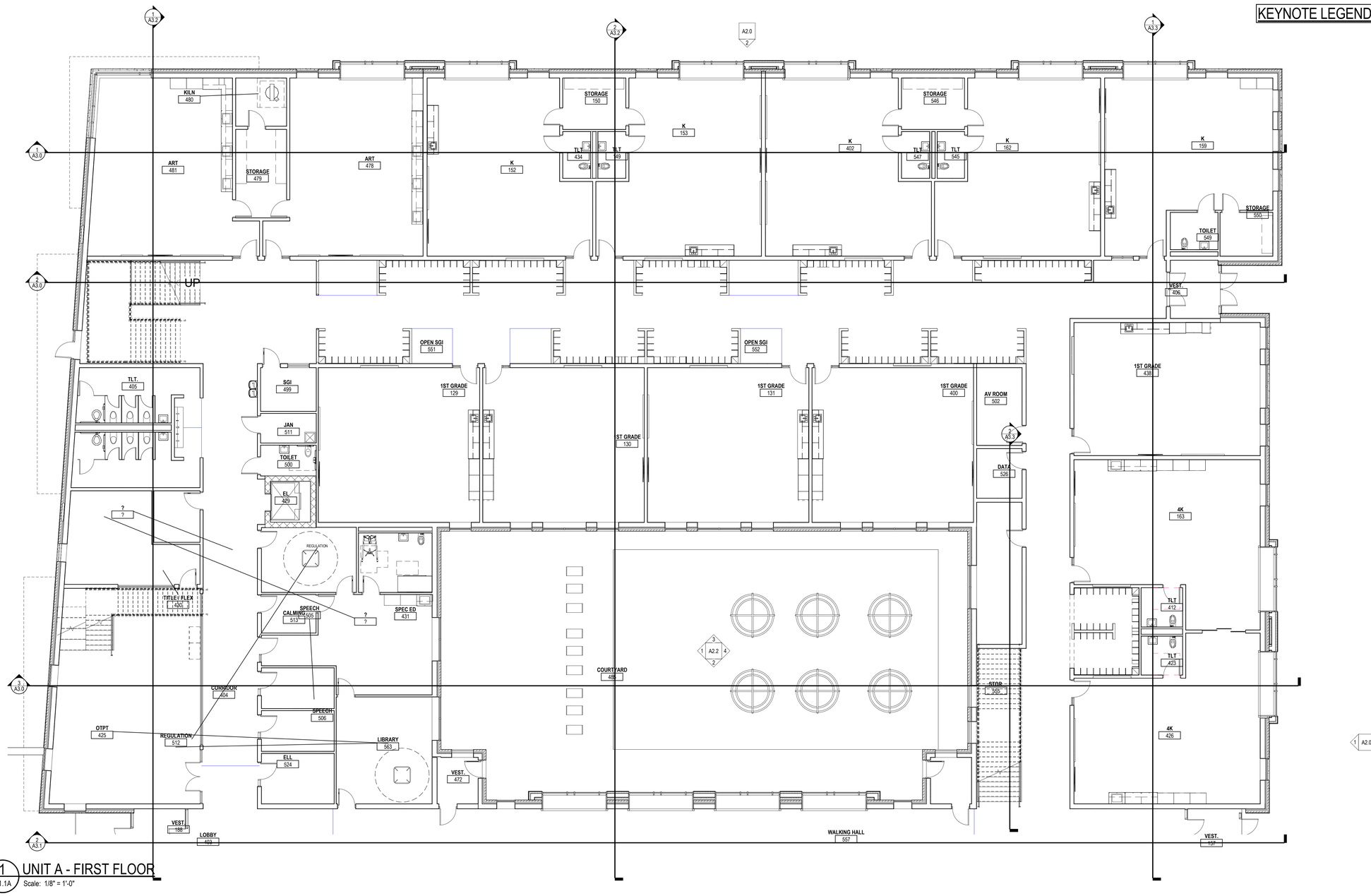
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# GENERAL NOTES

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# KEYNOTE LEGEND




1 A1.1A UNIT A - FIRST FLOOR
Scale: 1/8" = 1'-0"


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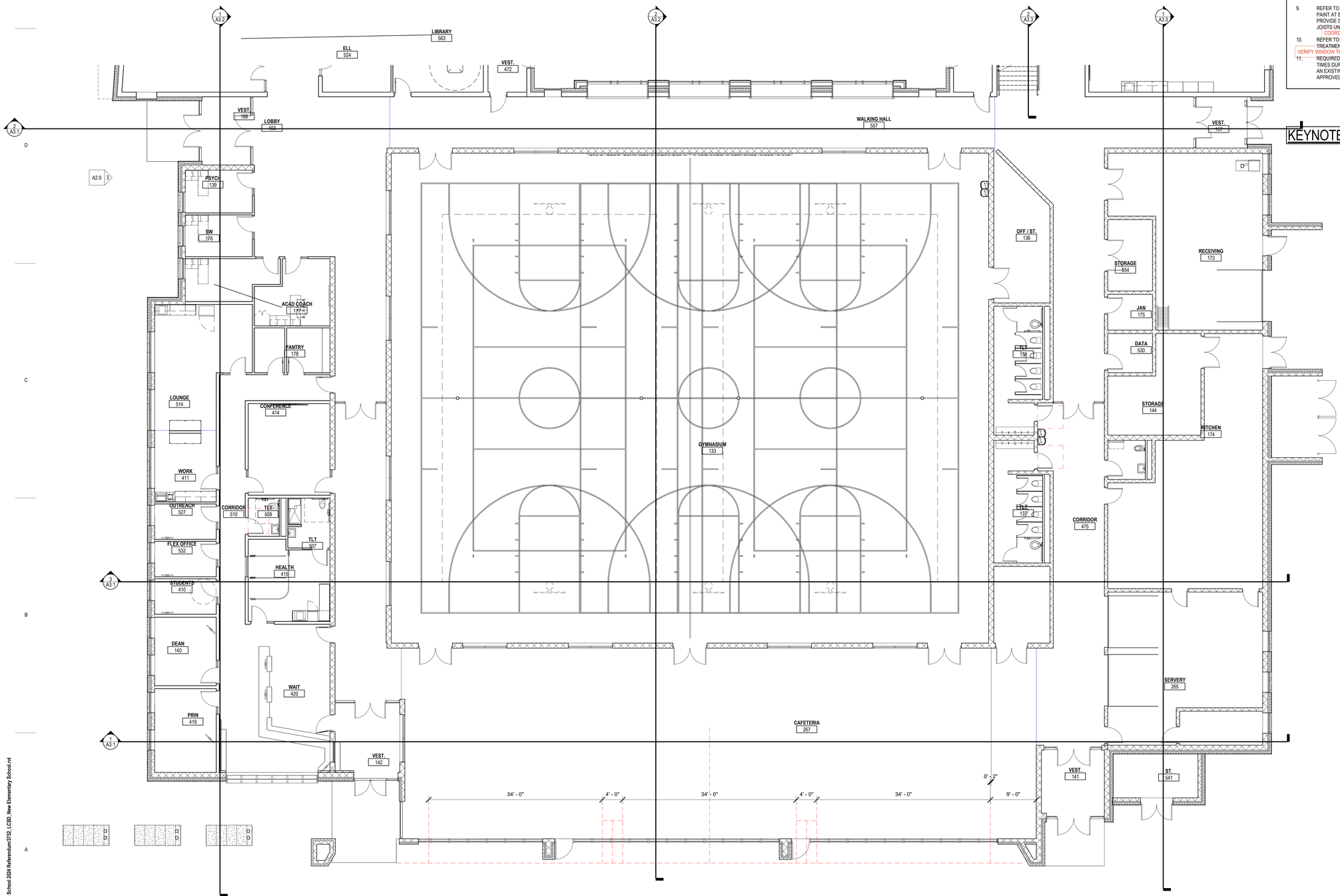
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**UNIT A - FIRST FLOOR PLAN**

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## KEYNOTE LEGEND

1 A1.1B **UNIT B - FIRST FLOOR**
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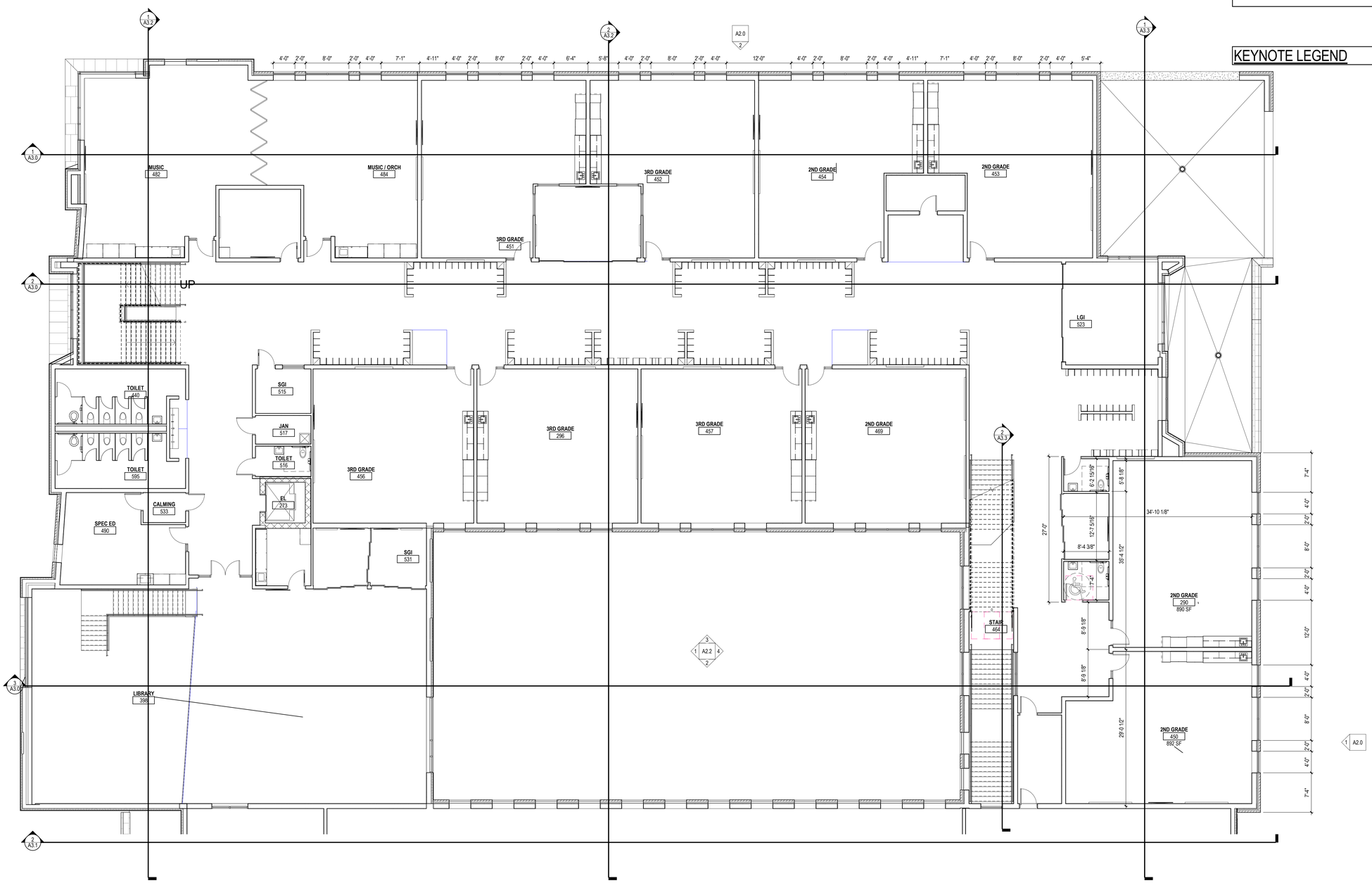
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# KEYNOTE LEGEND




1 A1.2A UNIT A - SECOND FLOOR
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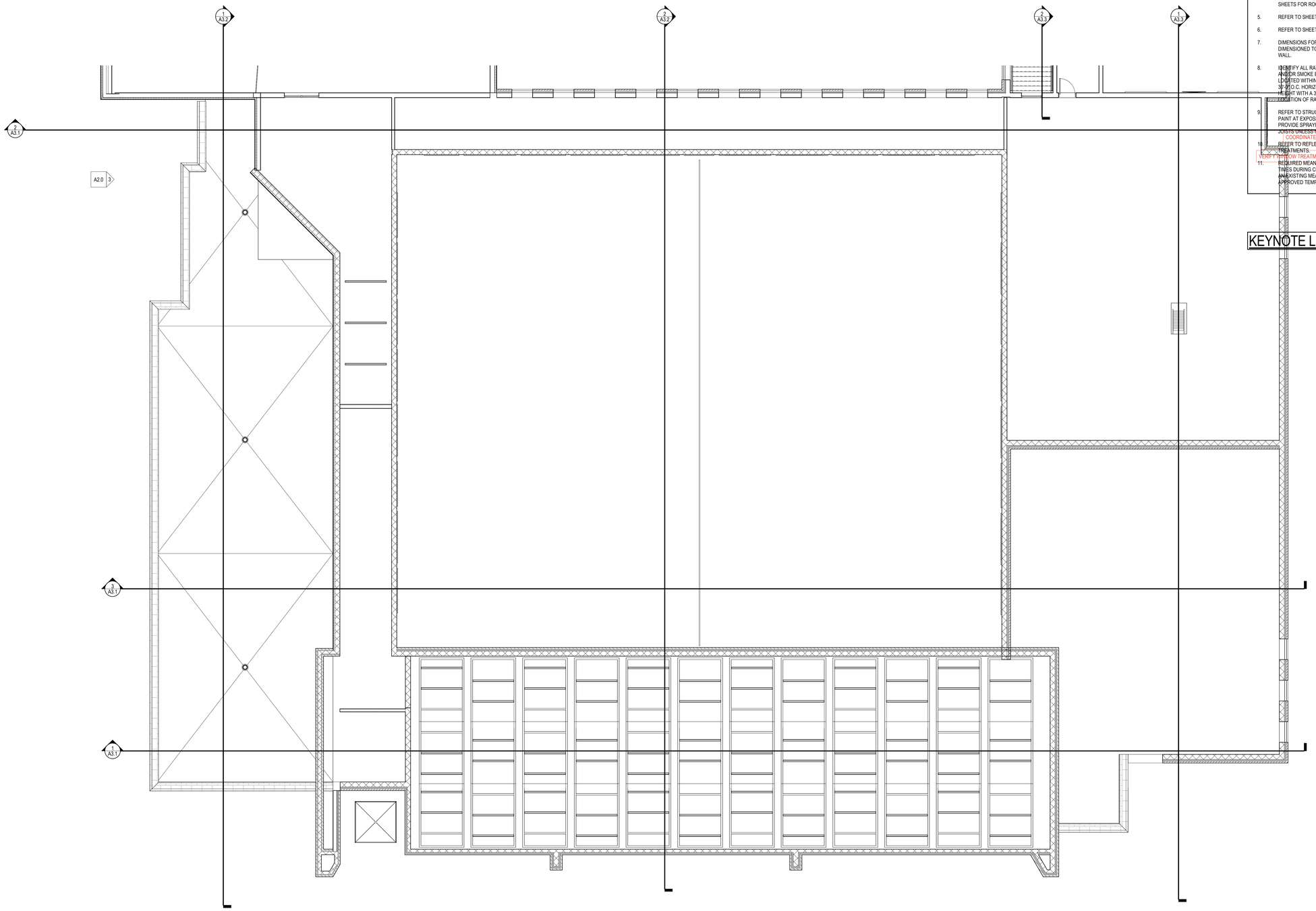
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## KEYNOTE LEGEND

## 1 A1.2B UNIT B - SECOND FLOOR

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**UNIT B - SECOND FLOOR PLAN**

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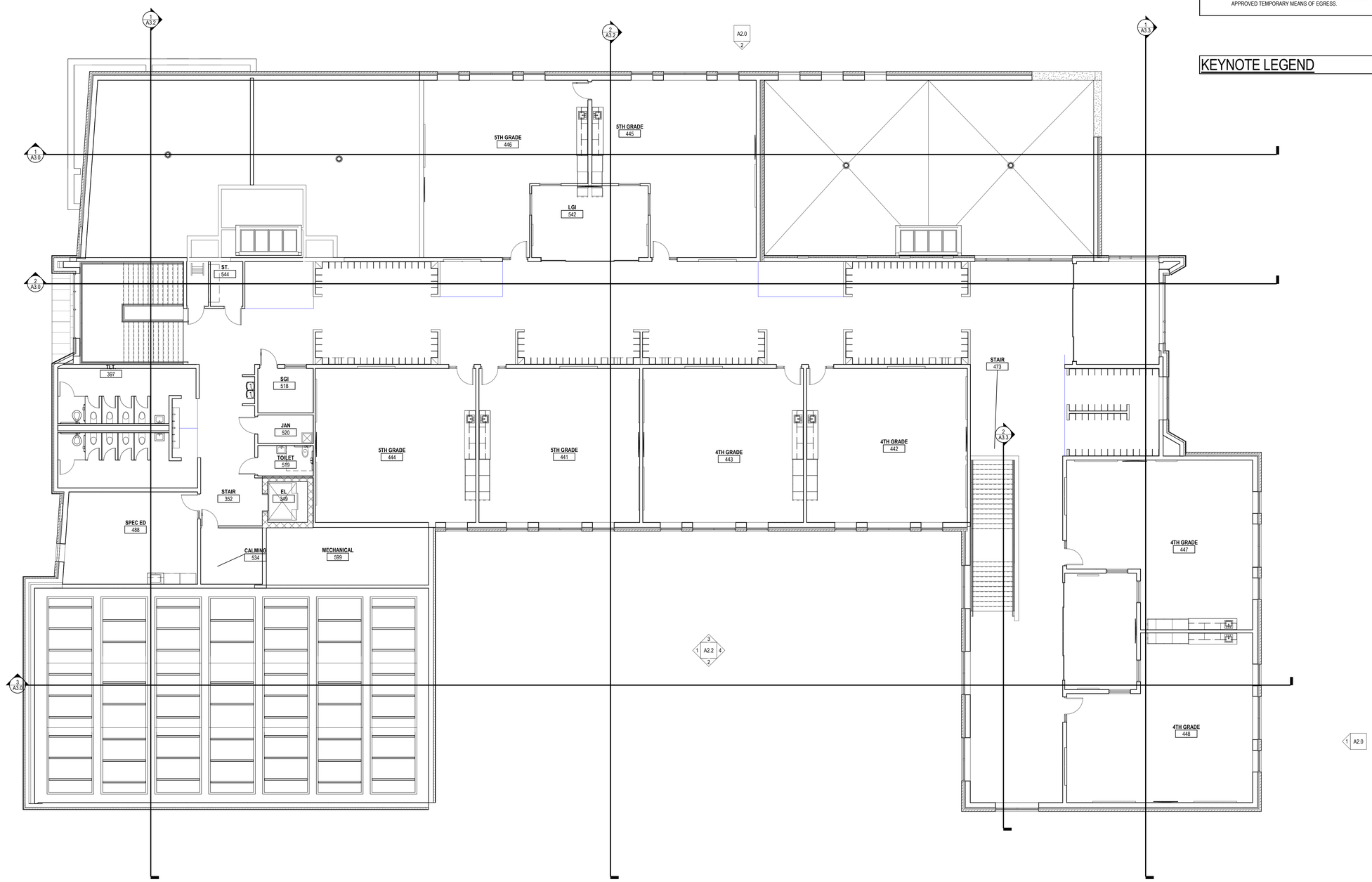
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# KEYNOTE LEGEND




1 A1.3A UNIT A - THIRD FLOOR (A2.0 3)
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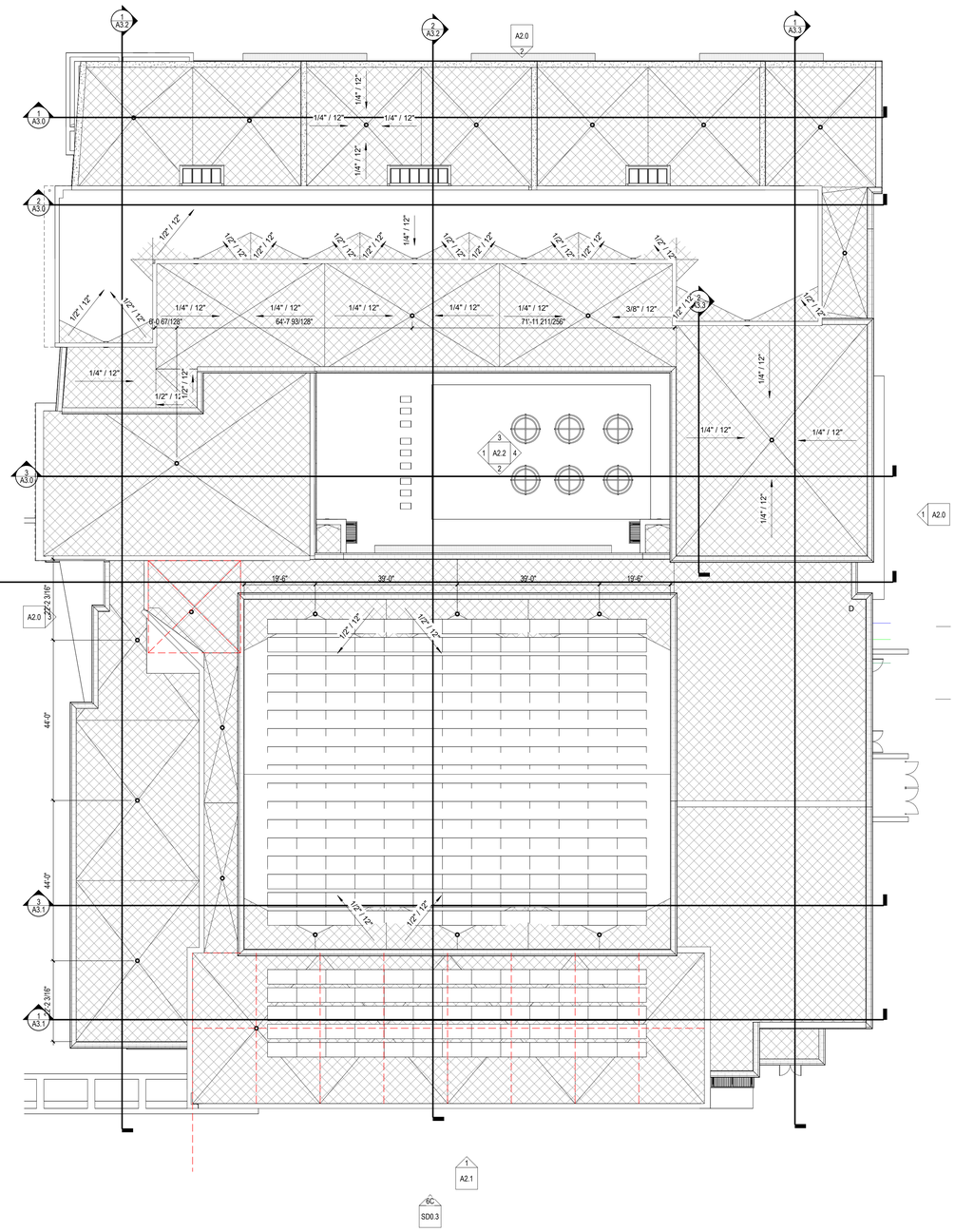
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# ROOF PLAN LEGEND & NOTES

VERIFY ALL ROOF TYPES & COORDINATE WITH OWNER. DELETE ALL TYPES NOT BEING USED IN PROJECT ONCE CONFIRMED.

ROOF TYPE 1 (RT-1):

- EPDM SHEET
- COVERBOARD
- 5" INSULATION DEPTH (MINIMUM) WITH A MINIMUM OF 2 LAYERS; STAGGER JOINTS; TAPERED INSULATION AS SHOWN ON PLAN
- THERMAL BARRIER AT SPRAY APPLIED FIRE PROOFING LOCATIONS ONLY - SEE STRUCTURAL
- SLOPED STRUCTURE WITH METAL ROOF DECK, SEE STRUCTURAL FOR SIZE & TYPE

ROOF TYPE 2 (RT-2):

- EPDM SHEET
- COVERBOARD
- TAPERED INSULATION, 1/4" PER FOOT MINIMUM SLOPE, 5" DEPTH (MINIMUM) WITH A MINIMUM OF 2 LAYERS; STAGGER JOINTS
- THERMAL BARRIER AT SPRAY APPLIED FIRE PROOFING LOCATIONS ONLY - SEE STRUCTURAL
- FLAT STRUCTURE WITH METAL ROOF DECK, SEE STRUCTURAL FOR SIZE & TYPE

ROOF TYPE 3 (RT-3):

- EPDM SHEET
- COVERBOARD
- 5" INSULATION DEPTH (MINIMUM) WITH A MINIMUM OF 2 LAYERS; STAGGER JOINTS; TAPERED INSULATION AS SHOWN ON PLAN
- THERMAL BARRIER
- SLOPED STRUCTURE WITH ACOUSTICAL METAL ROOF DECK, SEE STRUCTURAL FOR SIZE & TYPE

ROOF TYPE 4 (RT-4):

- EPDM SHEET
- COVERBOARD
- TAPERED INSULATION, 1/4" PER FOOT MINIMUM SLOPE, 5" DEPTH (MINIMUM) WITH A MINIMUM OF 2 LAYERS; STAGGER JOINTS
- THERMAL BARRIER
- FLAT STRUCTURE WITH ACOUSTICAL METAL ROOF DECK, SEE STRUCTURAL FOR SIZE & TYPE

ENSURE THIS TAG IS SHOWN AT ALL ROOF DRAINS & PERIMETER EDGES

#" DENOTES APPROXIMATE THICKNESS OF TAPERED INSULATION

DENOTES WALKWAY PAVERS

DENOTES ROOF CRICKETS;
MAINTAIN 1/2" PER FOOT SLOPE TO DRAINS

DENOTES TAPERED INSULATION;
MAINTAIN 1/4" PER FOOT SLOPE TO DRAINS

1. ALL ROOF SLOPES TO BE MINIMUM 1/4" PER FOOT UNLESS NOTED OTHERWISE.
2. ALL ROOF TOP CURBS TO BE SADDLED WITH MINIMUM 1/2" PER FOOT PITCH.
3. NOT ALL ROOF PENETRATIONS ARE SHOWN. VERIFY PENETRATIONS WITH ALL BUILDING TRADES.
4. SEE HVAC DRAWINGS FOR SIZES, QUANTITIES, LOCATIONS AND TYPES OF HVAC PENETRATIONS AND ROOF TOP MOUNTED EQUIPMENT.
5. SEE DETAIL ___ FOR VENT PIPE FLASHING DETAIL. VERIFY ALL LOCATIONS AND QUANTITIES W/ PLUMBING CONTRACTOR.
6. SEE DETAIL ___ FOR CURB FLASHING DETAIL.
7. SEE DETAIL ___ FOR TYPICAL ROOF DRAIN DETAIL.
8. SEE DETAIL ___ FOR TYPICAL OVERFLOW SCUPPER DETAIL.
9. SEE DETAIL ___ FOR TYPICAL ROOF SADDLES.
10. SEE DETAIL ___ FOR ROOF LADDER.

# KEYNOTE LEGEND

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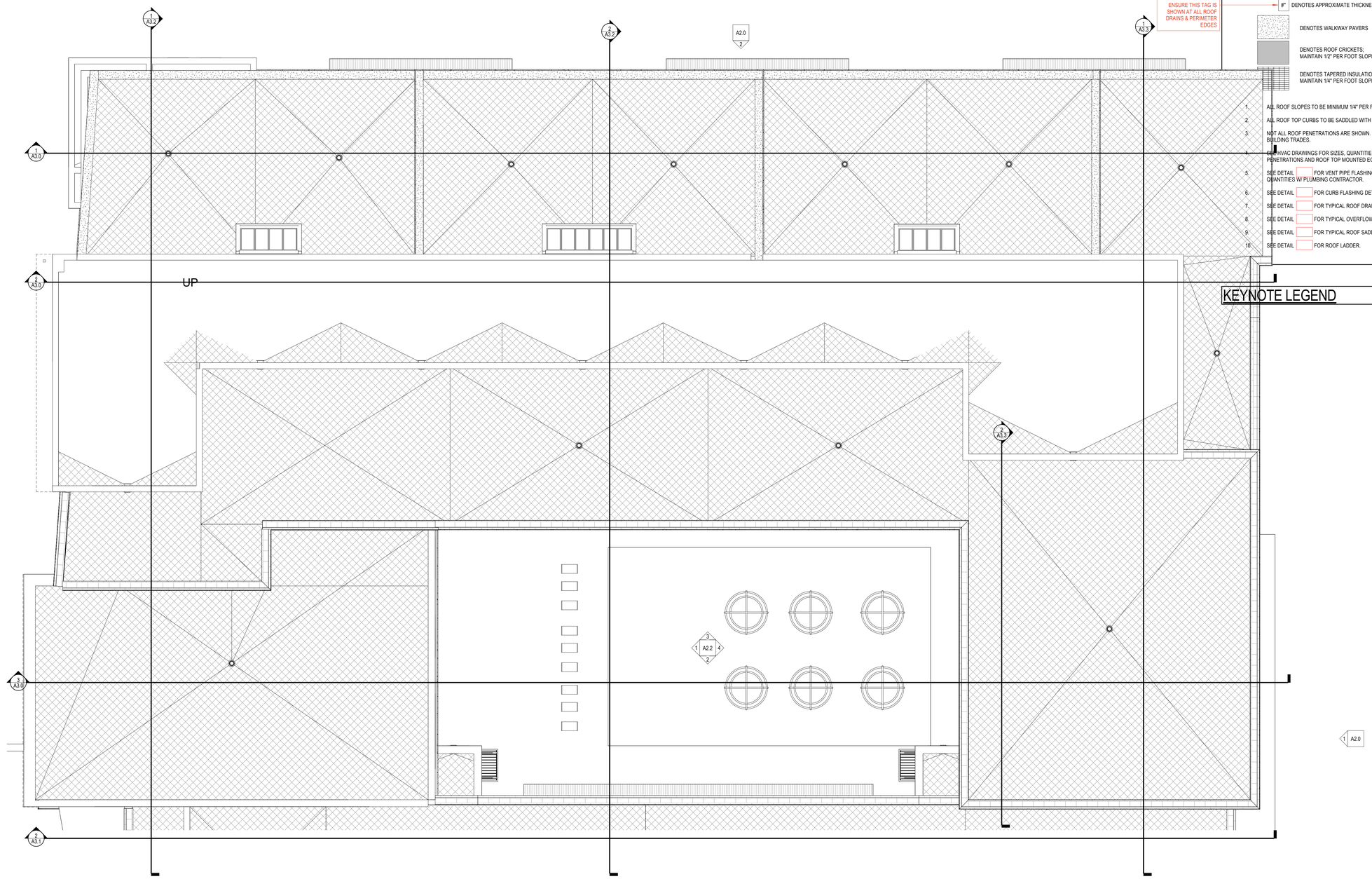
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- SLOPED STRUCTURE WITH METAL ROOF DECK, SEE STRUCTURAL FOR SIZE & TYPE

ROOF TYPE 2 (RT-2):

- EPDM SHEET
- COVERBOARD
- TAPERED INSULATION, 1/4" PER FOOT MINIMUM SLOPE, 5" DEPTH (MINIMUM) WITH A MINIMUM OF 2 LAYERS; STAGGER JOINTS
- THERMAL BARRIER AT SPRAY APPLIED FIRE PROOFING LOCATIONS ONLY - SEE STRUCTURAL
- FLAT STRUCTURE WITH METAL ROOF DECK, SEE STRUCTURAL FOR SIZE & TYPE

ROOF TYPE 3 (RT-3):

- EPDM SHEET
- COVERBOARD
- 5" INSULATION DEPTH (MINIMUM) WITH A MINIMUM OF 2 LAYERS; STAGGER JOINTS; TAPERED INSULATION AS SHOWN ON PLAN
- THERMAL BARRIER
- SLOPED STRUCTURE WITH ACOUSTICAL METAL ROOF DECK, SEE STRUCTURAL FOR SIZE & TYPE

ROOF TYPE 4 (RT-4):

- EPDM SHEET
- COVERBOARD
- TAPERED INSULATION, 1/4" PER FOOT MINIMUM SLOPE, 5" DEPTH (MINIMUM) WITH A MINIMUM OF 2 LAYERS; STAGGER JOINTS
- THERMAL BARRIER
- FLAT STRUCTURE WITH ACOUSTICAL METAL ROOF DECK, SEE STRUCTURAL FOR SIZE & TYPE

ENSURE THIS TAG IS SHOWN AT ALL ROOF DRAINS & PERIMETER EDGES

#" DENOTES APPROXIMATE THICKNESS OF TAPERED INSULATION

DENOTES WALKWAY PAVERS

DENOTES ROOF CRICKETS; MAINTAIN 1/2" PER FOOT SLOPE TO DRAINS

DENOTES TAPERED INSULATION; MAINTAIN 1/4" PER FOOT SLOPE TO DRAINS

1. ALL ROOF SLOPES TO BE MINIMUM 1/4" PER FOOT UNLESS NOTED OTHERWISE.
2. ALL ROOF TOP CURBS TO BE SADDLED WITH MINIMUM 1/2" PER FOOT PITCH.
3. NOT ALL ROOF PENETRATIONS ARE SHOWN. VERIFY PENETRATIONS WITH ALL BUILDING TRADES.
4. SEE HVAC DRAWINGS FOR SIZES, QUANTITIES, LOCATIONS AND TYPES OF HVAC PENETRATIONS AND ROOF TOP MOUNTED EQUIPMENT.
5. SEE DETAIL ___ FOR VENT PIPE FLASHING DETAIL. VERIFY ALL LOCATIONS AND QUANTITIES W/ PLUMBING CONTRACTOR.
6. SEE DETAIL ___ FOR CURB FLASHING DETAIL.
7. SEE DETAIL ___ FOR TYPICAL ROOF DRAIN DETAIL.
8. SEE DETAIL ___ FOR TYPICAL OVERFLOW SCUPPER DETAIL.
9. SEE DETAIL ___ FOR TYPICAL ROOF SADDLES.
10. SEE DETAIL ___ FOR ROOF LADDER.

## KEYNOTE LEGEND

1 A1.4A UNIT A - ROOF PLAN
Scale: 1/8" = 1'-0"

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New Elementary School
La Crosse School District
807 East Ave S
La Crosse, WI 54601

REVISIONS:

| # | DATE | DESCRIPTION |
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Project Number: 3752

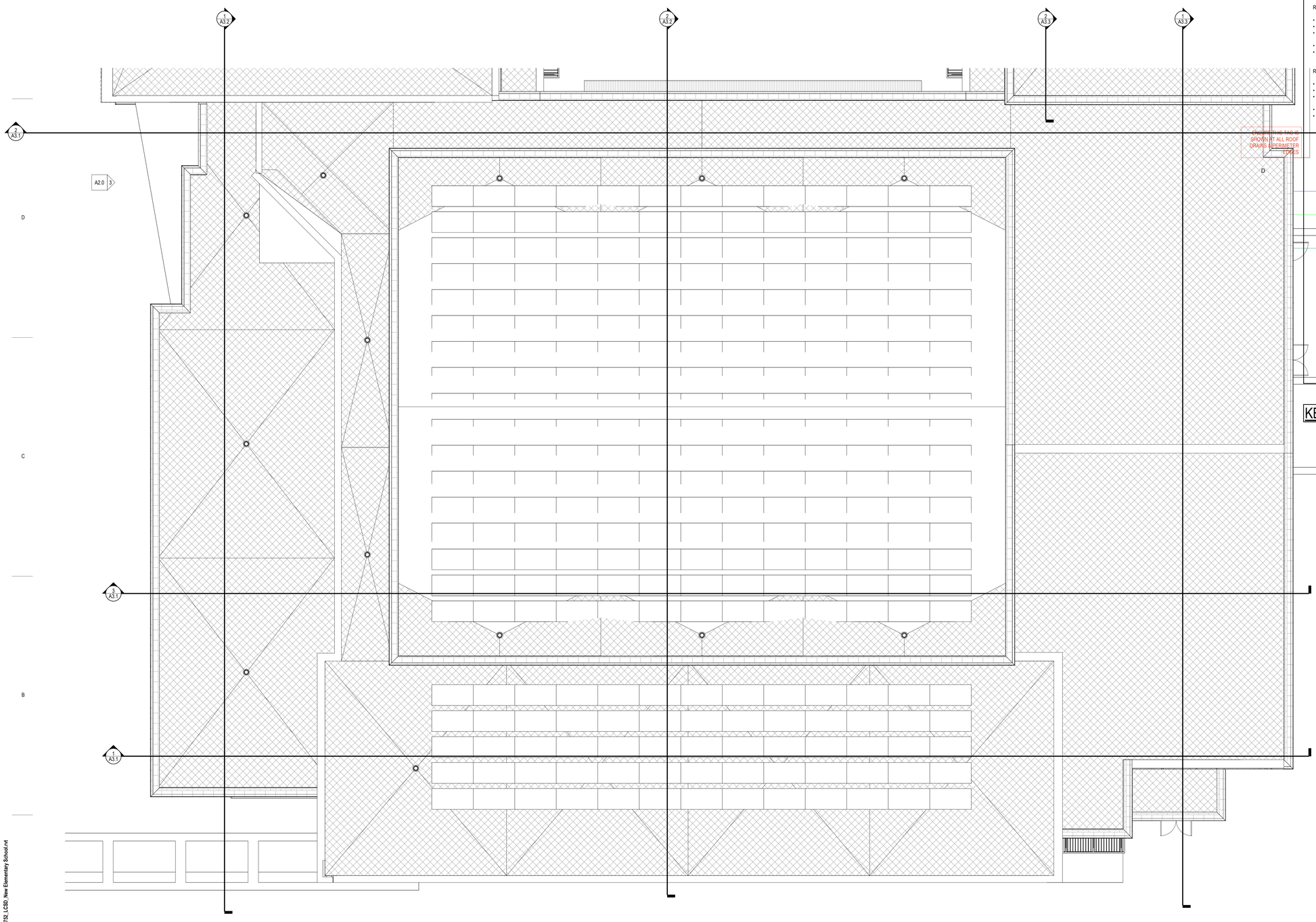
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Sheet Title: UNIT A - ROOF PLAN

Sheet Number: A1.4A

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## UNIT B - ROOF PLAN

1 A1.4B — Scale: 1/8" = 1'-0"

## ROOF PLAN LEGEND & NOTES

VERIFY ALL ROOF TYPES & COORDINATE WITH OWNER. DELETE ALL TYPES NOT BEING USED IN PROJECT ONCE CONFIRMED.

ROOF TYPE 1 (RT-1):

- EPDM SHEET
- COVERBOARD
- 5" INSULATION DEPTH (MINIMUM) WITH A MINIMUM OF 2 LAYERS; STAGGER JOINTS; TAPERED INSULATION AS SHOWN ON PLAN
- THERMAL BARRIER AT SPRAY APPLIED FIRE PROOFING LOCATIONS ONLY - SEE STRUCTURAL
- SLOPED STRUCTURE WITH METAL ROOF DECK, SEE STRUCTURAL FOR SIZE & TYPE

ROOF TYPE 2 (RT-2):

- EPDM SHEET
- COVERBOARD
- TAPERED INSULATION, 1/4" PER FOOT MINIMUM SLOPE, 5" DEPTH (MINIMUM) WITH A MINIMUM OF 2 LAYERS; STAGGER JOINTS
- THERMAL BARRIER AT SPRAY APPLIED FIRE PROOFING LOCATIONS ONLY - SEE STRUCTURAL
- FLAT STRUCTURE WITH METAL ROOF DECK, SEE STRUCTURAL FOR SIZE & TYPE

ROOF TYPE 3 (RT-3):

- EPDM SHEET
- COVERBOARD
- 5" INSULATION DEPTH (MINIMUM) WITH A MINIMUM OF 2 LAYERS; STAGGER JOINTS; TAPERED INSULATION AS SHOWN ON PLAN A2.0
- THERMAL BARRIER
- SLOPED STRUCTURE WITH ACOUSTICAL METAL ROOF DECK, SEE STRUCTURAL FOR SIZE & TYPE

ROOF TYPE 4 (RT-4):

- EPDM SHEET
- COVERBOARD
- TAPERED INSULATION, 1/4" PER FOOT MINIMUM SLOPE, 5" DEPTH (MINIMUM) WITH A MINIMUM OF 2 LAYERS; STAGGER JOINTS
- THERMAL BARRIER
- FLAT STRUCTURE WITH ACOUSTICAL METAL ROOF DECK, SEE STRUCTURAL FOR SIZE & TYPE

CHOOSE THIS TAG IS SHOWN AT ALL ROOF DRAINS, PERIMETER EDGES

D

8" DENOTES APPROXIMATE THICKNESS OF TAPERED INSULATION

DENOTES WALKWAY PAVERS

DENOTES ROOF CRICKETS; MAINTAIN 1/2" PER FOOT SLOPE TO DRAINS

DENOTES TAPERED INSULATION; MAINTAIN 1/4" PER FOOT SLOPE TO DRAINS

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## KEYNOTE LEGEND

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**La Crosse, WI 54601**

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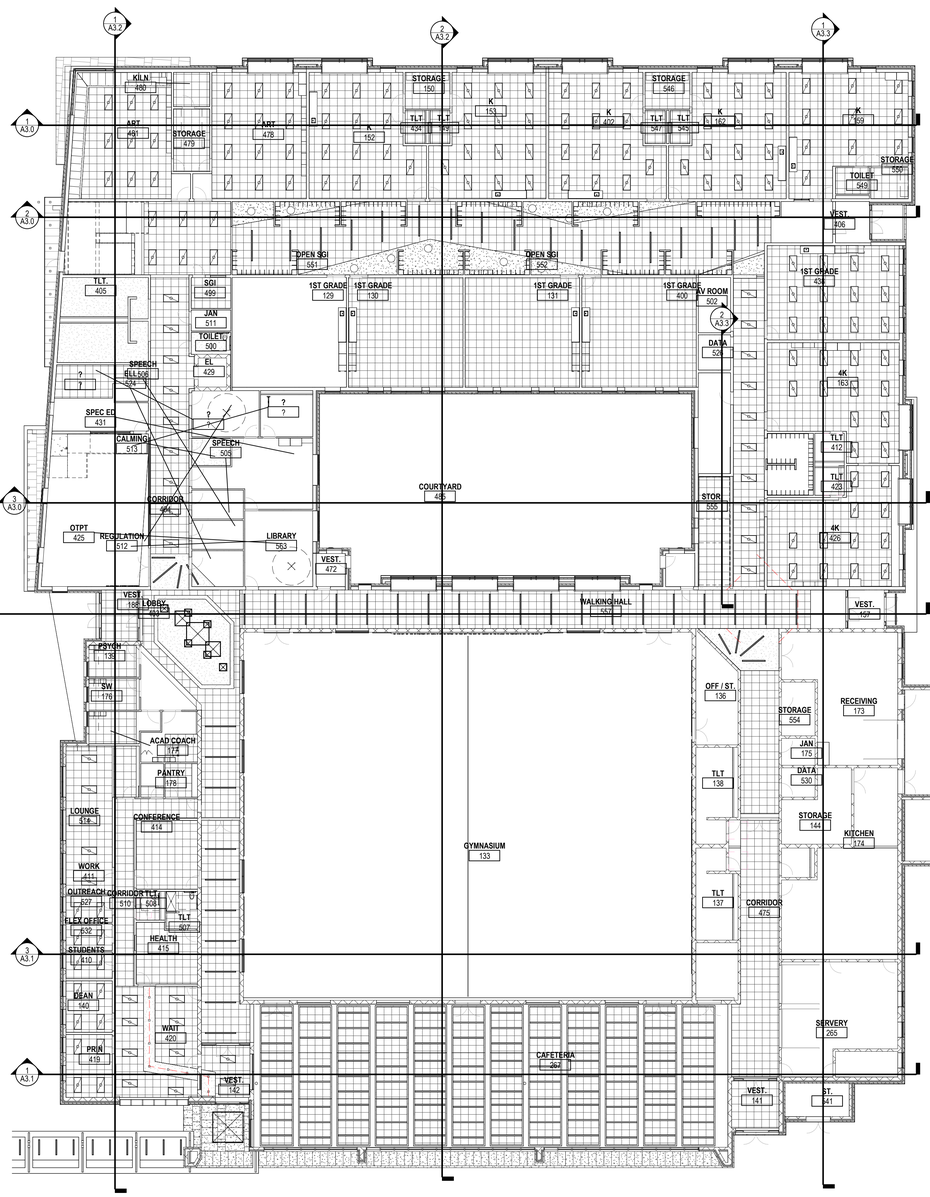
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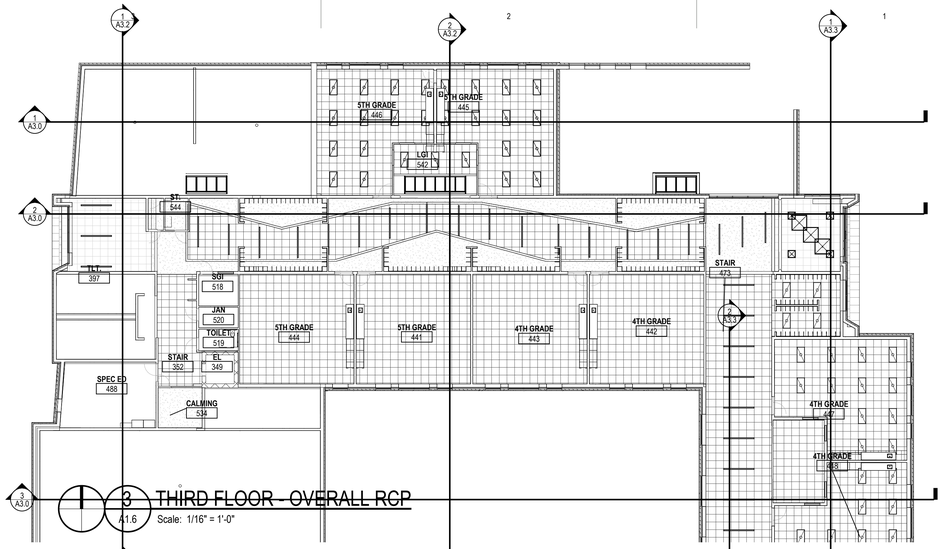
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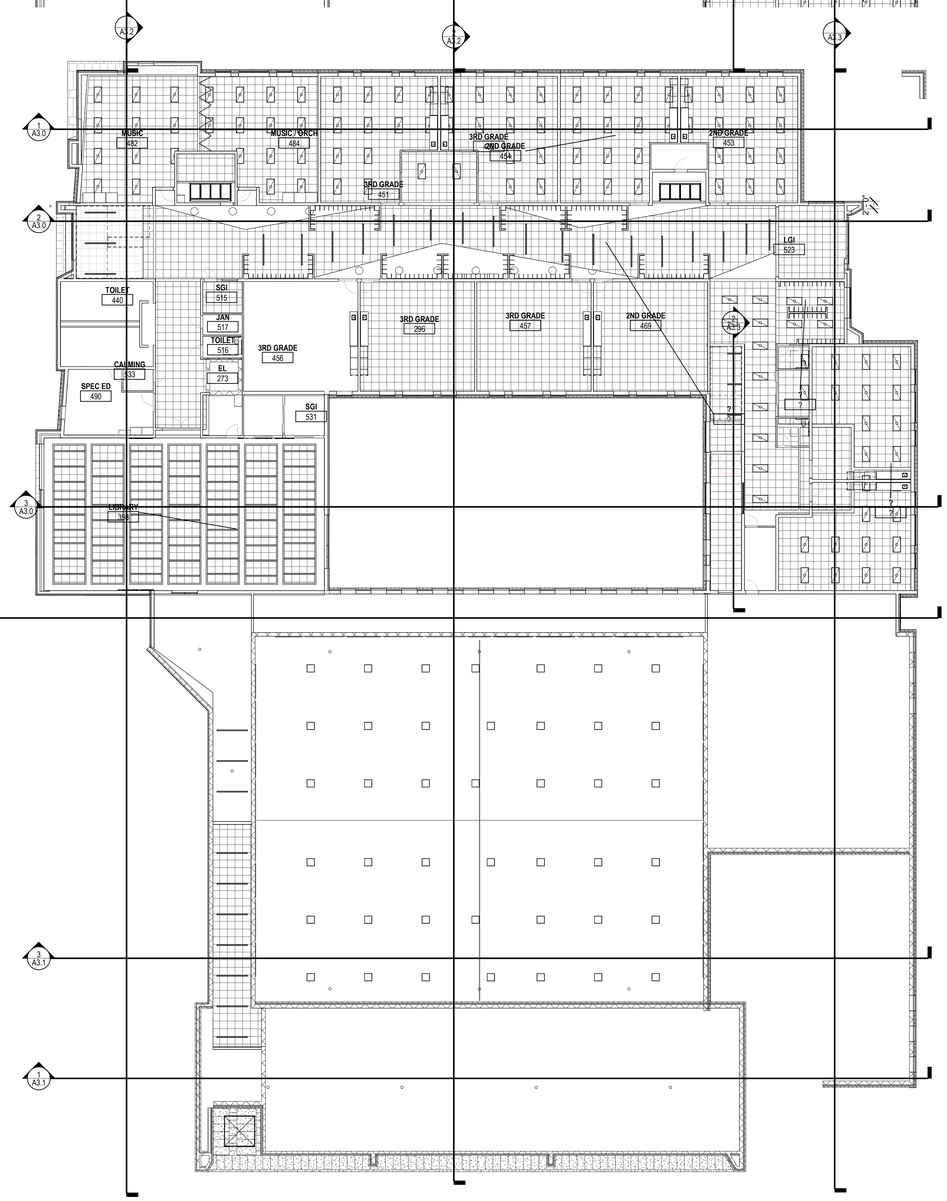
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1 A1.6 FIRST FLOOR - OVERALL RCP
Scale: 1/16" = 1'-0"




3 A1.6 THIRD FLOOR - OVERALL RCP
Scale: 1/16" = 1'-0"




2 A1.6 SECOND FLOOR - OVERALL RCP
Scale: 1/16" = 1'-0"

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# GENERAL CEILING PLAN NOTES

DELETE IF ALL-NEW BUILDING

EXISTING CEILING TILE & GRID

EXISTING GYP. BOARD CEILING

NEW CEILING TILE & GRID

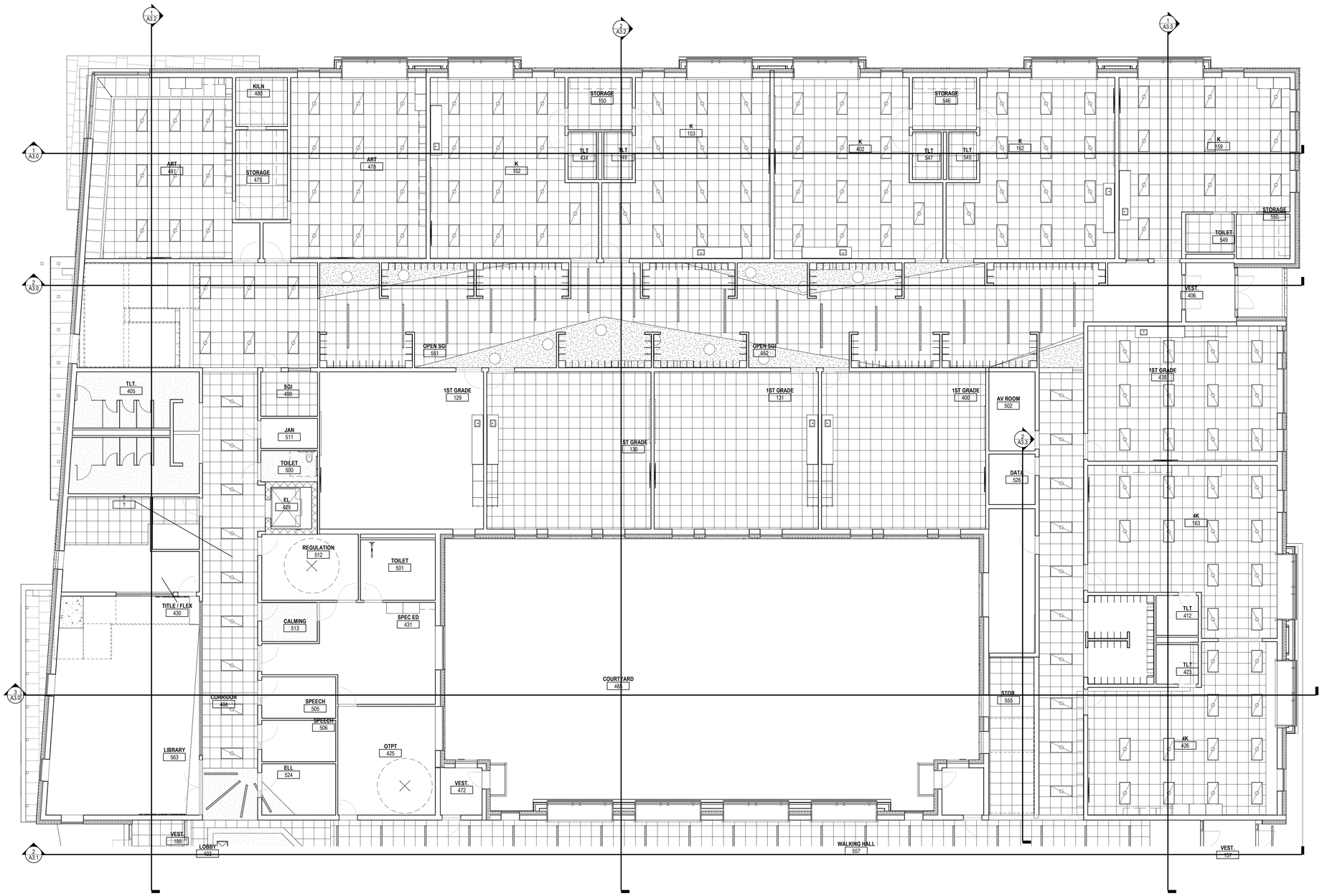
NEW GYP. BOARD CEILING

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2. ALL GYPSUM BOARD CEILINGS AND SOFFITS TO BE PAINTED PT-# UNLESS NOTED OTHERWISE. WHERE A PAINT COLOR IS INDICATED, PAINT THE BOTTOM AND FACE OF THE SOFFIT THE COLOR INDICATED.
3. COORDINATE WITH MECHANICAL, PLUMBING, FIRE PROTECTION AND ELECTRICAL THE EXACT LOCATIONS OF CEILING FIXTURES, DIFFUSERS, SPRINKLER HEADS, ETC. CONSULT WITH ARCHITECT REGARDING ANY DISCREPANCIES.

DELETE IF ALL-NEW BUILDING

4. REMOVE EXISTING CEILING PRIOR TO INSTALLING ANY NEW CEILING IF NOT NOTED ON THE DEMOLITION PLANS. ENSURE ALL CEILINGS FOR DEMOLITION ARE SHOWN ON DEMO RCP.
5. RECESSED LIGHT FIXTURES, SPEAKERS, EXIT LIGHTS, SMOKE DETECTORS, ETC. TO BE LOCATED IN THE CENTER OF CEILING TILES.
6. SEE ELECTRICAL AND MECHANICAL DRAWINGS FOR CEILING MOUNTED DEVICE SYMBOL LEGEND.
7. ALL CEILING TILE TO BE CENTERED IN BOTH DIRECTIONS IN EACH ROOM OR SPACE, UNLESS NOTED OTHERWISE. TYPICAL CEILING TILE SHOULD NOT BE CUT LESS THAN 6" WIDE IN ANY AREA. WHEN GRID LAYOUT RESULTS IN A CEILING TILE OF LESS THAN 4" FOR THE LENGTH OR WIDTH OF A ROOM, OMIT RUNNER NEAREST WALL AND CUT FULL TILE TO WIDTH AS NEEDED. ENSURE ALL CEILINGS ARE CENTERED IN ROOM. AVOID "SLIVERS" AND FULL TILES AT THE PERIMETER.

# KEYNOTE LEGEND




1 A1.6A **UNIT A - FIRST FLOOR RCP**

Scale: 1/8" = 1'-0"

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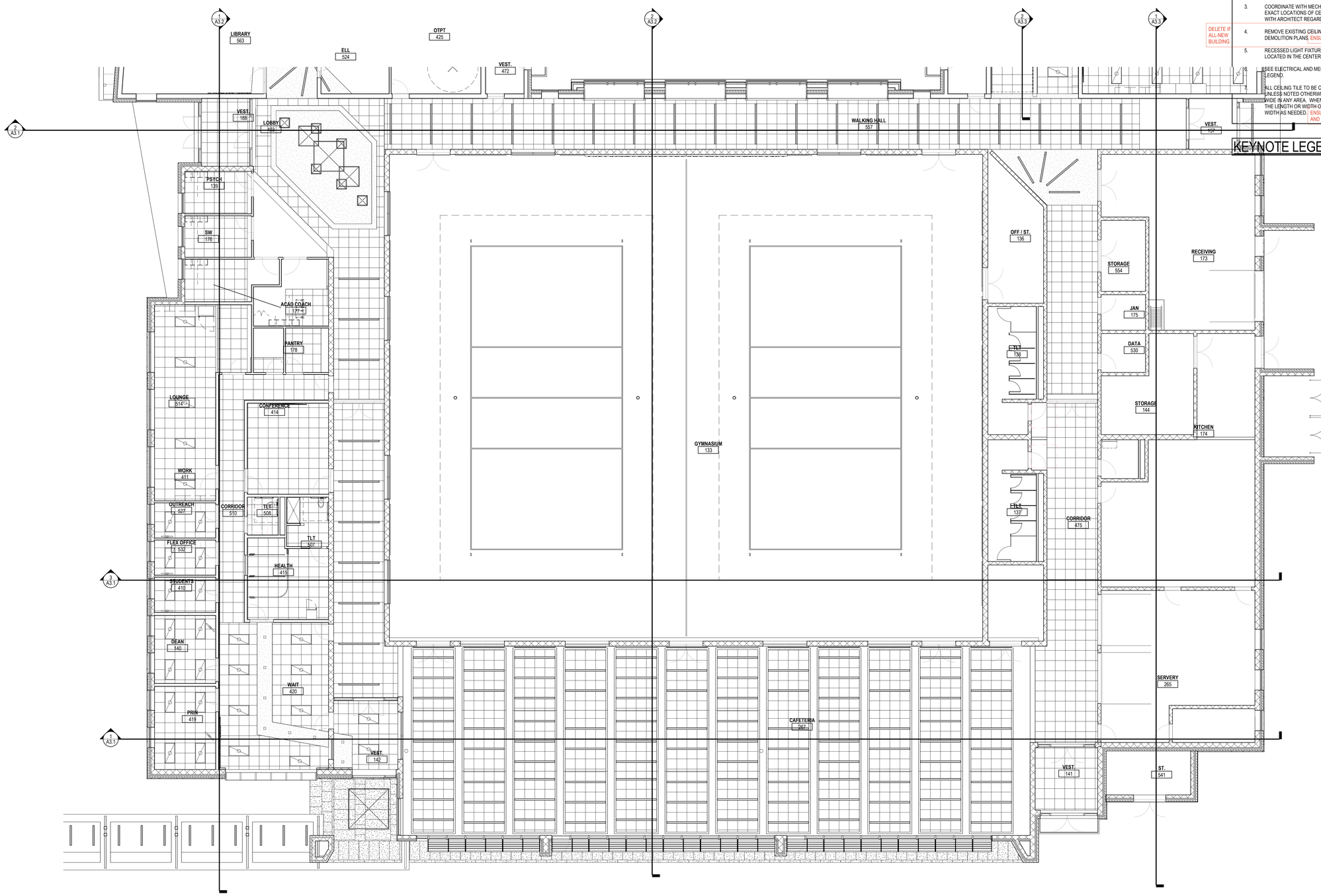
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**UNIT A - FIRST FLOOR RCP**

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## GENERAL CEILING PLAN NOTES

DELETE IF ALL-NEW BUILDING

EXISTING CEILING TILE & GRID

EXISTING GYP. BOARD CEILING

NEW CEILING TILE & GRID

NEW GYP. BOARD CEILING

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3. COORDINATE WITH MECHANICAL, PLUMBING, FIRE PROTECTION AND ELECTRICAL THE EXACT LOCATIONS OF CEILING FIXTURES, DIFFUSERS, SPRINKLER HEADS, ETC. CONSULT WITH ARCHITECT REGARDING ANY DISCREPANCIES.

DELETE IF ALL-NEW BUILDING

4. REMOVE EXISTING CEILING PRIOR TO INSTALLING ANY NEW CEILING IF NOT NOTED ON THE DEMOLITION PLANS. ENSURE ALL CEILINGS FOR DEMOLITION ARE SHOWN ON DEMO RCP.
5. RECESSED LIGHT FIXTURES, SPEAKERS, EXIT LIGHTS, SMOKE DETECTORS, ETC. TO BE LOCATED IN THE CENTER OF CEILING TILES.
6. SEE ELECTRICAL AND MECHANICAL DRAWINGS FOR CEILING MOUNTED DEVICE SYMBOL LEGEND.
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## KEYNOTE LEGEND

1 A1.6B UNIT B - FIRST FLOOR RCP
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# GENERAL CEILING PLAN NOTES

DELETE IF ALL-NEW BUILDING

EXISTING CEILING TILE & GRID

EXISTING GYP. BOARD CEILING

NEW CEILING TILE & GRID

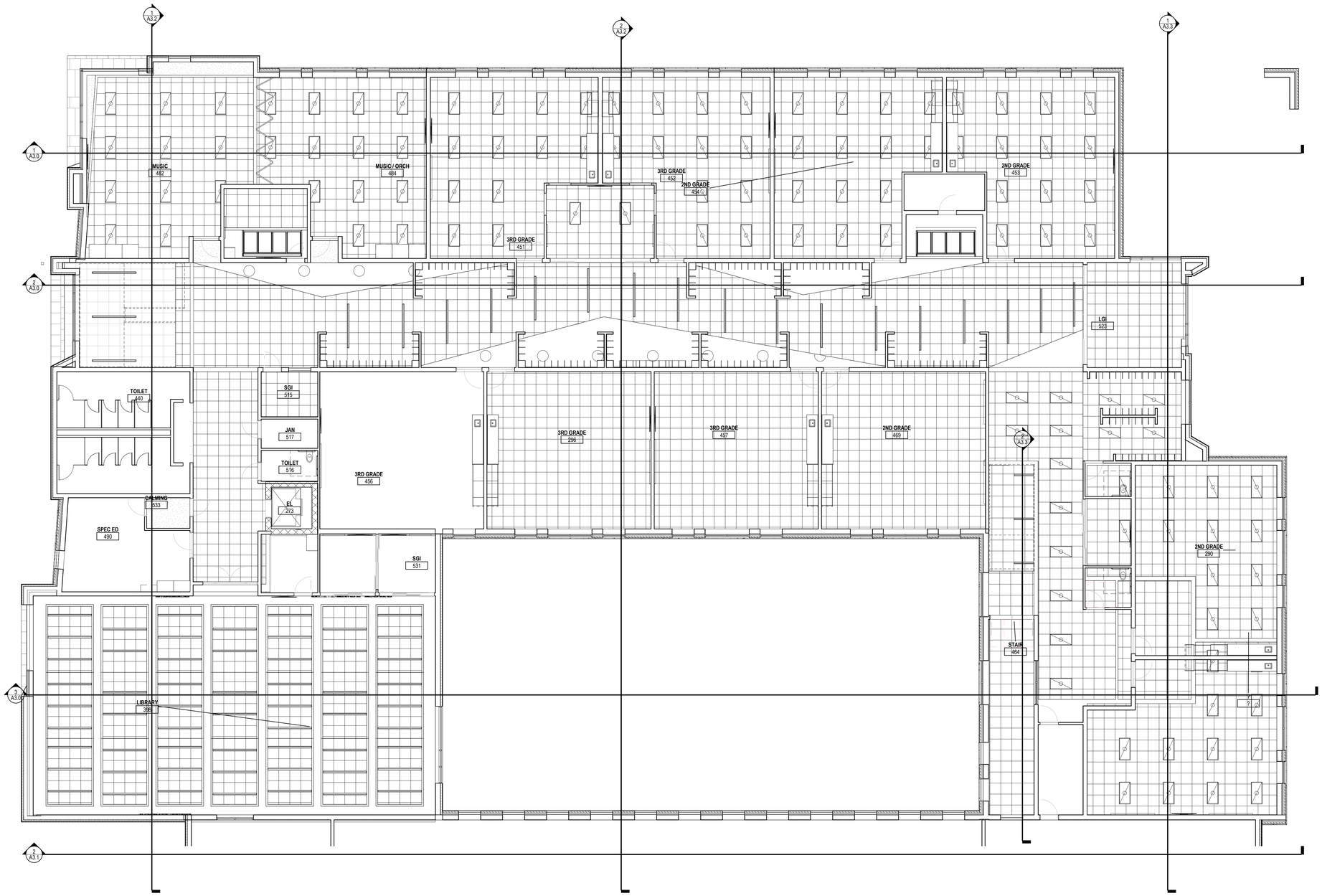
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DELETE IF ALL-NEW BUILDING

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## KEYNOTE LEGEND




1 A1.7A UNIT A - SECOND FLOOR RCP

Scale: 1/8" = 1'-0"


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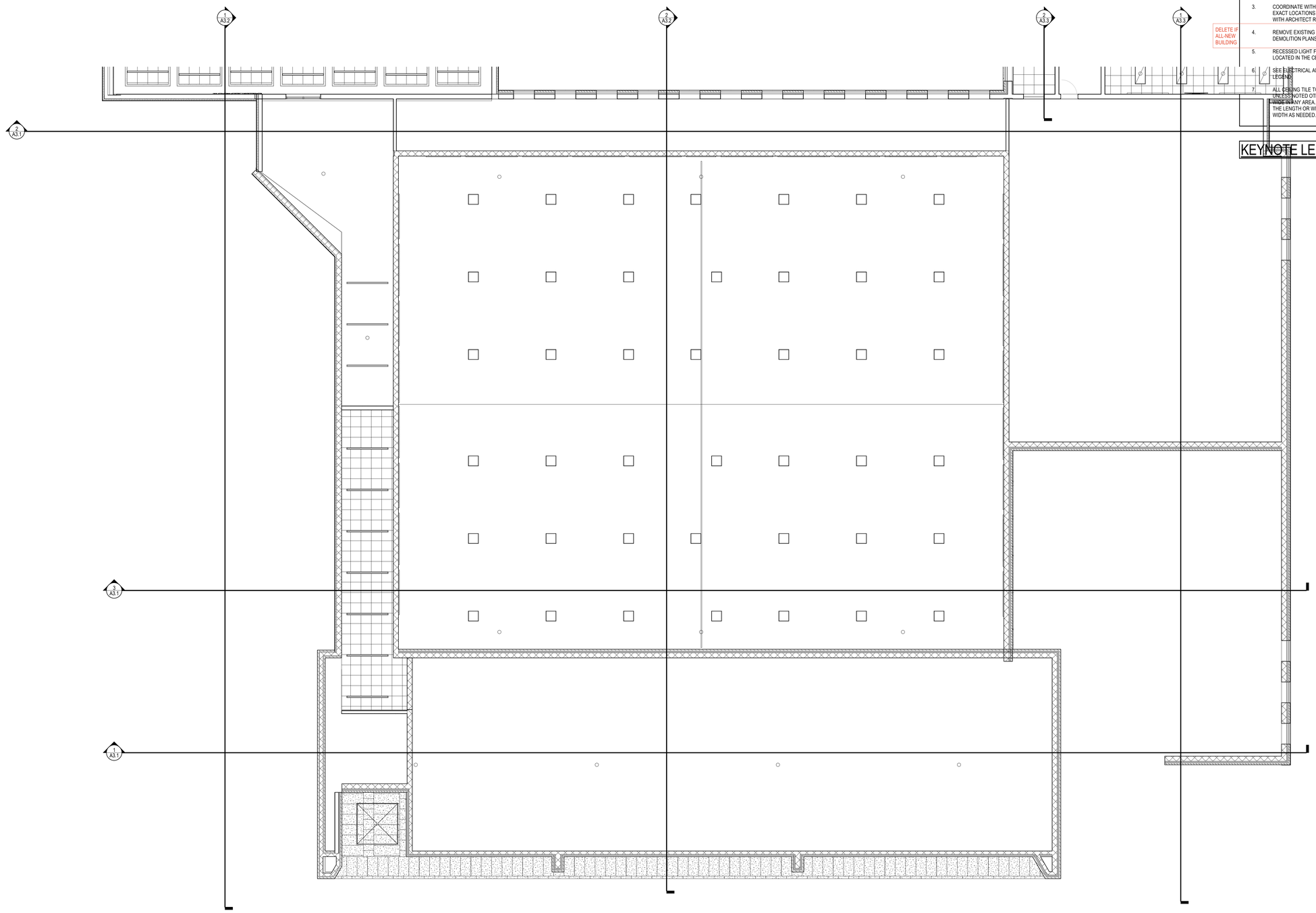
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## GENERAL CEILING PLAN NOTES

DELETE IF ALL-NEW BUILDING

EXISTING CEILING TILE & GRID

EXISTING GYP. BOARD CEILING

NEW CEILING TILE & GRID

NEW GYP. BOARD CEILING

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## KEYNOTE LEGEND

1 A1.7B **UNIT B - SECOND FLOOR RCP**
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# GENERAL CEILING PLAN NOTES

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EXISTING GYP. BOARD CEILING

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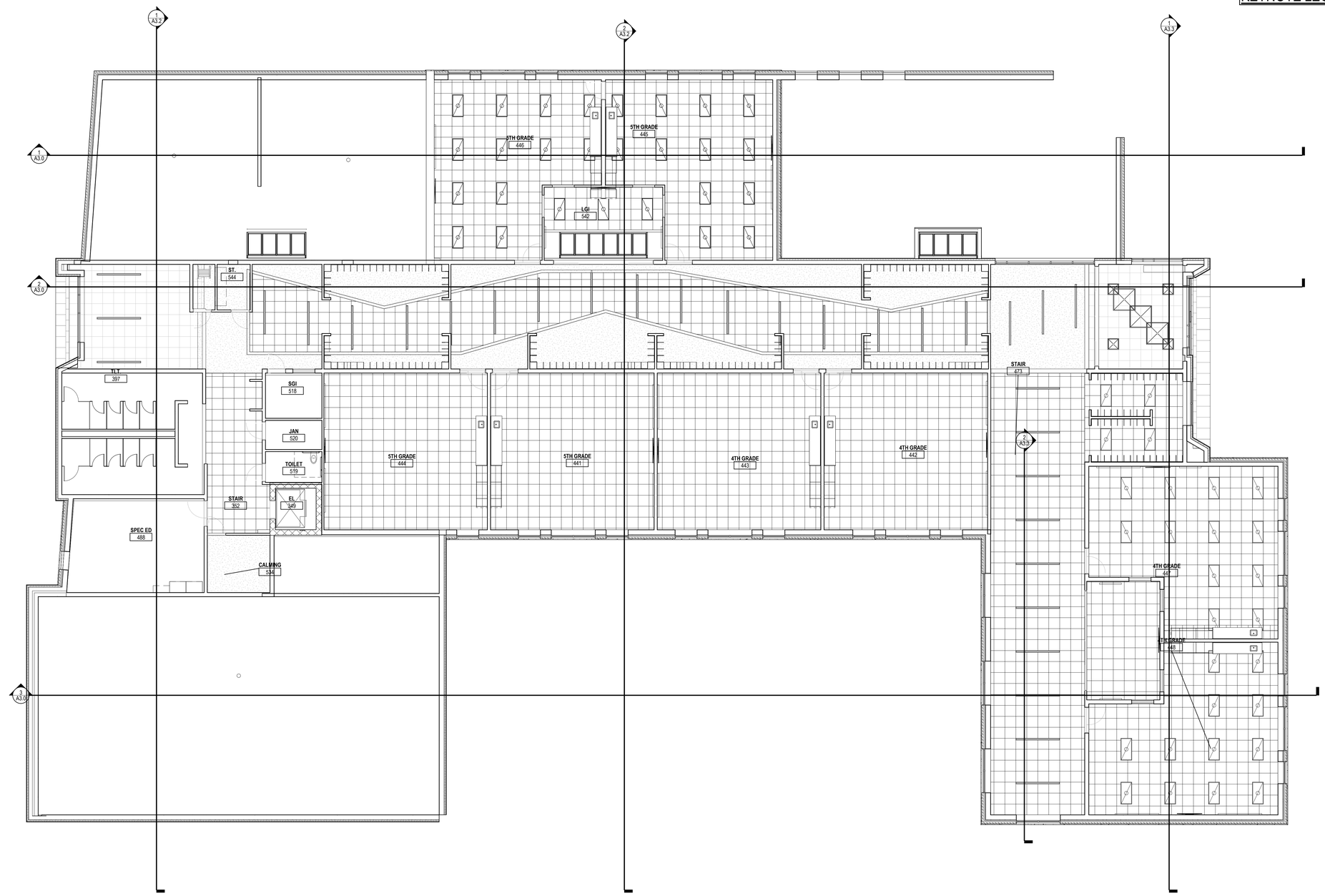
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# KEYNOTE LEGEND




1 A1.8A UNIT A - THIRD FLOOR RCP

Scale: 1/8" = 1'-0"


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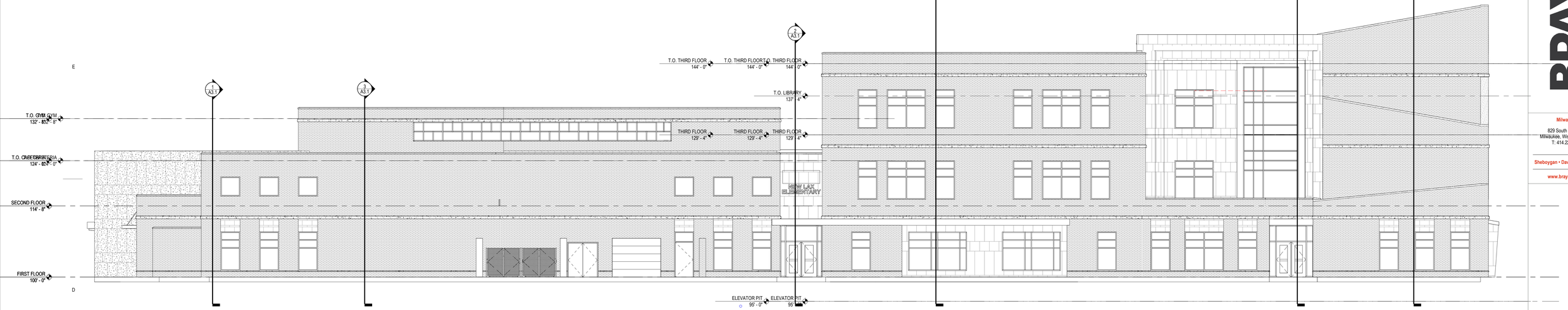
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Sheet Number: A1.8A

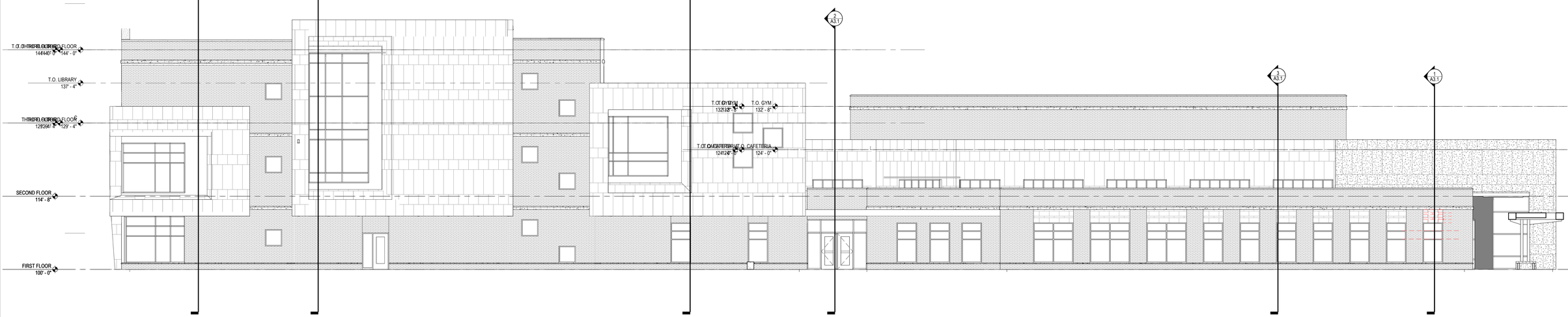
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## KEYNOTE LEGEND




1 A2.0 **BUILDING ELEVATION - EAST**
Scale: 1/8" = 1'-0"




2 A2.0 **BUILDING ELEVATION - NORTH**
Scale: 1/8" = 1'-0"





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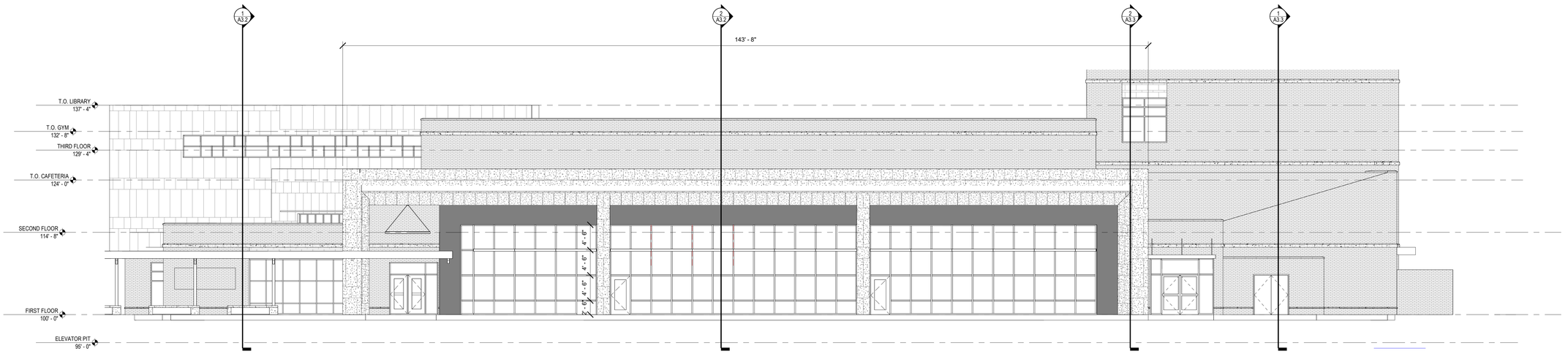
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**OVERALL EXTERIOR ELEVATIONS**

Sheet Number:
**A2.0**

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KEYNOTE LEGEND




1 A2.1 BUILDING ELEVATION - SOUTH
Scale: 1/8" = 1'-0"

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Sheet Title:
OVERALL EXTERIOR ELEVATIONS

Sheet Number:
A2.1

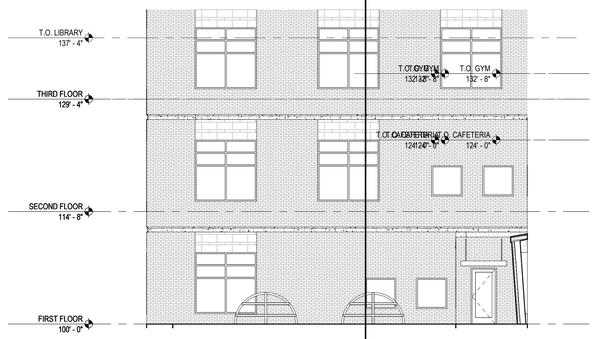
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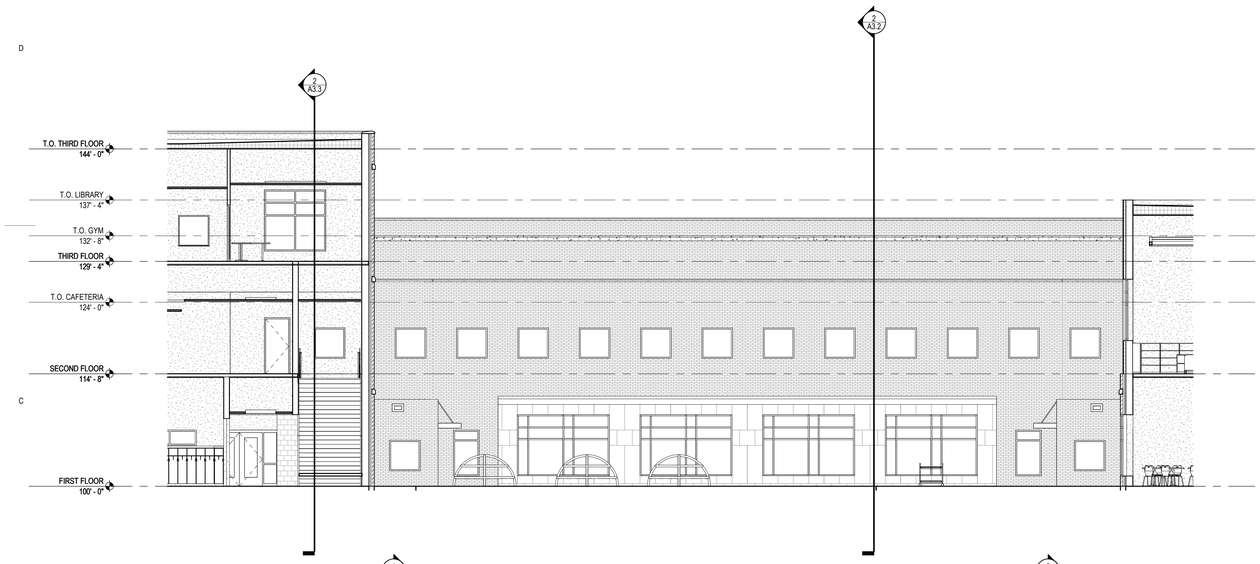

1 A2.2 BUILDING ELEVATION - COURTYARD EAST

Scale: 1/8" = 1'-0"



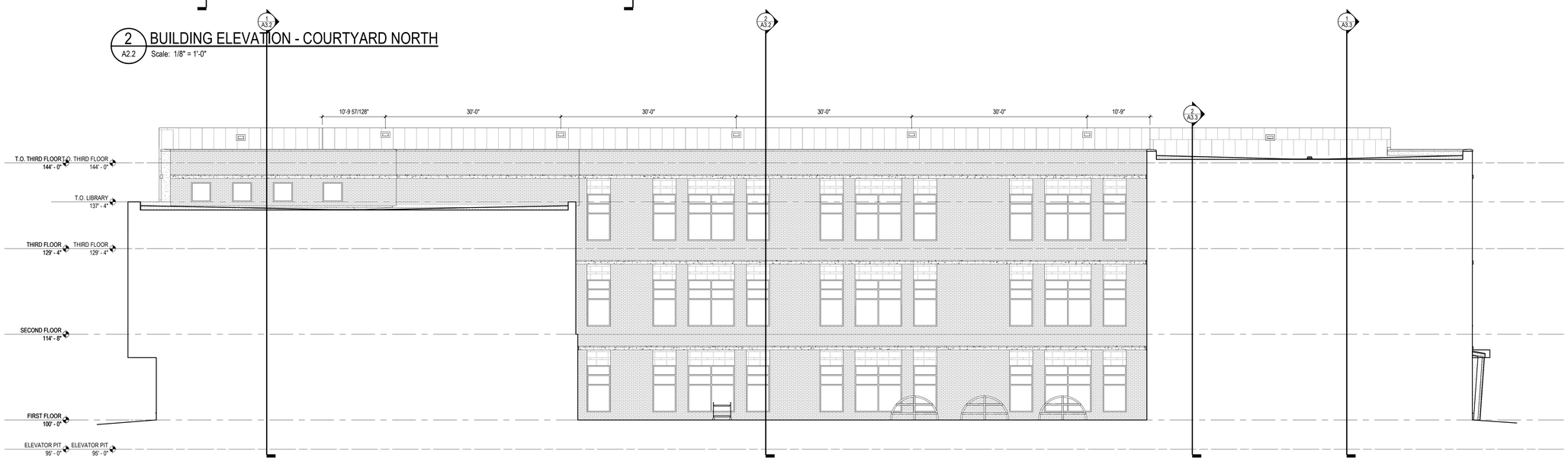

4 A2.2 BUILDING ELEVATION - COURTYARD WEST

Scale: 1/8" = 1'-0"




2 A2.2 BUILDING ELEVATION - COURTYARD NORTH

Scale: 1/8" = 1'-0"




3 A2.2 BUILDING ELEVATION - COURTYARD SOUTH

Scale: 1/8" = 1'-0"

## KEYNOTE LEGEND


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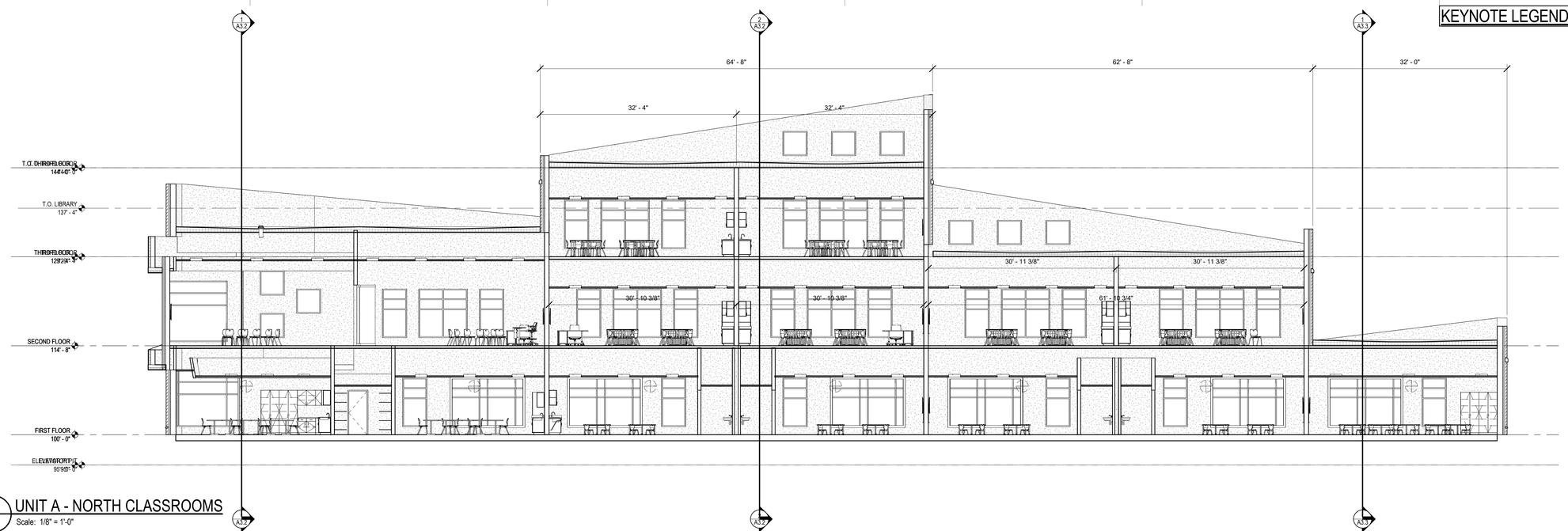
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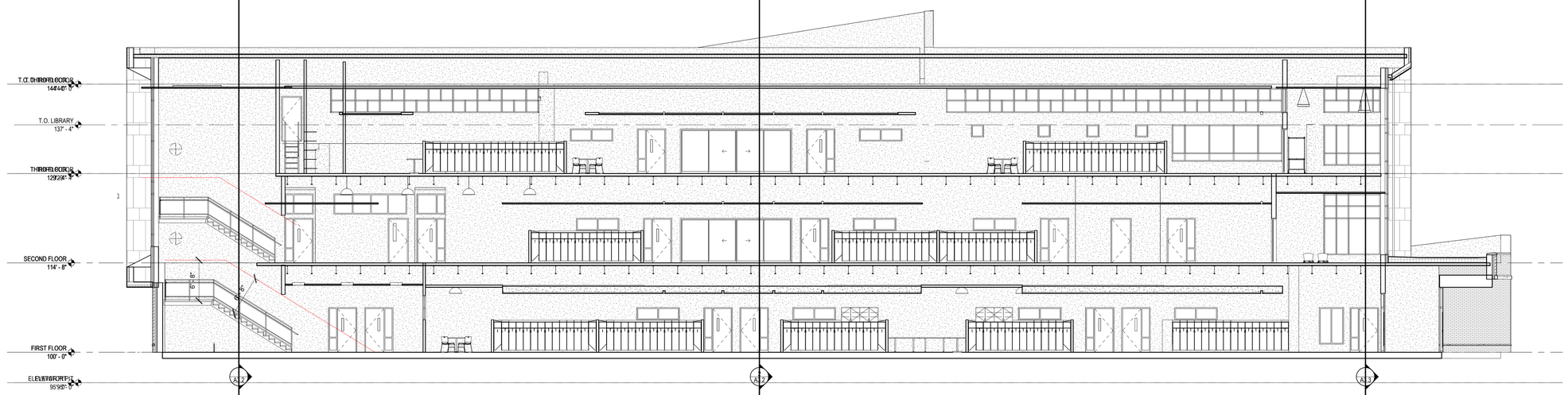
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## KEYNOTE LEGEND




1 A3.0 **UNIT A - NORTH CLASSROOMS**
Scale: 1/8" = 1'-0"




2 A3.0 **UNIT A - COLLAB**
Scale: 1/8" = 1'-0"




3 A3.0 **UNIT A - COURTYARD**
Scale: 1/8" = 1'-0"


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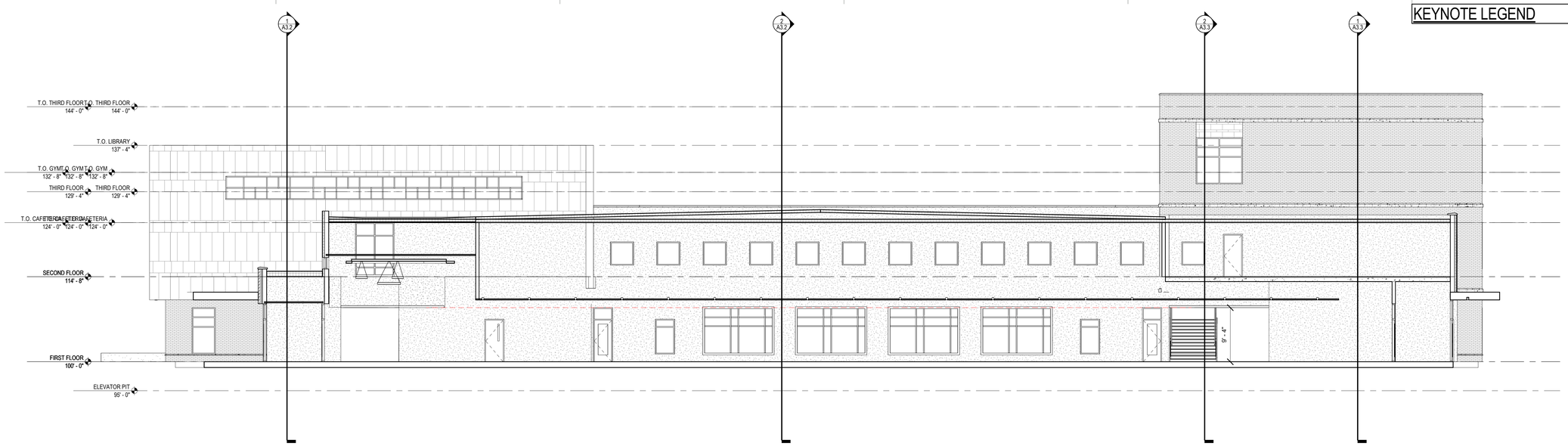
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**BUILDING SECTIONS**

Sheet Number:
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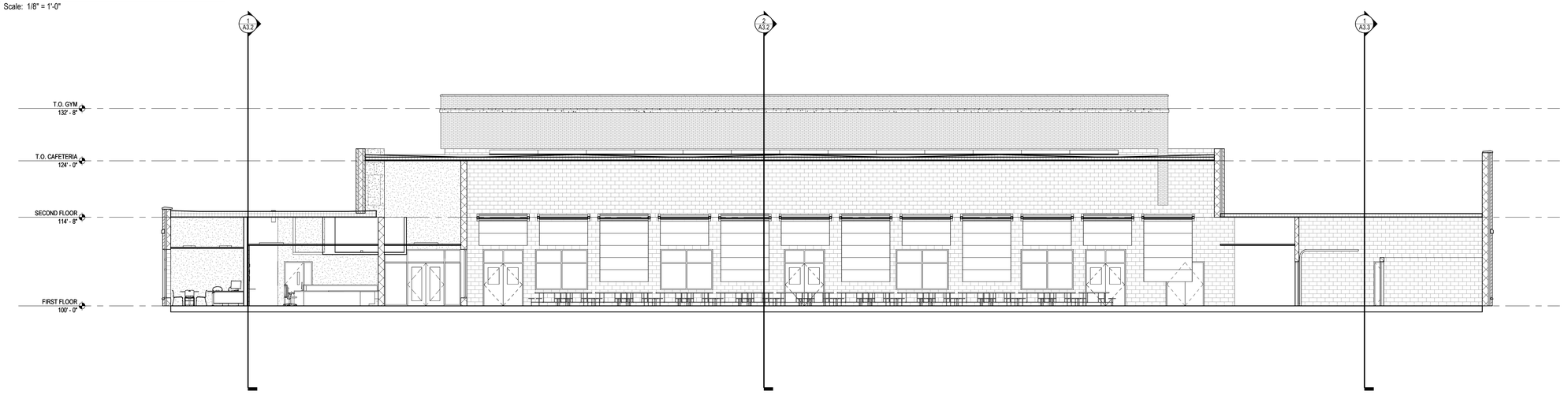
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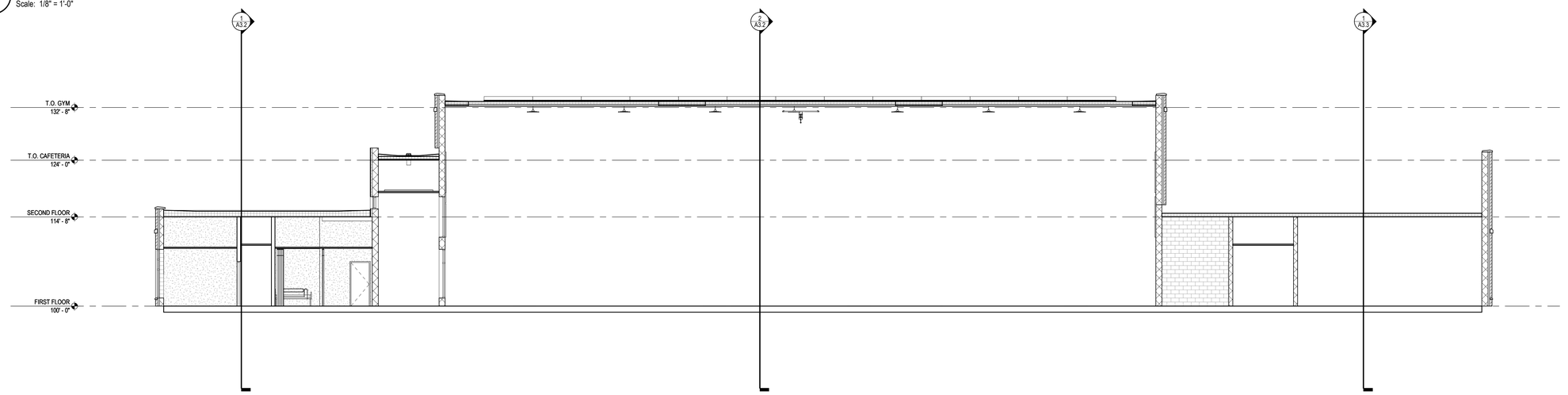
## KEYNOTE LEGEND




### 2 UNIT B - COURTYARD CORRIDOR
A3.1 Scale: 1/8" = 1'-0"




### 1 UNIT B - CAFETERIA
A3.1 Scale: 1/8" = 1'-0"




### 3 UNIT B - GYMNASIUM
A3.1 Scale: 1/8" = 1'-0"

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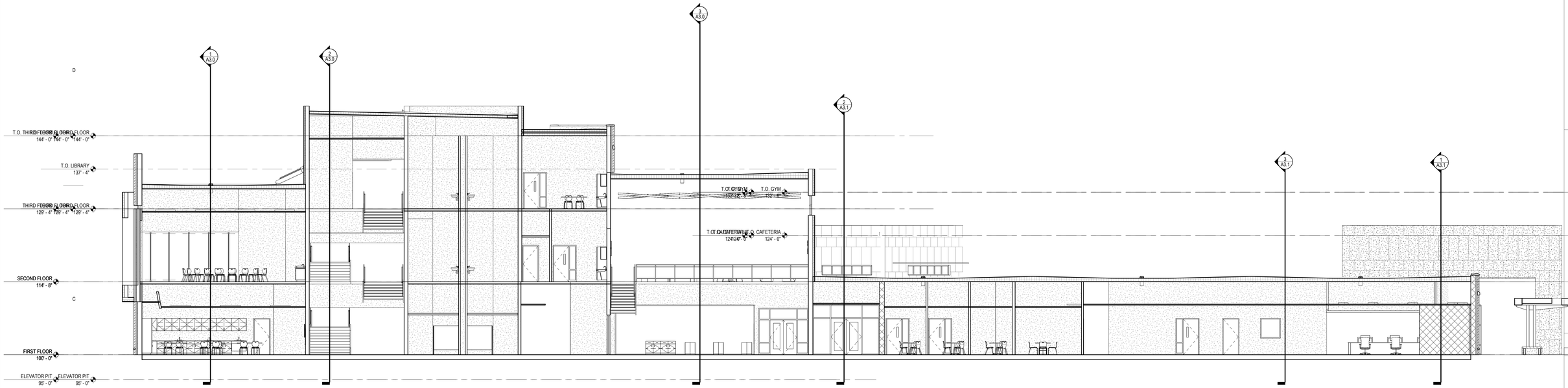
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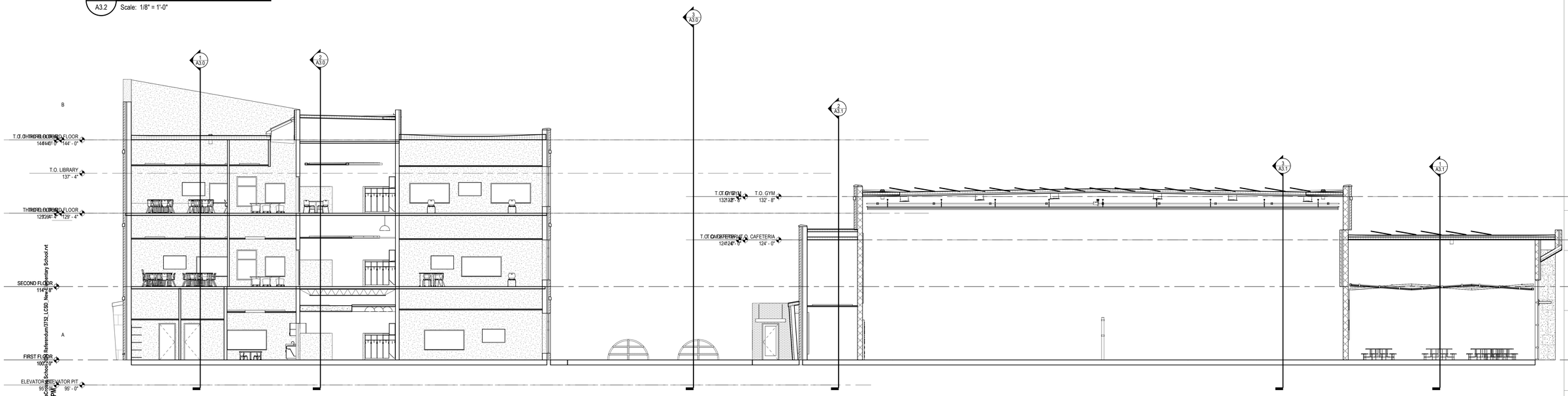
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## KEYNOTE LEGEND




1 A3.2 **UNIT A/B - MAIN CORRIDOR**
Scale: 1/8" = 1'-0"




2 A3.2 **UNIT A/B - COURTYARD**
Scale: 1/8" = 1'-0"

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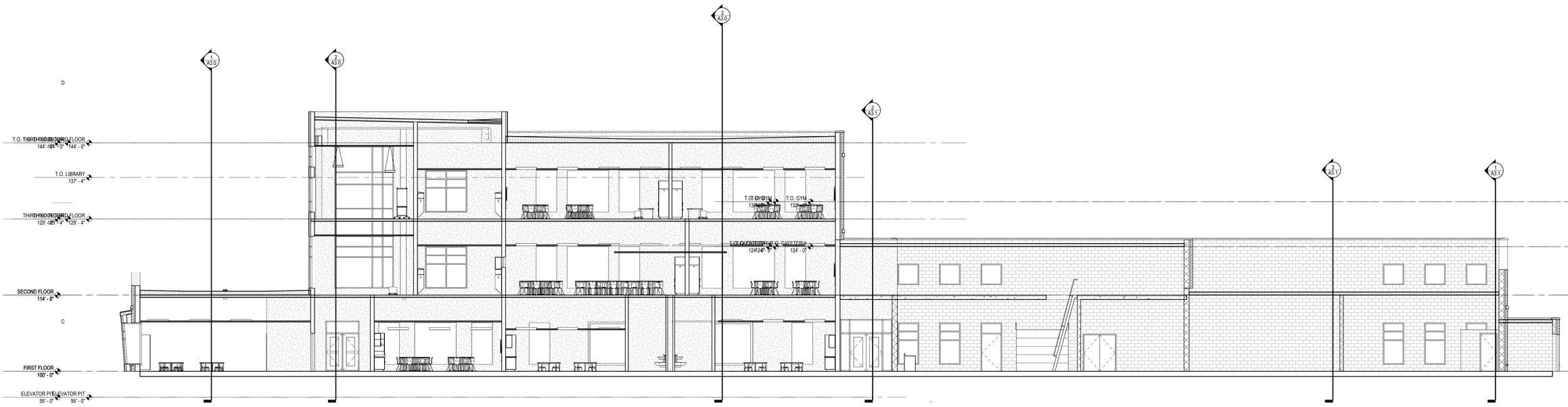
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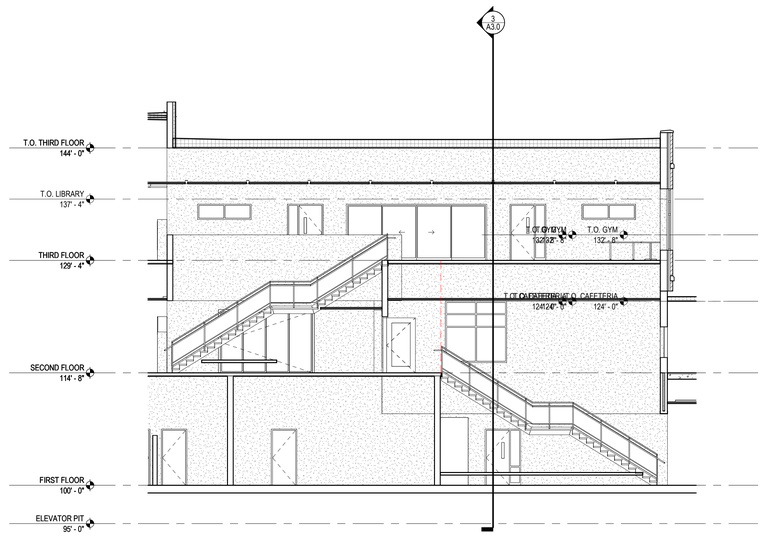
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## KEYNOTE LEGEND




1 A3.3 **UNIT A/B - EAST CLASSROOMS**
Scale: 1/8" = 1'-0"




2 A3.3 **UNIT A/B - STAIR**
Scale: 1/8" = 1'-0"

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New Building for

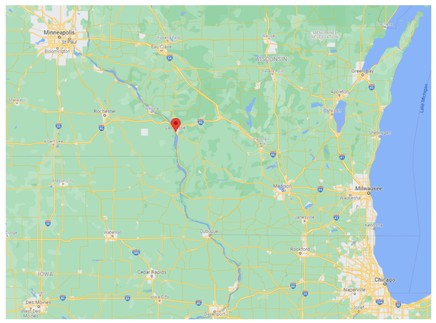
# New Elementary School

La Crosse School District
807 East Ave S
La Crosse, WI 54601

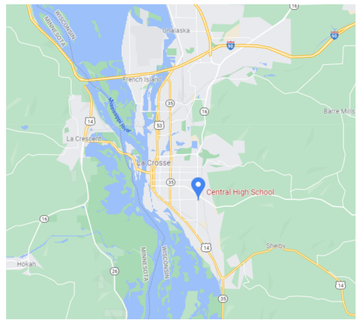
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## 05/06/25

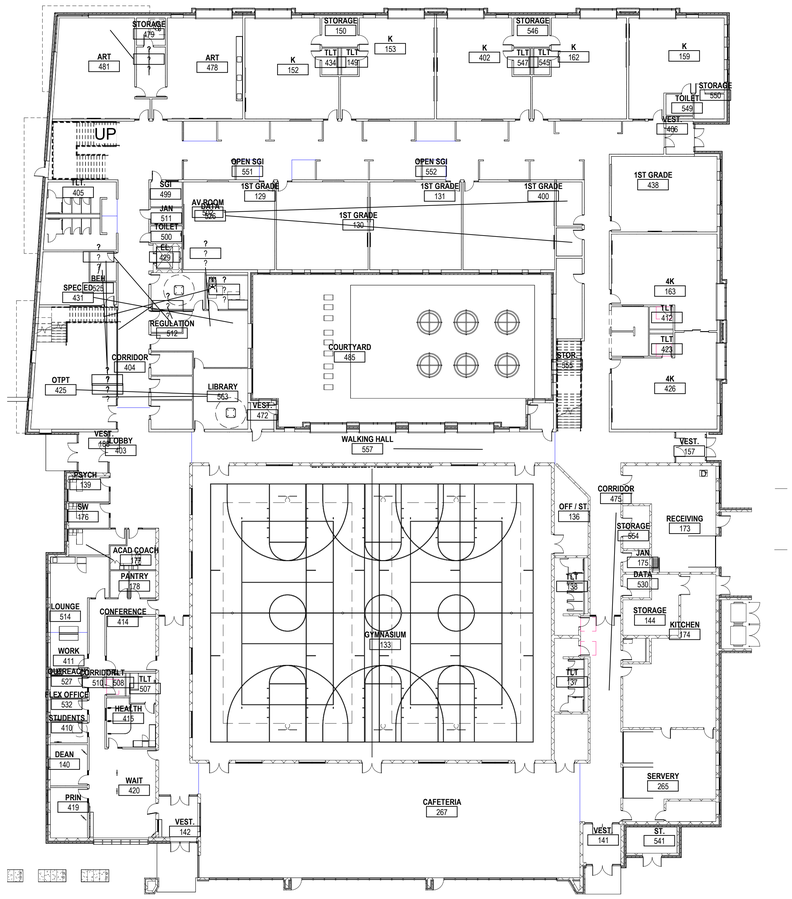
SCHOOL DISTRICT OF LA CROSSE

REGIONAL MAP

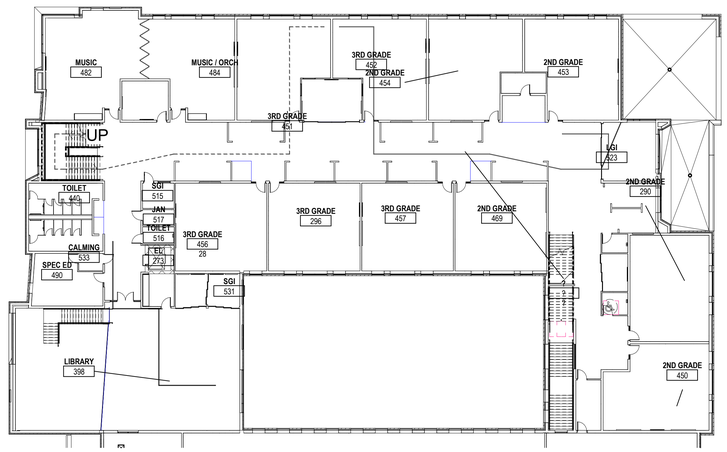



LOCAL MAP

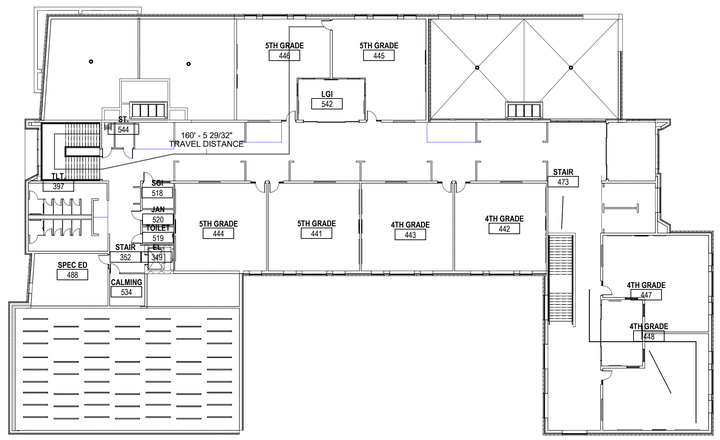

Onalaska
French Island
Barre Mills
La Crescent
La Crosse
Central High School
Holmen
Shelby
Mississippi River
Black River
Root River
Upper Iowa River
Wisconsin River
Kickapoo River
Coulee Region
Driftless Area
Great River Road
Great River Bluffs State Park
Great River Trail
Great River Bluffs Trail
Great River Bluffs Scenic Byway
Great River Bluffs Wildlife Area
Great River Bluffs Wildlife Management Area
Great River Bluffs Wildlife Refuge
Great River Bluffs Wildlife Sanctuary
Great River Bluffs Wildlife Preserve
Great River Bluffs Wildlife Reserve
Great River Bluffs Wildlife Park
Great River Bluffs Wildlife Zoo
Great River Bluffs Wildlife Aquarium
Great River Bluffs Wildlife Museum
Great River Bluffs Wildlife Center
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2 G0.1 FIRST FLOOR - CODE PLAN
Scale: 1" = 20'-0"




3 G0.1 SECOND FLOOR - CODE PLAN
Scale: 1" = 20'-0"

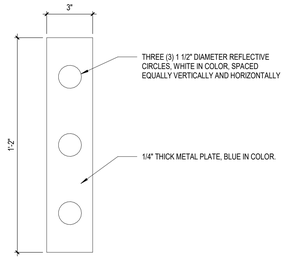



4 G0.1 THIRD FLOOR - CODE PLAN
Scale: 1" = 20'-0"

# FIRE RESISTIVE & CODE REVIEW LEGEND

**NOTE: THIS SHEET TO BE PRINTED IN COLOR**

| | | | | | | |
|---|---|---|---|---|---|---|
| | WORK IN EXISTING BUILDING | X X X | SMOKE TIGHT PARTITION TO DECK | SE1 SE1 SE1 | SHAFT ENCLOSURE 1 HOUR |
| | BUILDING ADDITION | CX CX CX | SMOKE TIGHT PARTITION TO INTERSTITIAL CEILING | SE2 SE2 SE2 | SHAFT ENCLOSURE 2 HOUR |
| | | PX PX PX | SMOKE TIGHT PARTITION TO PLENUM CEILING | SE3 SE3 SE3 | SHAFT ENCLOSURE 3 HOUR |
| ROOM NAME 101 45 | ROOM NAME<br>ROOM NUMBER<br>ROOM OCCUPANT LOAD | | | SE4 SE4 SE4 | SHAFT ENCLOSURE 4 HOUR |
| FE | FIRE EXTINGUISHER | SB1 SB1 SB1 | SMOKE BARRIER 1 HOUR | FB1 FB1 FB1 | FIRE BARRIER 1 HOUR |
| 27" R<br>32" P | EGRESS DIRECTION<br>EGRESS WIDTH REQUIRED<br>EGRESS WIDTH PROVIDE | SB2 SB2 SB2 | SMOKE BARRIER 2 HOUR | FB2 FB2 FB2 | FIRE BARRIER 2 HOUR |
| A 36" DOOR WILL PROVIDE 32-3/4" (+/-) OF EGRESS WIDTH | | SB3 SB3 SB3 | SMOKE BARRIER 3 HOUR | FB3 FB3 FB3 | FIRE BARRIER 3 HOUR |
| | | SB4 SB4 SB4 | SMOKE BARRIER 4 HOUR | FB4 FB4 FB4 | FIRE BARRIER 4 HOUR |
| | | FP0.5 FP0.5 FP0.5 | FIRE PARTITION 0.5 HOUR | FW1 FW1 FW1 | FIRE WALL 1 HOUR |
| | | FP1 FP1 FP1 | FIRE PARTITION 1 HOUR | FW2 FW2 FW2 | FIRE WALL 2 HOUR |
| | | | | FW3 FW3 FW3 | FIRE WALL 3 HOUR |
| | | 2HR 2HR 2HR | RATED CONSTRUCTION 2 HOUR | FW4 FW4 FW4 | FIRE WALL 4 HOUR |




1 G0.1 EXTERIOR FIRE WALL SIGN
Scale: 3" = 1'-0"

# PROJECT DATA

| | |
|---|---|
| SITE ADDRESS: | STREET ADDRESS<br>CITY, WI 55555 |
| COUNTY: | COUNTY NAME |
| APPLICABLE CODES: | 2015 INTERNATIONAL BUILDING CODE<br>WISCONSIN COMMERCIAL BUILDING CODE (SPS 361-366) |

BUILDING #A (EXISTING)
PORTION OF EXISTING BUILDING ADJACENT TO NEW CONSTRUCTION BUILT IN 1999
PRIMARY OCCUPANCY = E
CONSTRUCTION TYPE = IIB
NUMBER OF STORIES = ONE STORY + PENTHOUSE & MEZZANINE
NON-SPRINKLERED BUILDING

BUILDING AREA:
EXISTING 1ST FLOOR BUILDING AREA = ~128,300 SF
RENOVATED AREA = ~450 SF
<50% WORK AREA = LEVEL 2 ALTERATION
NO CHANGE IN OCCUPANCY
EXISTING EGRESS WIDTH MAINTAINED AT NEW DOOR OPENINGS
RENOVATION IS UNDER EXISTING CANOPY (NO INCREASE IN BUILDING AREA)

BUILDING #B (ADDITION)
BUILDING RENOVATION & ADDITION
PRIMARY OCCUPANCY = E
NON-SEPARATED OCCUPANCY = A-2 (CAFETERIA) PER IBC 303.1.3
CONSTRUCTION TYPE = IIB
NUMBER OF STORIES = ONE STORY
FULLY SPRINKLERED BUILDING

FIRE SEPARATION:
2-HR FIRE WALL (HORIZONTAL EXIT) BETWEEN BUILDING A & BUILDING B PER TABLE 706.4

BUILDING AREA:
MAXIMUM BUILDING HEIGHT PER TABLE 504.3 = 75' W/ SPRINKLER SYSTEM
MAXIMUM NUMBER OF STORIES PER TABLE 504.4 = 3 STORIES W/ SPRINKLER SYSTEM
ALLOWABLE AREA PER TABLE 506.2 = 43,500 SF MULTISTORY & SPRINKLERED
FRONTAGE INCREASE:
TOTAL PERIMETER = 1,590 LF
BUILDING PERIMETER ON PUBLIC WAY (30' OPEN SPACE) = 1,339 LF
CALCULATED FRONTAGE AREA INCREASE (84.2%-25%) = 8,586 SF
TOTAL ALLOWABLE BUILDING AREA = 66,586 SF
ACTUAL BUILDING AREA = 48,242 SF

OCCUPANT LOAD:
PER TYPE:
E OOC = 780 OCCUPANTS
A-2 OOC = 392 OCCUPANTS
TOTAL OCCUPANCY = 1,172 OCCUPANTS

EGRESS INFO:
1005.1 - MEANS OF EGRESS SIZING
EGRESS DOOR WIDTH REQUIRED (IBC 1005.3.2 EX #1) = 1,149 X .15 = 172.4"
EGRESS DOOR WIDTH PROVIDED = 495"
1006.3.1 - MIN. OF (4) EXITS >1000 OCC.
1014.3 - COMMON PATH OF TRAVEL = 75' MAX
1017.2 - EXIT ACCESS TRAVEL DISTANCE = 250' MAX W/ SPRINKLER
1020.1 - CORRIDOR RATING = 0 HR WITH SPRINKLER
1020.2 - CORRIDOR WIDTH = 72" MIN SERVING >100 OCC.
1020.4 EX#2 - DEAD END CORRIDOR = 50' MAX WITH SPRINKLER

MODIFY AS NECESSARY

**SANITARY FIXTURE CALCULATIONS**

SCENARIO #1: RESTROOMS FOR TYPICAL SCHOOL DAY BASED ON MAXIMUM EXPECTED STUDENT ENROLLMENT & CURRENT STAFF COUNTS* ('E' OCCUPANCY USE)
TOTAL 'E' OCCUPANTS: 578 (289 MEN & 289 WOMEN)

| | WATER CLOSETS | | LAVATORIES | | UNISEX RESTROOMS | DRINK FOUNTAINS |
|---|---|---|---|---|---|---|
| FORMULA | M (1:50) | W (1:50) | M (1:50) | W (1:50) | | 1:100 |
| REQUIRED | 5.8 | 5.8 | 5.8 | 5.8 | 0 | 5.8 |

SCENARIO #2: RESTROOMS FOR A GYMNASIUM AND STAGE EVENT ('A-3' OCCUPANCY USE)
TOTAL 'A-3' OCCUPANTS: 490 (245 MEN & 245 WOMEN)

| | WATER CLOSETS | | LAVATORIES | | UNISEX RESTROOMS | DRINK FOUNTAINS |
|---|---|---|---|---|---|---|
| FORMULA | M (1:125) | W (1:65) | M (1:200) | W (1:200) | | 1:500 |
| REQUIRED | 1.96 | 3.77 | 1.23 | 1.23 | 1 | .98 |

TOTAL FIXTURES PROVIDED

| | WATER CLOSETS | | LAVATORIES | | UNISEX RESTROOMS** | DRINK FOUNTAINS |
|---|---|---|---|---|---|---|
| | MEN**** | WOMEN | MEN | WOMEN | | |
| PROVIDED | 12 U / 8 WC | 13 | 14 | 15 | 9** | 31*** |

*SPS 362.2902 (1) (e) ALLOWS THE REQUIRED TOILET FIXTURE COUNT TO BE BASED ON ACTUAL OCCUPANT LOAD RATHER THAN THE LOAD DETERMINED BY SQUARE FOOTAGE.

EXPECTED STUDENT ENROLLMENT: ~450-500 STUDENTS
CURRENT STAFF: 78 STAFF
TOTAL OCCUPANCY ~528-578 OCCUPANTS

SCENARIO #1 (FULL SCHOOL) AND SCENARIO #2 (GYMNASIUM EVENT) ARE NOT EXPECTED TO TAKE PLACE AT THE SAME TIME. BOTH SCENARIO #1 AND SCENARIO #2 CAN BE MEET BY PROVIDED FIXTURES.

** UNISEX TOILETS CAN BE USED TOWARD MENS OR WOMENS REQUIRED FIXTURE TOTAL (MEN'S AND WOMEN'S TOTAL FIXTURES SHOWN ABOVE DO NOT INCLUDE ANY UNISEX FIXTURES).

*** (23) WALL MOUNTED DRINK FOUNTAINS AND (8) DRINK FOUNTAINS PROVIDED AT CLASSROOM SINKS

**** SPS 362.2902 - 66% OF WATER CLOSETS (WC) CAN BE URINALS (U) IN BOTH 'E' AND 'A' OCCUPANCIES

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Project Title:
New Building for
New Elementary School
La Crosse School District
807 East Ave S
La Crosse, WI 54601

REVISIONS:

| # | DATE | DESCRIPTION |
|---|---|---|

NOT FOR CONSTRUCTION

Project Number: 3752

Issued For: 50%DD Set 05/06/25

Sheet Title: CODE REVIEW

Sheet Number: G0.1

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Autodesk Docs://3752 - LaCrosse School 2024 Referendum/3752_LCSD_New Elementary School.rvt
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# FIRE PROTECTION SYMBOLS AND ABBREVIATIONS

THIS IS A COMPREHENSIVE SYMBOL AND ABBREVIATION LIST - NOT ALL SYMBOLS ARE APPLICABLE TO THESE DRAWINGS.

## PIPING SYSTEMS

| | |
|---|---|
| FP-D | FIRE PROTECTION DRY |
| FP | FIRE PROTECTION OTHER |
| FP-P | FIRE PROTECTION PRE-ACTION |
| FP-W | FIRE PROTECTION WET |

## ACCESSORIES

- BACKFLOW PREVENTER - DC
- BACKFLOW PREVENTER - DCDA
- BACKFLOW PREVENTER - PVB
- BACKFLOW PREVENTER - RP
- BALL VALVE
- BUTTERFLY VALVE
- CHECK VALVE
- THREE WAY VALVE - ELECTRICAL
- THREE WAY VALVE
- PRESSURE GAUGE
- PRESSURE REGULATING VALVE
- SOLENOID VALVE
- MECHANICAL VALVE
- TEMPERATURE GAUGE
- WATER HAMMER ARRESTER
- PRESSURE GAUGE

## FITTINGS

- ELBOW LONG SWEEP 90
- ELBOW SHORT SWEEP 90
- ELBOW
- TEE
- CROSS
- CAP
- COUPLING
- PLUG
- PIPE BREAKLINE
- P TRAP
- PIPE DROP / DOWN
- PIPE RISE / UP
- PIPE BRANCH RISE / UP
- PIPE BRANCH DROP / DOWN
- 2"ø SAN — PIPE TAG: SIZE & SYSTEM ABBREVIATION
- 2"ø — PIPE TAG: SIZE
- SAN — PIPE TAG: SYSTEM ABBREVIATION
- 2"ø (2) — PIPE TAG: SIZE & FIXTURE UNITS

## SPRINKLER HEADS

- UPRIGHT
- PENDANT
- CONCEALED
- RECESSED
- SIDE WALL

## DEVICES

- FDC - FIRE DEPARTMENT CONNECTION
- FPTC - FIRE PUMP TEST CONNECTION
- HORN
- HORN / STROBE
- STROBE
- FS — FLOW SWITCH CONNECTION
- TS — TAMPER SWITCH CONNECTION

## GENERAL ANNOTATION SYMBOLS

- X — REFERS TO PLAN NOTE NUMBER
- XXX-X / XXX — FIXTURE AND EQUIPMENT TAG; SPECIALTY EQUIPMENT REFERENCE TAG
- XX / XXX — DETAIL NUMBER; SHEET NO. WHERE DETAIL IS LOCATED
- X / XXX — VIEW REFERENCE TAG
- XX / XXX — VIEW REFERENCE TAG
- PIPE BREAKLINE
- CONNECT NEW TO EXISTING
- 1 — REVISION NUMBER
- ARCH = XXX.XX' / CIVIL = XXX.XX' — ELEVATION MARKER

## ABBREVIATIONS

| | |
|---|---|
| ABV | ABOVE |
| ADD | ADDENDUM |
| AFF | ABOVE FINISHED FLOOR |
| AFG | ABOVE FINISHED GRADE |
| ALT | ALTERNATE |
| ARCH | ARCHITECT / ARCHITECTURAL |
| ASHRAE | AMERICAN SOCIETY OF HEATING, REFRIGERATING, AND AIR-CONDITIONING ENGINEERS |
| ASPE | AMERICAN SOCIETY OF PLUMBING ENGINEERS |
| BAS | BUILDING AUTOMATION SYSTEM |
| BFF | BELOW FINISHED FLOOR |
| BJ | BETWEEN JOISTS |
| BLDG | BUILDING |
| BLW | BELOW |
| BTU | BRITISH THERMAL UNITS |
| BTUH | BRITISH THERMAL UNITS PER HOUR |
| BV | BALANCE VALVE |
| CFH | CUBIC FEET PER HOUR |
| CFM | CUBIC FEET PER MINUTE |
| CM | CONSTRUCTION MANAGER / GENERAL CONTRACTOR |
| CLG | CEILING |
| DEG | DEGREE(S) |
| DIA | DIAMETER |
| DN | DOWN |
| DP | DIFFERENTIAL PRESSURE |
| EC | ELECTRICAL CONTRACTOR |
| ELEC | ELECTRICAL |
| ELEV | ELEVATION |
| EQ | EQUIPMENT |
| ETR | EXISTING TO REMAIN |
| EX | EXISTING |
| F | DEGREES FAHRENHEIT |
| FBO | FURNISHED BY OTHERS |
| FFE | FINISHED FLOOR ELEVATION |
| FPC | FIRE PROTECTION / SPRINKLER CONTRACTOR |
| FT | FOOT / FEET |
| GA | GAUGE |
| GAL | GALLON |
| GC | GENERAL CONTRACTOR / CONSTRUCTION MANAGER |
| GPH | GALLONS PER HOUR |
| GPM | GALLONS PER MINUTE |
| HC | HVAC CONTRACTOR |
| HP | HORSE POWER |
| HVAC | HEATING, VENTILATION, AND AIR CONDITIONING |
| ID | INSIDE DIAMETER |
| I.E. | INVERT ELEVATION |
| IN | INCH |
| INV | INVERT |
| LB | POUND |
| LF | LINEAR FOOT / FEET |
| MAX | MAXIMUM |
| MBH | ONE THOUSAND BTU PER HOUR |
| MC | MECHANICAL CONTRACTOR |
| MECH | MECHANICAL |
| MFR | MANUFACTURER |
| MIN | MINIMUM |
| MIN | MINUTE |
| MISC | MISCELLANEOUS |
| NC | NORMALLY CLOSED |
| NFPA | NATIONAL FIRE PROTECTION ASSOCIATION |
| NIC | NOT IN CONTRACT |
| NO | NORMALLY OPEN |
| NTS | NOT TO SCALE |
| Ø | DIAMETER |
| OC | ON CENTER |
| OD | OUTSIDE DIAMETER |
| PC | PLUMBING CONTRACTOR |
| PLBG | PLUMBING |
| PRESS | PRESSURE |
| PSI | POUNDS PER SQUARE INCH |
| PSIG | POUNDS PER SQUARE INCH GAUGE |
| REC | RECESSED |
| RI | ROUGH-IN |
| RM | ROOM |
| RPM | REVOLUTIONS PER MINUTE |
| SF | SQUARE FOOT |
| TEMP | TEMPERATURE |
| TYP | TYPICAL |
| UC | UNDERCOUNTER |
| UG | UNDERGROUND |
| VTR | VENT THROUGH ROOF |
| W/ | WITH |
| W/O | WITHOUT |

# FIRE PROTECTION SHEET INDEX


| | |
|---|---|
| FP000 | GENERAL NOTES |
| FP1.0 | OVERALL FIRST FLOOR PLAN |
| FP1.2 | OVERALL SECOND FLOOR PLAN |
| FP1.3 | OVERALL THIRD FLOOR PLAN |


# GENERAL DEMOLITION NOTES

1. SEE SPECIFICATIONS FOR MORE INFORMATION.
2. EXISTING ITEMS SHOWN ON THESE DRAWING REFLECT A COMBINATION OF THE EXISTING CONTRACT DRAWINGS, AS-BUILT RECORD DRAWINGS, FIELD SURVEYS AND THE ENGINEER'S ESTIMATION OF PIPE ROUTES. ALL EXISTING PIPE LOCATIONS AND ELEVATIONS SHALL BE FIELD VERIFIED. NOTIFY THE ENGINEER FOR ANY REUSED OF EXISTING PIPING THAT IS IN GOOD CONDITION AND SIZE APPROPRIATELY.
3. ITEMS SHOWN AS HEAVY DASHED ARE TO BE DEMOLISHED.
4. ITEMS SHOWN AS LIGHTER ARE EXISTING TO REMAIN.
5. COORDINATE DEMOLITION OF EXISTING PIPING TO BE REMOVED WITH GENERAL CONTRACTOR.
6. FIELD VERIFY LOCATIONS OF EXISTING PIPE MAINS. REUSE ANY PIPING OF SUFFICIENT SIZE IN GOOD CONDITION. REROUTE AS REQUIRED PER FIELD CONDITIONS.
7. WHERE EXISTING PIPING IS SHOWN TO BE REMOVED, CAP BRANCH PIPE IF NOT BEING USED FOR NEW CONSTRUCTION.

# GENERAL INSTALLATION NOTES

1. SEE THE FIRE SUPPRESSION SPECIFICATION FOR MORE INFORMATION.
2. THE AUTOMATIC FIRE SPRINKLER SYSTEM SHALL BE INSTALLED PER NFPA, BUILDING CODE, FIRE CODE, AND THE LOCAL AUTHORITY HAVING JURISDICTION REQUIREMENTS. IT IS THE RESPONSIBILITY OF THE FIRE PROTECTION CONTRACTOR TO PREPARE ALL FINAL DRAWINGS, CALCULATIONS, AND SUBMITTALS TO THE AUTHORITY HAVING JURISDICTION.
3. PROVIDE AUTOMATIC FIRE SPRINKLER SYSTEM ZONES AND COVERAGE REQUIREMENTS (DENSITIES AND HAZARDS) PER NFPA.
4. THE FIRE PROTECTION CONTRACTOR SHALL PROVIDE THE BASE REQUIREMENTS THAT ARE SHOWN IN THE DRAWINGS. FOR ITEMS NOT SHOWN ON THE DRAWINGS, PROVIDE THE MINIMUM REQUIREMENTS PER NFPA.
5. ALL PIPING SHALL BE CONCEALED WHEN INSTALLED OUTSIDE OF THE MECHANICAL ROOM AND STORAGE AREAS. IF THE BUILDING'S CONSTRUCTION DOES NOT PERMIT THE CONCEALING OF THE PIPING, LOCATIONS AND ROUTING OF THE PIPING SHALL BE APPROVED BY ARCHITECT/OWNER PRIOR TO ANY INSTALLATION.
6. ROUTE PIPING IN STRUCTURAL WEBBING OF FLOOR/CEILING JOIST IN AREAS WHERE PIPING IS EXPOSED.
7. VERIFY THE LOCATION OF ALL OTHER TRADES FIXTURES, EQUIPMENT, ELECTRICAL PANELS, PIPING, DUCT, EXHAUST HOODS WITH PLUMBING HVAC AND ELECTRICAL PLANS. ROUTE PIPING AS REQUIRED PER NFPA.
8. VERIFY LOCATION OF ALL SPACES, SUCH AS ATTICS, CLOSETS, CONCEALED SPACES, TUNNELS, MECHANICAL ROOMS, STORAGE AREAS, SKYLIGHTS, CLOUD CEILINGS, COOLERS / FREEZERS, CORROSIVES ENVIRONMENTS, CANOPIES, AND OVERHEAD DOORS WITH ARCHITECTURAL PLANS. PROVIDE SPRINKLER COVERAGE (DENSITIES AND HAZARDS) AT THESE LOCATIONS PER NFPA. IF A DRY SYSTEM IS REQUIRED, THE FIRE PROTECTION CONTRACTOR SHALL COORDINATE ANY ELECTRICAL REQUIREMENT WITH THE ARCHITECT/ENGINEER.
9. PROVIDE FIRE SPRINKLER COVERAGE FOR AN ELEVATOR HOIST-WAY AND MACHINE EQUIPMENT ROOM AS REQUIRED PER THE ELEVATOR'S SHOP DRAWING AND NFPA.
10. PROVIDE CLOSELY SPACED FIRE SPRINKLERS NEAR THE DRAFT STOPS FOR ALL VERTICAL OPENINGS PER NFPA.
11. PROVIDE STANDPIPE HOSE CONNECTIONS FOR EXIT STAIRS AND HORIZONTAL EXITS AS REQUIRED PER THE LOCAL FIRE MARSHAL AND NFPA.
12. ZONES RISER ISOLATION VALVES, TAMPER SWITCH, FLOW SWITCHES, PRESSURE GAUGE AND DRAINS VALVES SHALL BE INSTALLED IN AN ACCESSIBLE LOCATION AND CONCEALED FROM BUILDING OCCUPANT'S VIEW WHEN LOCATED OUTSIDE OF MECHANICAL ROOMS OR STORAGE AREAS.
13. PROVIDE DRAIN DOWN VALVES AT LOW POINTS OF THE FIRE SPRINKLERS SYSTEMS AND LOCATE THE INSPECTOR TEST AS AT THE MOST REMOTE END OF FIRE SPRINKLER SYSTEM ZONES PER NFPA.
14. FIRE SPRINKLER HEADS LOCATED IN CEILING WITH LAY-IN CEILING TILES SHALL BE CENTERED WITHIN THE TILE. FLEXIBLE CONNECTORS TO FIRE SPRINKLER HEADS ARE ACCEPTABLE. THE PIPING SHALL BE LOCATED A MINIMUM OF 6" ABOVE THE CEILING TILE TO ALLOW FOR REMOVAL OF THE CEILING TILE.
15. FIRE SPRINKLER HEADS LOCATED IN A PAINTED SOFFIT, WOOD SOFFIT, OR CEILING, SHALL HAVE ITS CONCEALED COVER PLATED COLOR MATCH WITH THAT OF THE ADJACENT SURFACE. THE SPRINKLER HEADS CONCEAL COVER PLATE COLOR SHALL BE FACTORY PAINTED. ALL FIRE SPRINKLER HEADS CONCEALED COVER PLATE COLORS SHALL BE APPROVED BY THE ARCHITECTURAL PRIOR TO ORDERING AND INSTALLATION.
16. PROVIDE FIRE SPRINKLER COVERAGE IN AREAS OF THE EXISTING BUILDING WHERE NEW FIRE SPRINKLER MAIN SUPPLY PIPING IS ROUTED THROUGH PER NFPA.
17. PROVIDE FIRE SPRINKLER COVERAGE INSIDE EXHAUST DUCTS THAT ARE 10 INCHES AND GREATER CONVEYING COMBUSTIBLE MATERIALS IN THE TECHNICAL EDUCATION AREA AS REQUIRED PER IBC, EIBC, AND NFPA STANDARDS. STAINLESS STEEL FLEXIBLE BRAIDED SUPPLY AND PENDANT FIRE SPRINKLER HEADS ARE ACCEPTABLE. PROVIDE DRY BARREL TYPE FIRE SPRINKLER HEADS OR DRY SYSTEM FOR EXHAUST DUCT THAT IS EXPOSED TO FREEZING TEMPERATURES.
18. PROVIDE EXTERIOR DUST COLLECTOR WITH A DRY FIRE SPRINKLER PIPE CONNECTION. PROVIDE GALVANIZED PIPE AND FITTINGS OUTSIDE OF THE BUILDING. COORDINATE FINAL LOCATION, SIZE AND FITTING TYPE WITH HVAC CONTRACTOR.
19. PROVIDE A WATER DELUGE SYSTEM FOR 2 HOUR PROSCENIUM OPENING. FIRE SPRINKLER HEADS SHALL BE OPEN-HEAD TYPE WITH ELECTRIC HEAT DETECTION SYSTEM. COORDINATE PIPE ROUTING IN AREAS WITH THEATER LIGHTS AND RIGGING EQUIPMENT PRIOR TO DESIGN AND INSTALLATION. THE WATER DELUGE SYSTEM ZONE RISER SHALL BE A DRY TYPE VALVE AND WITH AIR COMPRESSOR.
20. PROVIDE PERFORMANCE STAGES WITH ORDINARY HAZARD GROUP 2 DESIGN CRITERIA FIRE SPRINKLER COVERAGE. THE FIRE SPRINKLER HEADS SHALL BE PLACED BELOW ALL PLATFORMS AND CATWALKS. THE FIRE SPRINKLERS PIPING SHALL BE BLACK STEEL AND FIRE SPRINKLERS HEADS SHALL BE HIGH TEMPERATURE UPRIGHT OR PENDANT WITH A BRASS FINISH. COORDINATE ALL PIPE ROUTING IN AREAS WITH THEATER LIGHTS AND RIGGING EQUIPMENT PRIOR TO DESIGN AND INSTALLATION. PROVIDE CLASS II FIRE HOSE CONNECTION AND HOSE CABINET ON EACH SIDE OF THE STAGE OPENING PER NFPA.


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Sheet Title: **GENERAL NOTES**

Sheet Number: **FP000**


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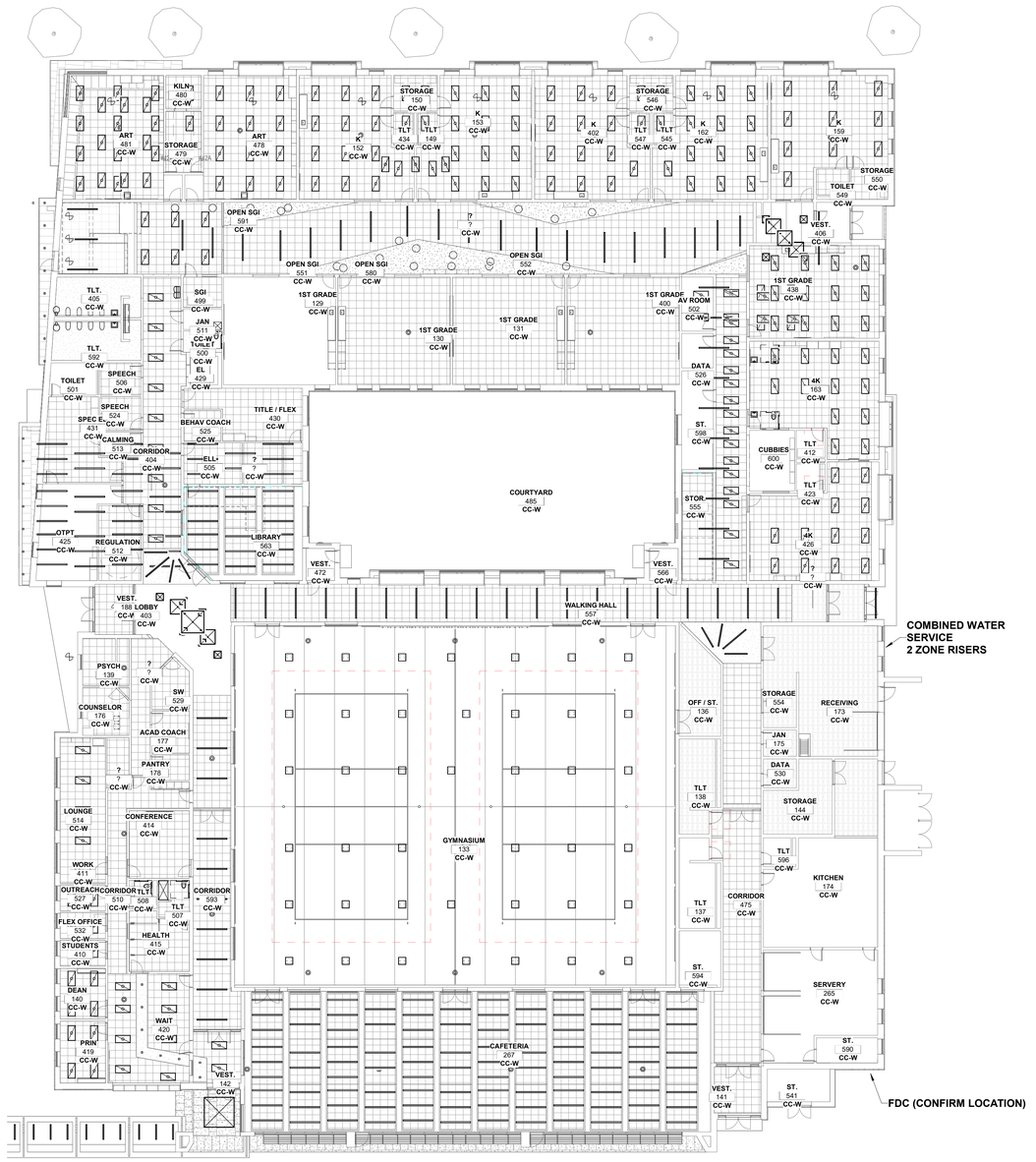


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# SHEET KEYNOTES

COMBINED WATER SERVICE 2 ZONE RISERS

FDC (CONFIRM LOCATION)




1 FP1.0 OVERALL FIRST FLOOR PLAN

Scale: 1/16" = 1'-0"


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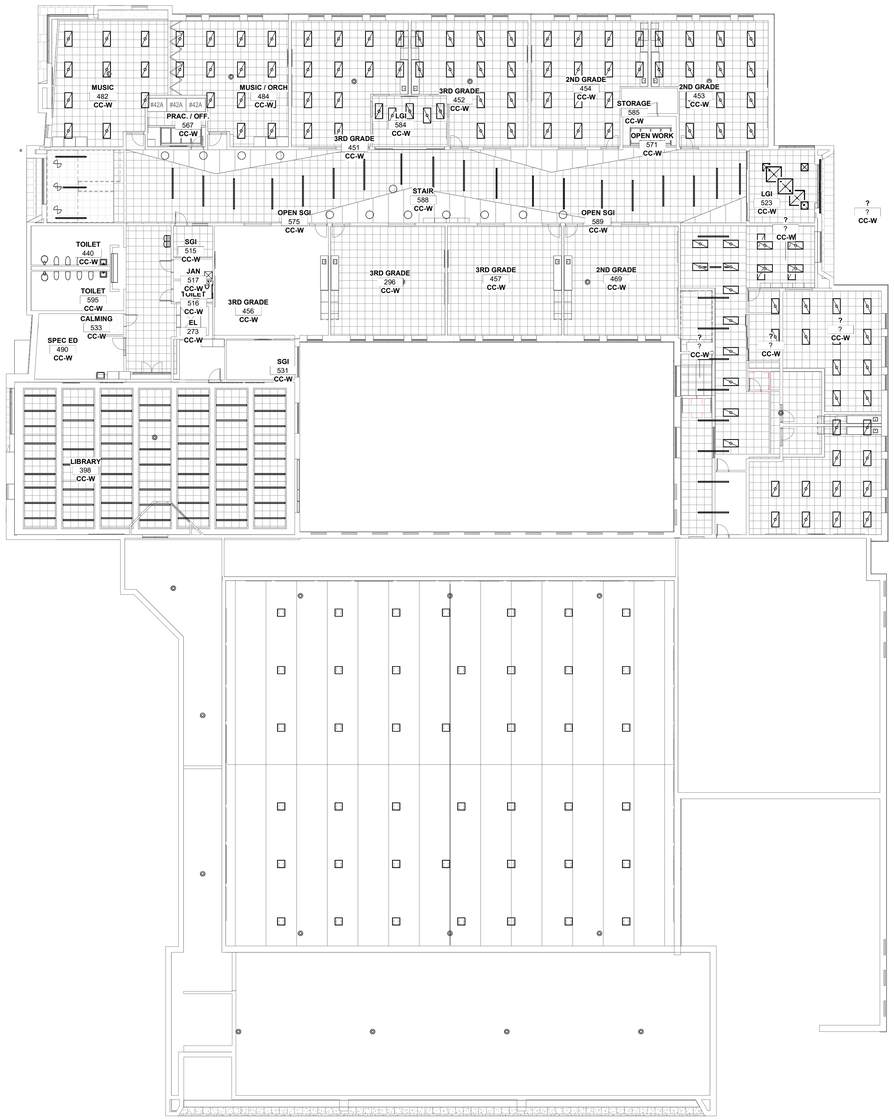
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OVERALL FIRST FLOOR PLAN

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## SHEET KEYNOTES




1 FP1.2 OVERALL SECOND FLOOR PLAN

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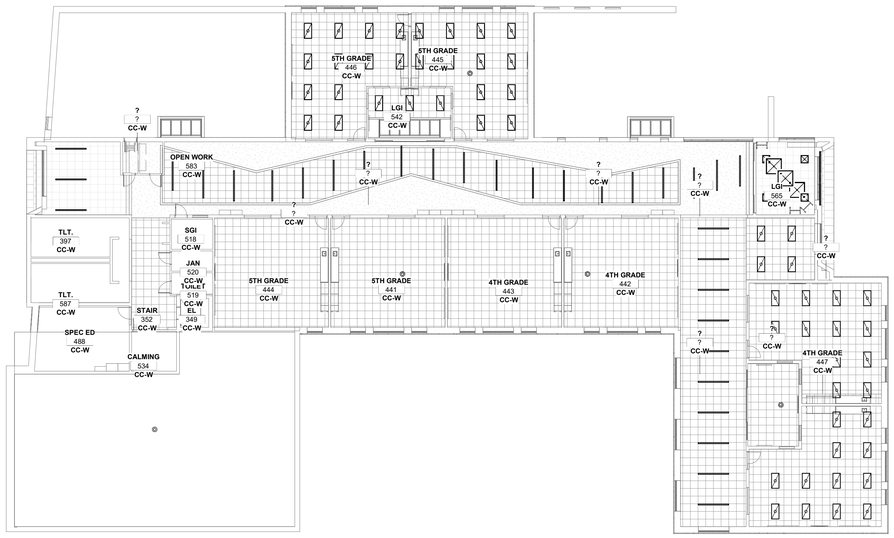
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**OVERALL SECOND FLOOR PLAN**

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## SHEET KEYNOTES




1 FP1.3 OVERALL THIRD FLOOR PLAN

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**OVERALL THIRD FLOOR PLAN**

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# PLUMBING SYMBOLS AND ABBREVIATIONS

THIS IS A COMPREHENSIVE SYMBOL AND ABBREVIATION LIST - NOT ALL SYMBOLS ARE APPLICABLE TO THESE DRAWINGS.

## PIPING SYSTEMS

| | | |
|---|---|---|
| CW | DOMESTIC COLD WATER - HARD |
| CWS | DOMESTIC COLD WATER - SOFT |
| HW | DOMESTIC HOT WATER - SUPPLY 120 |
| HW-140 | DOMESTIC HOT WATER - SUPPLY 140 |
| HWR | DOMESTIC HOT WATER - RETURN 120 |
| HWR-140 | DOMESTIC HOT WATER - RETURN 140 |
| NP-CW | NON-POTABLE COLD WATER - HARD |
| NP-CWS | NON-POTABLE COLD WATER - SOFT |
| NP-HW | NON-POTABLE HOT WATER - SUPPLY |
| NP-HWR | NON-POTABLE HOT WATER - RETURN |
| DI | DEIONIZED WATER |
| RO | REVERSE OSMOSIS WATER |
| TW | TEMPERED WATER - SUPPLY |
| TWR | TEMPERED WATER - RETURN |
| SAN | SANITARY WASTE |
| V | SANITARY VENT |
| GW | GREASE WASTE |
| AW | ACID WASTE |
| AV | ACID VENT |
| ST | STORM DRAINAGE - PRIMARY |
| OF | OVERFLOW DRAINAGE - SECONDARY |
| CLV | CLEAR WATER VENT |
| CLW | CLEAR WATER WASTE |
| DT | DRAIN TILE |
| PW | PROCESS WASTE |
| PV | PROCESS VENT |
| PD | PUMPED DISCHARGE |
| EXHAUST | EQUIPMENT - EXHAUST |
| INTAKE | EQUIPMENT - INTAKE |
| CA | COMPRESSED AIR |
| G | NATURAL GAS |
| LP | LIQUID PROPANE |
| M-AIR | MEDICAL - AIR |
| M-CO | MEDICAL - CARBON DIOXIDE |
| M-N | MEDICAL - NITROGEN |
| M-NO | MEDICAL - NITROUS OXIDE |
| M-OX | MEDICAL - OXYGEN |
| M-VAC | MEDICAL - VACUUM |
| ACE | WELDING - ACETYLENE |
| ARG | WELDING - ARGON |
| ARG/CO | WELDING - ARGON/CARBON DIOXIDE |
| OX | WELDING - OXYGEN |
| WV | WELDING - VENT |
| | DEMOLISHED PIPING |

## ACCESSORIES

| | |
|---|---|
| | BACKFLOW PREVENTER - DC |
| | BACKFLOW PREVENTER - DCDA |
| | BACKFLOW PREVENTER - PVB |
| | BACKFLOW PREVENTER - RP |
| | SHUTOFF VALVE |
| | BUTTERFLY VALVE |
| | BALANCING VALVE |
| | BACKWATER VALVE |
| | CHECK VALVE |
| | THERMOSTATIC MIXING VALVE |
| | DRAIN TEMPERING VALVE |
| | THREE WAY VALVE - ELECTRICAL |
| | THREE WAY VALVE |
| | PRESSURE REGULATING VALVE |
| | SOLENOID VALVE |
| | MECHANICAL VALVE |
| | SUCTION DIFFUSER |
| | THERMOMETER |
| | WATER HAMMER ARRESTER |
| | PRESSURE GAUGE |

## FITTINGS

| | |
|---|---|
| | ELBOW LONG SWEEP 90 |
| | ELBOW SHORT SWEEP 90 |
| | ELBOW |
| | TEE |
| | TEE |
| | CROSS |
| | TEE DOUBLE |
| | WYE 45 DEG |
| | WYE 45 DEG DOUBLE |
| | WYE COMBINATION |
| | WYE COMBINATION DOUBLE |
| | CAP |
| | COUPLING |
| | PLUG |
| | PIPE BREAKLINE |
| | P TRAP |
| | PIPE DROP / DOWN |
| | PIPE RISE / UP |
| | PIPE BRANCH RISE / UP |
| | PIPE BRANCH DROP / DOWN |
| 2"ø SAN | PIPE TAG: SIZE & SYSTEM ABBREVIATION |
| 2"ø | PIPE TAG: SIZE |
| SAN | PIPE TAG: SYSTEM ABBREVIATION |
| 2"ø (0) | PIPE TAG: SIZE & FIXTURE UNITS |

## GENERAL

### ANNOTATION SYMBOLS

| | |
|---|---|
| X | REFERS TO PLAN NOTE NUMBER |
| XXX-X / XXX | FIXTURE AND EQUIPMENT TAG<br>SPECIALTY EQUIPMENT REFERENCE TAG |
| XX / XXX | DETAIL NUMBER<br>SHEET NO. WHERE DETAIL IS LOCATED |
| X / XXX | VIEW REFERENCE TAG |
| XX / XXX | VIEW REFERENCE TAG |
| | PIPE BREAKLINE |
| | CONNECT NEW TO EXISTING |
| 1 | REVISION NUMBER |
| ARCH = XXX.XX' / CIVIL = XXX.XX' | ELEVATION MARKER |

### ABBREVIATIONS

| | |
|---|---|
| ABV | ABOVE |
| ADD | ADDENDUM |
| AFF | ABOVE FINISHED FLOOR |
| AFG | ABOVE FINISHED GRADE |
| ALT | ALTERNATE |
| ARCH | ARCHITECT / ARCHITECTURAL |
| ASHRAE | AMERICAN SOCIETY OF HEATING, REFRIGERATING, AND AIR-CONDITIONING ENGINEERS |
| ASPE | AMERICAN SOCIETY OF PLUMBING ENGINEERS |
| BAS | BUILDING AUTOMATION SYSTEM |
| BFF | BELOW FINISHED FLOOR |
| BJ | BETWEEN JOISTS |
| BLDG | BUILDING |
| BLW | BELOW |
| BTU | BRITISH THERMAL UNITS |
| BTUH | BRITISH THERMAL UNITS PER HOUR |
| BV | BALANCE VALVE |
| CB | CONSTRUCTION BULLETIN |
| CFH | CUBIC FEET PER HOUR |
| CFM | CUBIC FEET PER MINUTE |
| CM | CONSTRUCTION MANAGER |
| CLG | CEILING |
| DEG | DEGREE(S) |
| DIA | DIAMETER |
| DN | DOWN |
| DP | DIFFERENTIAL PRESSURE |
| EC | ELECTRICAL CONTRACTOR |
| ELEC | ELECTRICAL |
| ELEV | ELEVATION |
| EQ | EQUIPMENT |
| ETR | EXISTING TO REMAIN |
| EX | EXISTING |
| F | DEGREES FAHRENHEIT |
| FBO | FURNISHED BY OTHERS |
| FFE | FINISHED FLOOR ELEVATION |
| FPC | FIRE PROTECTION / SPRINKLER CONTRACTOR |
| FT | FOOT / FEET |
| GA | GAUGE |
| GAL | GALLON |
| GC | GENERAL CONTRACTOR |
| GPH | GALLONS PER HOUR |
| GPM | GALLONS PER MINUTE |
| HC | HVAC CONTRACTOR |
| HP | HORSE POWER |
| HVAC | HEATING, VENTILATION, AIR CONDITIONING |
| ID | INSIDE DIAMETER |
| I.E. | INVERT ELEVATION |
| IN | INCH |
| INV | INVERT |
| LB | POUND |
| LF | LINEAR FOOT / FEET |
| MAX | MAXIMUM |
| MBH | ONE THOUSAND BTU PER HOUR |
| MC | MECHANICAL CONTRACTOR |
| MECH | MECHANICAL |
| MFR | MANUFACTURER |
| MIN | MINIMUM |
| MIN | MINUTE |
| MISC | MISCELLANEOUS |
| NC | NORMALLY CLOSED |
| NFPA | NATIONAL FIRE PROTECTION ASSOCIATION |
| NIC | NOT IN CONTRACT |
| NO | NORMALLY OPEN |
| NTS | NOT TO SCALE |
| Ø | DIAMETER |
| OC | ON CENTER |
| OD | OUTSIDE DIAMETER |
| PC | PLUMBING CONTRACTOR |
| PLBG | PLUMBING |
| PRESS | PRESSURE |
| PS | PROJECT SUPPLEMENT |
| PSI | POUNDS PER SQUARE INCH |
| PSIG | POUNDS PER SQUARE INCH GAUGE |
| REC | RECESSED |
| REV | REVISION |
| RI | ROUGH-IN |
| RM | ROOM |
| RPM | REVOLUTIONS PER MINUTE |
| SF | SQUARE FOOT |
| TEMP | TEMPERATURE |
| TYP | TYPICAL |
| UC | UNDERCOUNTER |
| UG | UNDERGROUND |
| VTR | VENT THROUGH ROOF |
| W/ | WITH |
| W/O | WITHOUT |

| PLUMBING CONNECTION SCHEDULE | | | | | | | |
|---|---|---|---|---|---|---|---|
| TAG | DESCRIPTION | COLD WATER CONNECTION SIZE | CWFU | HOT WATER CONNECTION SIZE | HWFU | WASTE CONNECTION SIZE | WFU |

## GENERAL DEMOLITION NOTES

1. SEE SPECIFICATIONS FOR MORE INFORMATION.
2. EXISTING ITEMS SHOWN ON THESE DRAWING REFLECT A COMBINATION OF THE EXISTING CONTRACT DRAWINGS, AS-BUILT RECORD DRAWINGS, FIELD SURVEYS AND THE ENGINEER'S ESTIMATION OF PIPE ROUTES. ALL EXISTING PIPE LOCATIONS AND ELEVATIONS SHALL BE FIELD VERIFIED. NOTIFY THE ENGINEER FOR ANY REUSED OF EXISTING PIPING THAT IS IN GOOD CONDITION AND SIZE APPROPRIATELY.
3. ITEMS SHOWN AS HEAVY DASHED ARE TO BE DEMOLISHED.
4. ITEMS SHOWN AS LIGHTER ARE EXISTING TO REMAIN.
5. COORDINATE DEMOLITION OF EXISTING PIPING TO BE REMOVED WITH GENERAL CONTRACTOR.
6. FIELD VERIFY LOCATIONS OF EXISTING PIPE MAINS. REUSE ANY PIPING OF SUFFICIENT SIZE IN GOOD CONDITION. REROUTE AS REQUIRED PER FIELD CONDITIONS.
7. WHERE EXISTING PIPING IS SHOWN TO BE REMOVED, CAP BRANCH PIPE IF NOT BEING USED FOR NEW CONSTRUCTION.

## GENERAL INSTALLATION NOTES

1. SEE PLUMBING SPECIFICATIONS FOR MORE INFORMATION.
2. PLUMBING INSTALLATION SHALL BE INSTALLED PER WISCONSIN UNIFORM PLUMBING CODE AND PER LOCAL PLUMBING CODE FOR ITEMS NOT NOTED OR SHOWN ON THESE DRAWINGS.
3. EXISTING ITEMS SHOWN ON THESE DRAWING REFLECT A COMBINATION OF THE EXISTING CONTRACT DRAWINGS, AS-BUILT RECORD DRAWINGS, FIELD SURVEYS AND THE ENGINEER'S ESTIMATION OF PIPE ROUTES. ALL EXISTING PIPE LOCATIONS AND ELEVATIONS SHALL BE FIELD VERIFIED. REROUTE NEW PIPING AS REQUIRED PER FIELD CONDITIONS.
4. FIELD VERIFY UNDERGROUND PIPING LOCATION, DEPTH AND SIZE AT POINT OF CONNECTION AND THAT NEW PIPE ROUTE IS CLEAR OF UTILITIES AND OTHER OBSTRUCTIONS PRIOR TO INSTALLATION OF ANY UNDERGROUND PIPING. COSTS INCURRED FOR FAILURE TO DO SO SHALL BE THE CONTRACTOR'S RESPONSIBILITY.
5. ALL PIPING IS TO BE CONCEALED. IF BUILDING CONSTRUCTION DOES NOT PERMIT CONCEALING PIPING, LOCATIONS AND ROUTING ARE TO BE APPROVED BY ARCHITECT/OWNER PRIOR TO INSTALLATION.
6. ROUTE ALL PIPING IN COORDINATION WITH OTHER TRADES.
7. FLOOR AND WALL CLEANOUT LOCATIONS NOT PERMITTED TO BE MOVED WITHOUT APPROVAL OF ARCHITECT/ENGINEER.
8. SEE ARCHITECTURAL PLANS FOR ADA RELATED INSTALLATION DETAILS.
9. SEE STRUCTURAL FOOTING, TRUSS AND JOIST ELEVATIONS AND DETAILS.
10. SEE HVAC PLANS FOR AREAS THAT ARE USED AS A RETURN AIR PLENUM. PROVIDE PLENUM RATED PIPE OR PIPE WRAP AS PER PROJECT'S PLUMBING SPECIFICATIONS.
11. SEE HVAC PLANS AND HVAC CONTRACTOR FOR FINAL LOCATION OF HVAC EQUIPMENT IN MECHANICAL / BOILER ROOMS TO COORDINATE FINAL LOCATIONS OF FLOOR / HUB / SITE DRAINS PRIOR TO INSTALLATION.
12. SEE PLUMBING ISOMETRICS SHEETS FOR PIPE SIZE AND LOAD INFORMATION NOT SHOWN ON FLOOR PLANS.
13. SLOPE ALL SANITARY AND STORM PIPING 3" AND LARGER AT 1/8"/FT UNLESS NOTED OTHERWISE.
14. SLOPE ALL SANITARY AND STORM PIPING 2" AND SMALLER AT 1/4"/FT UNLESS NOTED OTHERWISE.
15. SLOPE ALL SANITARY GREASE WASTE PIPING AT 1/4"/FT UNLESS NOTED OTHERWISE.
16. INSTALL CLEANOUTS AT STACKS WHICH PENETRATE THE LOWEST FLOOR LEVEL 30" A.F.F. UNLESS NOTED OTHERWISE.
17. PROVIDE ISOLATION BALL VALVES AND BALANCING VALVES TO ISOLATE COLD WATER, HOT WATER SUPPLY AND HOT WATER RETURN BRANCH PIPING FROM ITS COLD WATER, HOT WATER SUPPLY AND HOT WATER RETURN MAIN PIPING FOR EACH PLUMBING FIXTURE OR ROOMS WITH MULTIPLE PLUMBING FIXTURES. ALL WATER SUPPLY PIPING SHALL BE FLUSHED AND HOT WATER PIPING SYSTEM BALANCED BY PLUMBING CONTRACTOR. SEE PLUMBING SPECIFICATIONS FOR MORE INFORMATION.
18. INSTALL EXTERIOR HOSE BIBBS AT 18" A.F.F. UNLESS NOTED OTHERWISE.
19. INSTALL INTERIOR HOSE BIBBS AT 36" A.F.F. UNLESS NOTED OTHERWISE.
20. INSTALL DOWN SPOUT NOZZLES AT 18" A.F.F. UNLESS NOTED OTHERWISE.

## PLUMBING SHEET INDEX


| | |
|---|---|
| P000 | GENERAL NOTES |
| P1.0 | OVERALL FOUNDATION PLAN |
| P1.1 | OVERALL FIRST FLOOR PLAN |
| P1.2 | OVERALL SECOND FLOOR PLAN |
| P1.3 | OVERALL THIRD FLOOR PLAN |
| P1.4 | ROOF PLAN |


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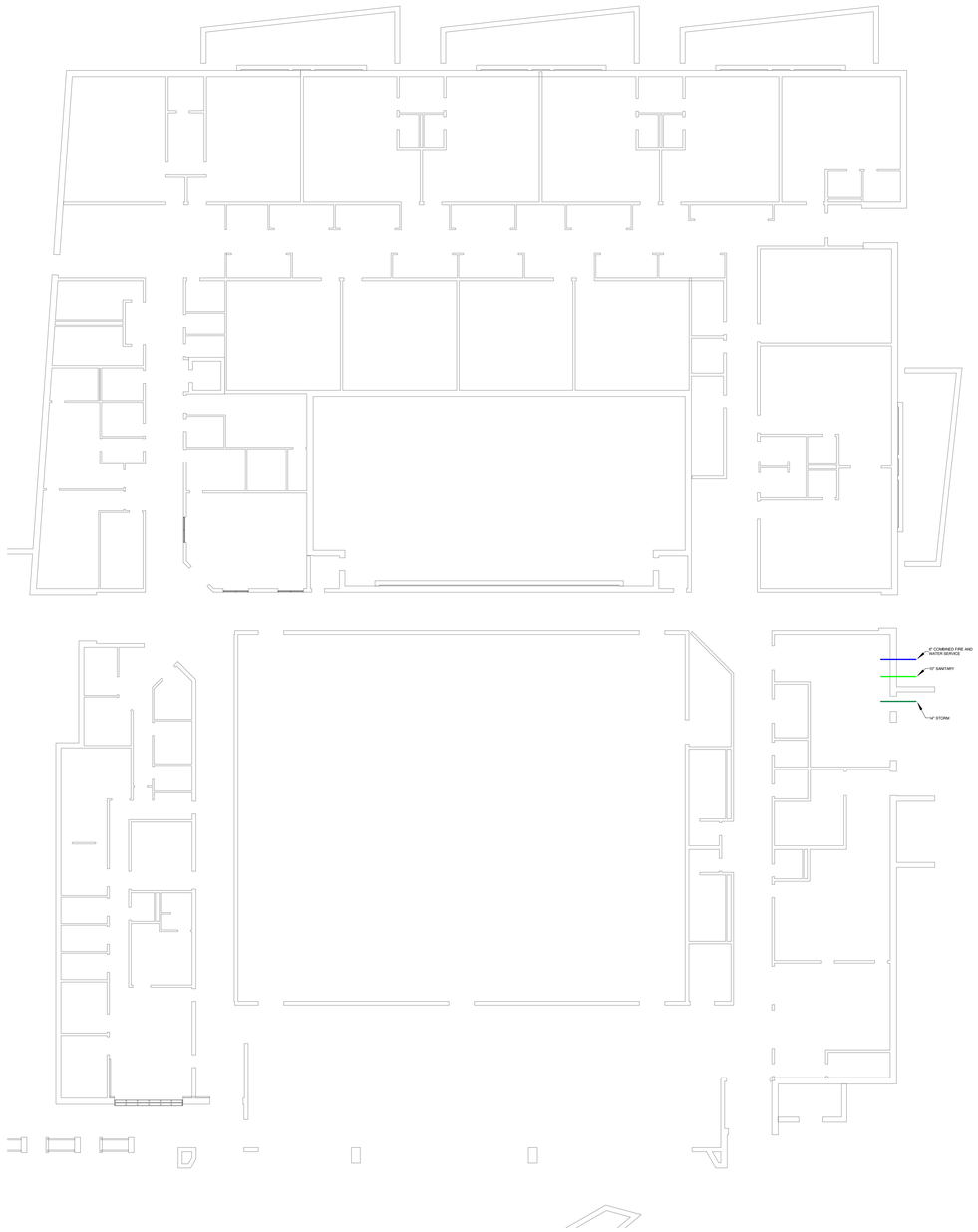
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## SHEET KEYNOTES




1 P1.0 OVERALL FOUNDATION P LAN
Scale: 1/16" = 1'-0"

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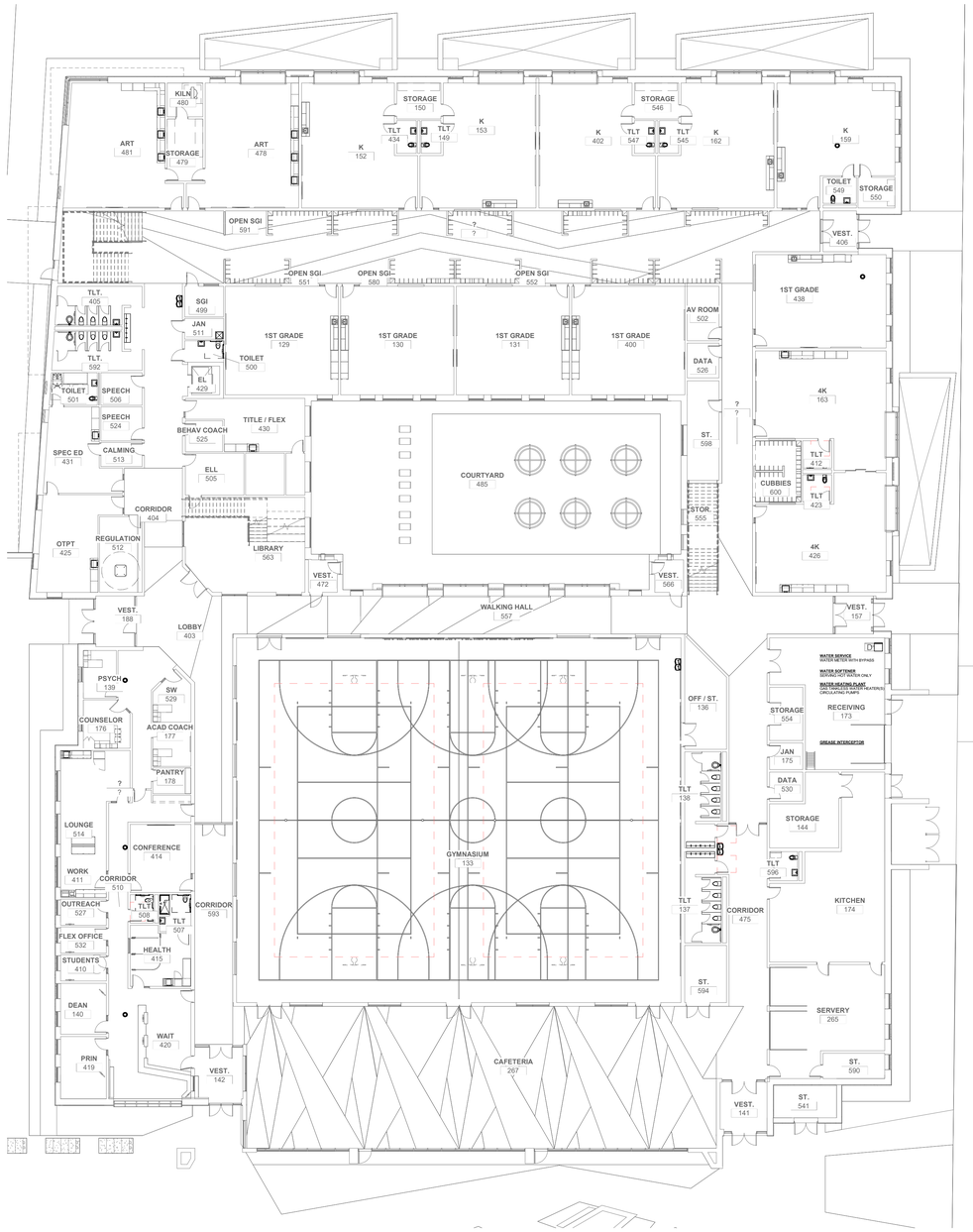
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**OVERALL FOUNDATION PLAN**

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P1.0

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## SHEET KEYNOTES




1 P1.1 OVERALL FIRST FLOOR PLAN

Scale: 1/16" = 1'-0"


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Project Title:
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**New Elementary School**
**La Crosse School District**
**807 East Ave S**
**La Crosse, WI 54601**

REVISIONS:

| # | DATE | DESCRIPTION |
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Sheet Title:
**OVERALL FIRST FLOOR PLAN**

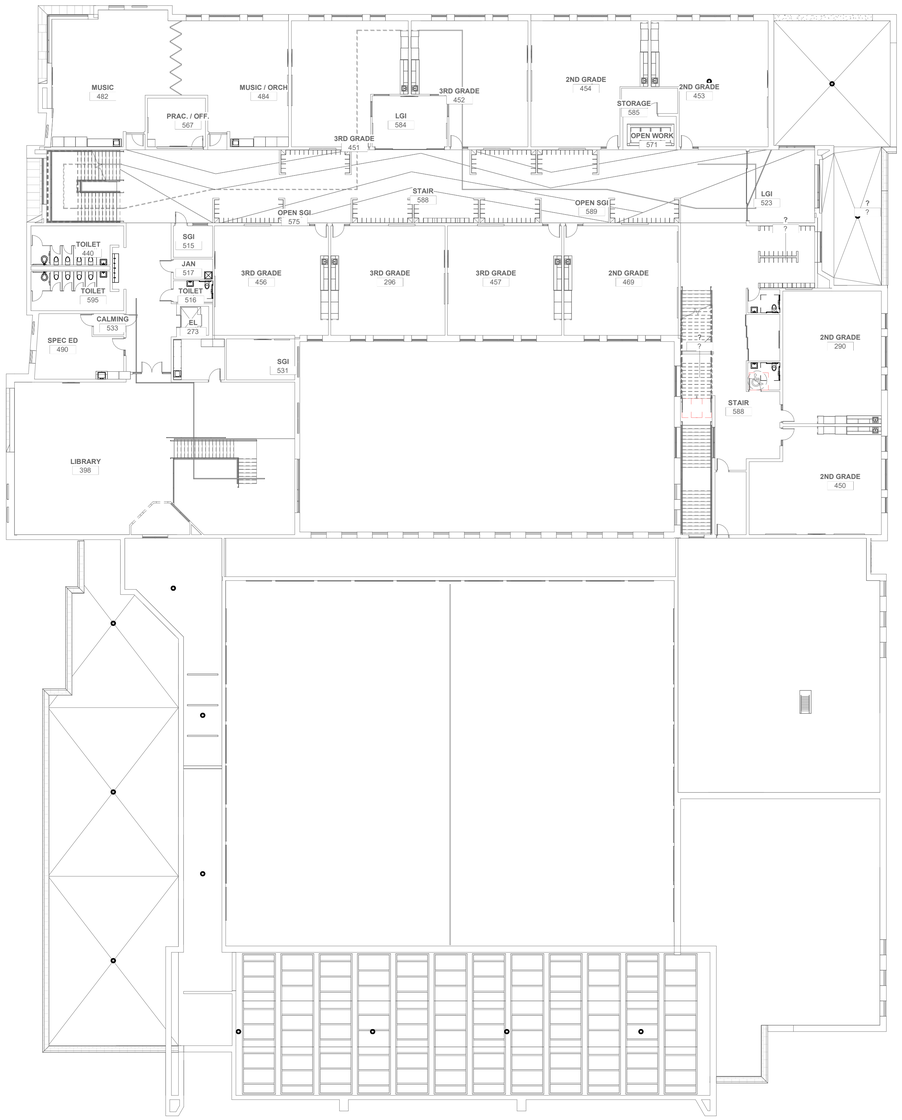
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E D C B A

## SHEET KEYNOTES




1 / P1.2 OVERALL SECOND FLOOR PLAN

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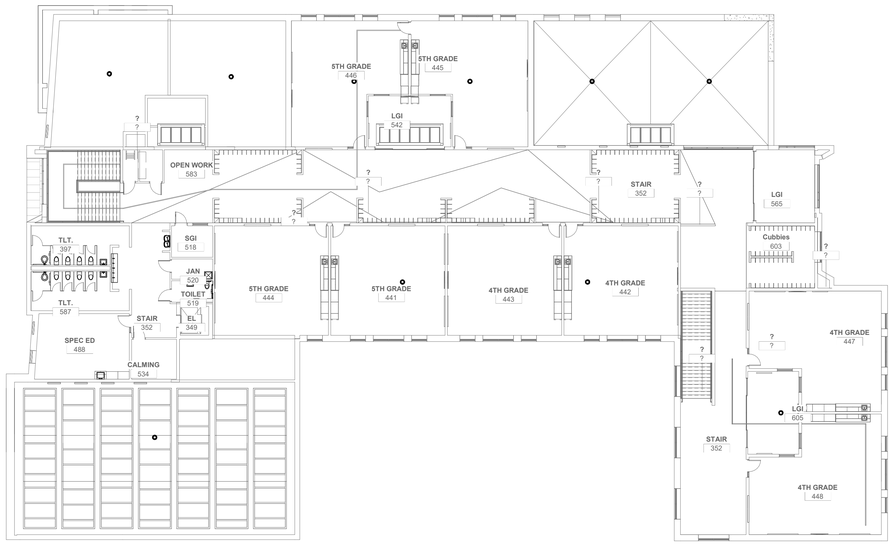
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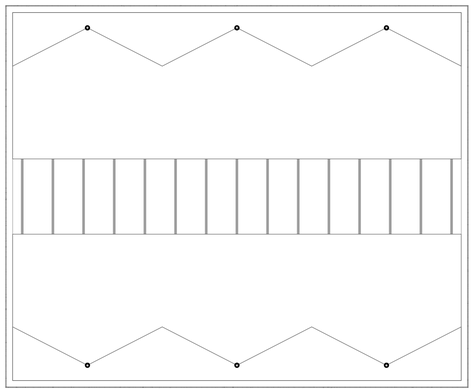
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## SHEET KEYNOTES



1 P1.3 OVERALL THIRD FLOOR PLAN

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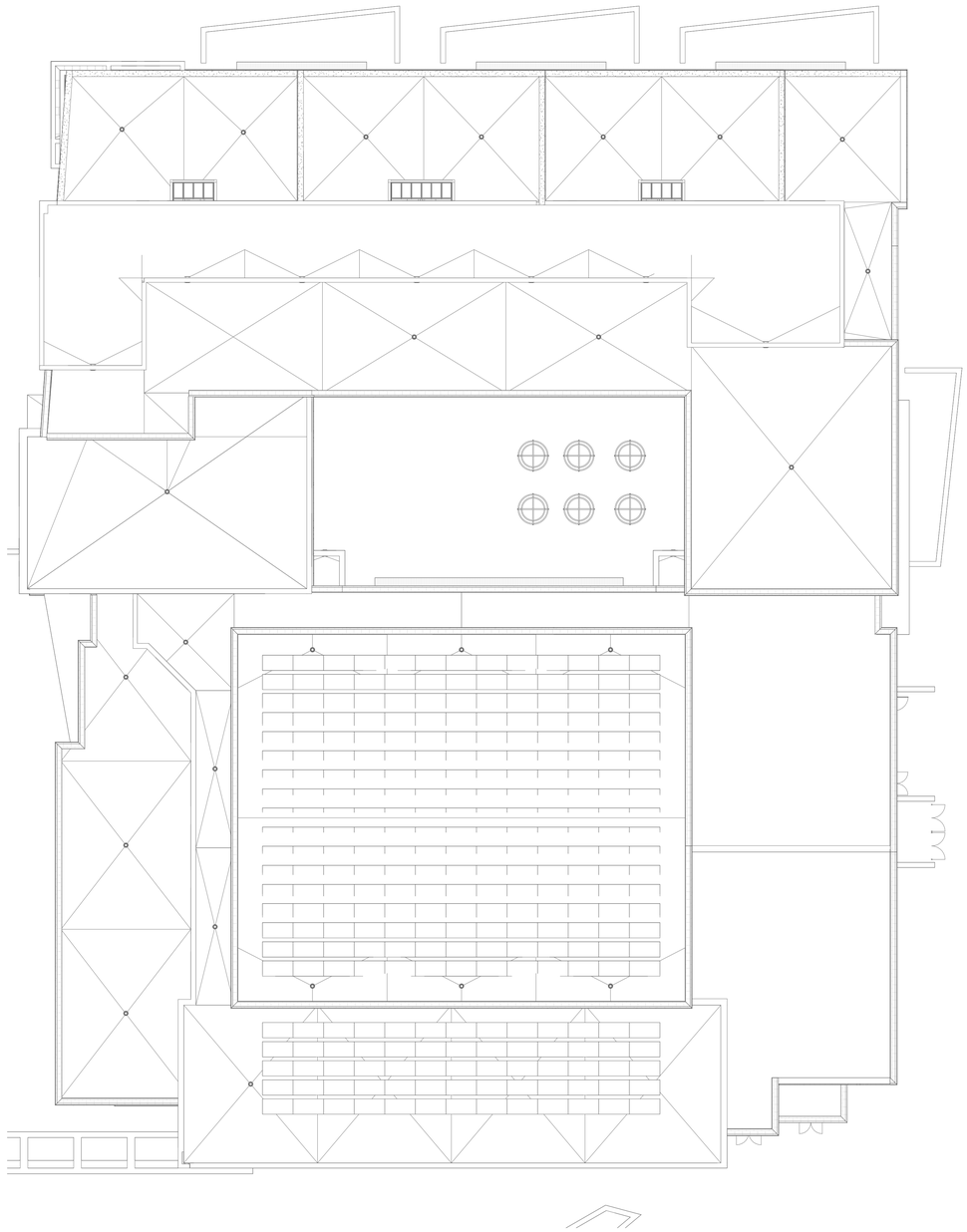
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# SHEET KEYNOTES



1 P1.4 ROOF PLAN
Scale: 1/16" = 1'-0"


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**ROOF PLAN**

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# ELECTRICAL SYMBOLS AND ABBREVIATIONS

THIS IS A COMPREHENSIVE SYMBOL AND ABBREVIATION LIST - NOT ALL SYMBOLS ARE APPLICABLE TO THESE DRAWINGS.

## ELECTRICAL DISTRIBUTION EQUIPMENT

- UTILITY TRANSFORMER
- UTILITY TRANSFORMER
- CT CABINET AND METER SOCKET
- GSAP — GENERATOR ANNUNCIATOR PANEL FLUSH OR SURFACE
- PANEL - NEW SURFACE MOUNTED; TAG = "XX"
- PANEL - NEW FLUSH MOUNTED; TAG = "XX"
- PANEL - EXISTING SURFACE MOUNTED; TAG = "XX"
- PANEL - EXISTING FLUSH MOUNTED; TAG = "XX"
- TRANSFORMER - FLOOR MOUNT; TAG = "T-XX"
- TRANSFORMER - WALL MOUNT; TAG = "T-XX"
- TRANSFER SWITCH; TAGS = "ATS-XXX-Y", "MTS-XXX-Y"
  TAGS XXX, 700=EMERGENCY SYSTEM
  701=LEGALLY REQUIRED STANDBY SYSTEM
  702=OPTIONAL STANDBY SYSTEM
  TAG Y=SYSTEM NUMBER
- SURGE PROTECTION DEVICE; TAG = SPD
  CONTACTOR ENCLOSURE PANEL; TAG = CX
  X = CONTACTOR NUMBER

## MOTOR CONTROL

- X — HEAVY DUTY DISCONNECT; TAG XX, WP=WEATHERPROOF, F=FUSIBLE
- NON-COMBINATION STARTER
- COMBINATION STARTER
- MS — MANUAL STARTER
- X — MOTOR OUTLET TERMINAL, ON ROOF MOUNTED UNITS PROVIDE WEATHER PROOF DISCONNECTS. X = HORSE POWER OR MOTOR NUMBER IF SCHEDULE IS PROVIDED

## WIRING DEVICES

### SWITCHES

- SX — SINGLE POLE SWITCH; TAG X, 3=3 WAY, 4=4 WAY, D=DIMMER, H=LLUMINATED, K=KEYED, M=MOTION SENSOR SWITCH, P=PILOT, R=RELAY, WP=WEATHERPROOF, LVX=LOW VOLTAGE, TAG X = INDICATES NUMBER OF BUTTONS AT EACH SWITCH LOCATION
- $X — SWITCH TO BE DEMOLISHED, X = ABBREVIATIONS ABOVE
- SINGLE PUSH BUTTON - ADA DOOR BUTTON
  ADDITIONAL FUNCTIONALITY DEFINED ON PLANS
- UP/DN PUSH BUTTON
- MULTI PUSH BUTTON

### RECEPTACLES

NO TAG=STANDARD RECEPTACLE, TAGS X, ARC=ARC FAULT RATED, CM=CEILING MOUNTED, CW=CLOTHES WASHER, D=EXTRA DEEP, DW=DISH WASHER, EWC=ELECTRIC WATER COOLER, FM=FLOOR MOUNTED, FR=FREEZER, HG=HOSPITAL GRADE, GD=GARBAGE DISPOSER, GFI=GROUND FAULT INTERRUPTER, GR=GAS RANGE, ICE=ICE MACHINE, IG=ISOLATED GROUND, MIC=MICROWAVE, REF=REFRIGERATOR, SP=SURGE PROTECTED, UCREF=UNDER COUNTER REFRIGERATOR, DEVICE, UPS=UNINTERRUPTED POWER SUPPLY, USB=UNIVERSAL SERIAL BUS, VM=VENDING MACHINE, WP/GFI=WEATHER PROOF/GROUND FAULT INTERRUPTER

- X — RECEPTACLE - SINGLE OR SIMPLEX
- X — RECEPTACLE - DUPLEX
  LINE INDICATES DEVICE MOUNTED AT 42" TO CENTER.
  NO LINE INDICATES DEVICE MOUNTED AT STANDARD HEIGHT UNLESS TAGGED OTHERWISE, TYPICAL ALL RECEPTACLE TYPES AND BLANK BOXES
- XX — RECEPTACLE - TAG XX, CD=CLOTHES DRYER (NEMA14-30R) ER=ELECTRIC RANGE (NEMA14-50R), K=KILN W=WELDER (NEMA 6-50R)
- X — RECEPTACLE - DUPLEX MOUNTED ABOVE COUNTER
- X — RECEPTACLE - DUPLEX WITH TOP HALF SWITCHED
- X — RECEPTACLE - DOUBLE DUPLEX
- X — RECEPTACLE - DUPLEX TRIPLE
- X — RECEPTACLE - DUPLEX QUAD
- X — BLANK OUTLET FOR FUTURE DEVICES 4" BOX, SINGLE GANG RING, BLANKPLATE, 1"C. STUBBED INTO ACCESSIBLE CEILING

## WIRING DEVICES

### MISCELLANEOUS

- SURFACE RACEWAY
- EQUIPMENT CONNECTION
- X — SPECIAL OUTLET, TAG = NUMBER ON SCHEDULE
- H — ELECTRIC HAND/HAIR DRYER
- X — DUAL SERVICE FLOOR BOX, TAG = TYPE IN SPECS
- X — SINGLE SERVICE FLOOR BOX, TAG = TYPE IN SPECS
- X — POKE THROUGH FLOOR BOX, TAG = TYPE IN SPECS
- CD — CORD DROP
- CR — CORD REEL
- LB — 90° CONDUIT BODY - THROUGH WALL
- LR — 90° CONDUIT BODY - TURN RIGHT
- LL — 90° CONDUIT BODY - TURN LEFT
- X EV — ELECTRIC VEHICLE CHARGING STATION, SEE PLANS FOR TYPES
- X RV — RECREATIONAL VEHICLE CHARGING STATION, SEE PLANS FOR TYPES
- X SP — MARINA SHORE POWER STATION, SEE PLANS FOR TYPES
- PP — POWER POLE - CEILING DOWN TO FLOOR
- J — J-BOX
- CONDUIT STUB THROUGH WALL BUSHED EACH END
- CABLE TRAY

## LIGHTING

### FIXTURES

- X — LIGHT FIXTURE - POLE MOUNTED; TAG = TYPE IN FIXTURE SCHEDULE
- X — LIGHT FIXTURE - CEILING MOUNTED FLUSH OR SURFACE; TAG = TYPE IN FIXTURE SCHEDULE
- X — WALL MOUNTED - FLUSH OR SURFACE; TAG = TYPE IN FIXTURE SCHEDULE
- X — LIGHT FIXTURE - FLUSH OR SURFACE; TAG = TYPE IN FIXTURE SCHEDULE
- X — EMERGENCY BATTERY UNIT LIGHT - WALL MOUNTED
- EMERGENCY BATTERY UNIT LIGHT - CEILING MOUNTED
- REMOTE EMERGENCY LIGHT

### EXITS

- EXIT LIGHT - ONE FACE
  LINES REPRESENT WALL MOUNTED. NO LINES REPRESENT CEILING MOUNTED, TYPICAL ALL EXITS
- EXIT LIGHT - ONE FACE WITH EMERGENCY HEADS
  ARROWS INDICATE DIRECTION TO EXIT
- EXIT LIGHT - TWO FACE

### SENSORS

- DUAL TECHNOLOGY OCCUPANCY SENSOR
  LINES REPRESENT WALL MOUNTED. NO LINES REPRESENT CEILING MOUNTED, TYPICAL ALL SENSORS
- MULTI DIRECTION DUAL TECHNOLOGY OCCUPANCY SENSOR
- U — ULTRASONIC OCCUPANCY SENSOR
- L — LONG THROW OCCUPANCY SENSOR
- HB — HIGH BAY OCCUPANCY SENSOR
- PC — PHOTOCELL
- DA — DAYLIGHT SENSOR

## FIRE ALARM PANELS & FIRE ALARM SUPPORT ITEMS

- FACP — FIRE ALARM CONTROL PANEL
- FAAP — FIRE ALARM ANNUNCIATOR PANEL, FLUSH OR SURFACE
- FAPS — FIRE ALARM POWER SUPPLY
- FACC — FIRE ALARM COMMAND CENTER WITH MICROPHONE
- FM — FIRE ALARM MICROPHONE ENCLOSURE, FLUSH OR SURFACE
- RTS — REMOTE TEST SWITCH

### NOTIFICATION APPLIANCE

TAGS X=CANDELA LEVEL, EX=EXPLOSION PROOF, WP=WATERPROOF

- X — WALL MOUNTED AUDIO (HORN) / VISUAL NOTIFICATION APPLIANCE
- X — WALL MOUNTED AUDIO (VOICE) / VISUAL NOTIFICATION APPLIANCE
- WALL MOUNTED FIRE ALARM SPEAKER
- S X — WALL MOUNTED VISUAL NOTIFICATION APPLIANCE
- X — CEILING MOUNTED AUDIO (HORN) / VISUAL NOTIFICATION APPLIANCE
- X — CEILING MOUNTED AUDIO (VOICE) /VISUAL NOTIFICATION APPLIANCE
- CEILING MOUNTED FIRE ALARM SPEAKER
- S X — CEILING MOUNTED VISUAL NOTIFICATION APPLIANCE
- F — WALL MOUNTED AUDIO (HORN) NOTIFICATION APPLIANCE
- B — WALL MOUNTED BELL
- MH — MAGNETIC DOOR HOLDER PER DETAIL 3/E3.1
- EH — ELECTRIC DOOR HOLDER
- DM — ADDRESSABLE CONTROL MODULE

## FIRE ALARM

### INITIATION DEVICE

- XX F — MANUAL PULL STATION; TAG XX, LC=LEXAN COVER, EX=EXPLOSION PROOF, WP=WEATHERPROOF
- G — GAS DETECTOR
- H — HEAT DETECTOR
- S — SMOKE DETECTOR
- S — SMOKE DETECTOR WITH SOUNDER BASE
- DS — DUCT SMOKE DETECTOR PER DETAIL 2/E3.1
- ES — ELEVATOR SMOKE DETECTOR PER DETAIL 5/E3.1
- CM — CARBON MONOXIDE DETECTOR
- FS — SPRINKLER FLOW SWITCH PER DETAIL 6/E3.1
- PS — SPRINKLER PRESSURE SWITCH
- TS — SPRINKLER TAMPER SWITCH PER DETAIL 6/E3.1
- ADDRESSABLE MONITOR MODULE

## COMMUNICATION

### DATA RACKS

TAGS, TCWX = TELECOMMUNICATION CLOSET NUMBER

- DATA RACK - OPEN FRAME 2 POST FLOOR MOUNTED
- DATA RACK - OPEN FRAME 4 POST FLOOR MOUNTED
- DATA RACK - OPEN FRAME WALL MOUNTED
- DATA RACK - ENCLOSED WALL MOUNTED
- GROUND BAR PER DETAIL 6/E4.0

### DATA / IP PHONE, WAP & TELEPHONE DEVICES

TAGS X, NO TAG=SINGLE DROP, NUMBER INDICATES NUMBER OF DROPS IN EACH LOCATION, A=ATTENDANCE SCANNER, C=IP SECURITY CAMERA, K=KEYLESS ENTRY, USB=UNIVERSAL SERIAL BUS, V=VAPE SENSOR, W=WALL PHONE, PROVIDE 4" SQUARE DEEP BOX, 1-GANG RING, 1" CONDUIT STUBBED INTO ACCESSIBLE CEILING.

- X — DATA / IP PHONE OUTLET (STANDARD HEIGHT)
- X — DATA / IP PHONE OUTLET (MOUNTED AT 42" TO CENTER)
- WAP X — WIRELESS ACCESS POINT DATA OUTLET PER DETAIL 3/E8.0
- CDF X — COILED DATA FOR FUTURE USE
- X X — DATA / TELEPHONE OUTLET (STANDARD HEIGHT)
- X X — DATA / TELEPHONE OUTLET (MOUNTED AT 42" TO CENTER)
- X — TELEPHONE OUTLET (STANDARD HEIGHT)
- X — TELEPHONE OUTLET (MOUNTED AT 42" TO CENTER)

### SOUND SYSTEM INPUTS PAGING/INTERCOM SYSTEM

TAGS XX, NO TAG=GENERAL PUBLIC ANNOUNCEMENTS, AS=AUDITORIUM, CL=CLASSROOM, CS=CAFETERIA, FH=FIELD HOUSE, FS=FITNESS, GS=GYM, IP=INFRASTRUCTURE, MP=MULTI-PURPOSE, PS=POOL, WS=WEIGHT ROOM

- XX A — AUXILIARY/INPUT SOUND JACK
- XX M — MICROPHONE OUTLET
- XX MC — ADMINISTRATION STATION SEE SOUND SYSTEM RISER
- XX V — VOLUME CONTROL
- C — WALL MOUNTED CALL-IN SWITCH BOX OR HANDSET

### SPEAKERS

TAGS XX, NO TAG=GENERAL PUBLIC ANNOUNCEMENTS, AS=AUDITORIUM, CL=CLASSROOM, CS=CAFETERIA, FH=FIELD HOUSE, FS=FITNESS, GS=GYM, IP=INFRASTRUCTURE, MP=MULTI-PURPOSE, PS=POOL, WS=WEIGHT ROOM, WP=WEATHER PROOF

- XX W — SPEAKER - WALL RECESSED MOUNTED
- XX W — SPEAKER - WALL SURFACE MOUNTED WITH METAL ENCLOSURE
- XX W — SPEAKER - WALL SURFACE MOUNTED - TWO SIDED
- XX W — SPEAKER - WALL BRACKET MOUNTED
- XX H — SPEAKER HORN - WALL SURFACE MOUNTED
- XX C — SPEAKER - CEILING RECESSED MOUNTED
- XX — SPEAKER - CEILING SURFACE MOUNTED WITH METAL ENCLOSURE
- XX — SPEAKER - CEILING SURFACE MOUNTED WITH METAL ENCLOSURE - TWO SIDED
- XX P — SPEAKER - PENDANT MOUNTED
- XX P — SPEAKER - PENDANT MOUNTED - TWO SIDED
- XX — SPEAKER LOUD - SUSPENDED

### PUBLIC ANNOUNCEMENT INDICATION DEVICE

- S — PUBLIC ANNOUNCEMENT INDICATION STROBE LIGHT - WALL MOUNTED
- S — PUBLIC ANNOUNCEMENT INDICATION STROBE LIGHT - CEILING MOUNTED

## COMMUNICATION

### CLOCKS

TAGS XX, NO TAG=ANALOG, DI=DIGITAL, DS=DIGITAL W/ DATA & W/SPEAKER, DSF=DIGITAL W/SPEAKER & FLASHER, DV=DIGITAL VIDEO DISPLAY WITH DATA, IP=INFRASTRUCTURE SYNCHRONIZED TIMEKEEPING SYSTEM VIA NETWORK TIME PROTOCOL WG=WIRE GUARD

- XX — CLOCK - WALL MOUNTED - SINGLE FACE
- XX — CLOCK - WALL MOUNTED - TWO FACE
- XX — CLOCK - CEILING SURFACE MOUNTED - TWO FACE
- CLOCK / SPEAKER BAFFLE - COMBINATION

### TECHNOLOGY

- D — HDMI OUTLET
- TV — COAX OUTLET
- CEILING MOUNTED PROJECTOR PLATE
- P — MEDIA DELIVERY INPUT
- PP — MEDIA DELIVERY LOCATION

### MISCELLANEOUS

- ARS — AREA OF REFUGE (CALL BOX) / TWO-WAY COMMUNICATION SYSTEM
- ARM — AREA OF REFUGE MASTER (BASE STATION) / TWO-WAY COMMUNICATION SYSTEM
- TWC — TWO-WAY COMMUNICATION (CALL BOX) PER DETAIL 2/E8.1
- TWCM — TWO-WAY COMMUNICATION MASTER (BASE STATION) PER DETAIL 2/E8.1

## SECURITY

### DOOR CONTROL

- AI — 2 WAY INTERCOM SYSTEM DOOR STATION
- AM — 2 WAY INTERCOM SYSTEM MASTER STATION
- AO — AUTOMATIC DOOR OPENER
- CA — COMBINATION CARD READER AND KEY PAD
- CR — CARD READER PER DETAIL 2/E5.1
- DB — DURESS BUTTON
- DC — DOOR POSITION SWITCH CONTACT
- DP — DOOR POSITION SWITCH
- EL — ELECTRIC TRIM LATCH PER DETAIL 2/E5.1
- ES — ELECTRIC DOOR STRIKE PER DETAIL 2/E5.1
- GB — GLASS BREAK SENSOR
- KP — KEY PAD
- LS — LIMIT SWITCH
- SM — SECURITY SYSTEM MOTION DETECTOR CEILING MOUNTED 360°
- HM — SECURITY SYSTEM MOTION DETECTOR - WALL MOUNTED
- ML — MAGNETIC LOCK
- P — PANIC PULL STATION
- RX — REQUEST TO EXIT
- SS — SECURITY STROBE
- SINGLE PUSH BUTTON - FUNCTIONALITY DEFINED ON PLANS

### CAMERAS

- SECURITY CAMERA - DIRECTIONAL
  LINES REPRESENT WALL OR POLE MOUNTED
  NO LINES REPRESENT CEILING MOUNTED (TYPICAL)
- SECURITY CAMERA - 90°
- SECURITY CAMERA - 180°
- SECURITY CAMERA - 270°
- SECURITY CAMERA - 360°

## HEALTHCARE

- NH — NURSE CALL LIGHT
- N — NURSE CALL BEDSIDE STATION
- NS — NURSE CALL STAFF ACKNOWLEDGE
- NS — NURSE CALL CONTROL ROOM STATION
- NCPS — NURSE CALL POWER SUPPLY

## GENERAL

### ANNOTATION SYMBOLS

- X — REFERS TO PLAN NOTE NUMBER
- XXX — SPECIALTY EQUIPMENT REFERENCE TAG
- XX / XXX — DETAIL NUMBER / SHEET NO. WHERE DETAIL IS LOCATED

### LINE LEGEND

- CIRCUIT HOMERUN TO 20A/1P BREAKER UNLESS SHOWN OTHERWISE ON DRAWINGS.
- INDICATES ITEMS CONNECTED TO SAME CIRCUIT BUT CONTROLLED SEPARATELY
- INDICATES ITEMS CONNECTED TO SAME CIRCUIT AND CONTROLLED TOGETHER

## ABBREVIATIONS

| | |
|---|---|
| AFF | ABOVE FINISH FLOOR |
| AFG | ABOVE FINISH GRADE |
| AHU | AIR HANDLING UNIT |
| ATS | AUTOMATIC TRANSFER SWITCH |
| C | CONDUIT |
| CCU | COOLING CONDENSER UNIT |
| CKT | CIRCUIT |
| CM | CEILING MOUNTED |
| CT | CURRENT TRANSFORMER |
| CON | CONTACTOR |
| CP | CONTROL PANEL |
| CUH | CABINET UNIT HEATER |
| DIS | DISCONNECT |
| DM | DEMARC |
| EB | EXISTNG BREAKER TO REMAIN UNLESS NOTED OTHERWISE |
| EBB | ELECTRICAL BASE BOARD |
| EC | ELECTRICAL CONTRACTOR |
| ECB | ENCLOSED CIRCUIT BREAKER |
| EDH | ELECTRIC DUCT HEATER |
| EF | EXHAUST FAN |
| EL | ELEVATION |
| EM | EMERGENCY |
| EQ | EQUIPMENT |
| EOL | END OF LINE RESISTOR |
| ETR | EXISTING TO REMAIN |
| EUH | ELECTRIC UNIT HEATER |
| EWC | ELECTRIC WATER COOLER |
| EWH | ELECTRIC WALL HEATER |
| EX | EXPLOSION PROOF |
| F | FUSIBLE PROOF |
| FBO | FURNISHED BY OTHERS |
| FC | VARIABLE SPEED FAN CONTROL SWITCH |
| FR | FREEZER |
| FSD | FIRE SMOKE DAMPER |
| GC | GENERAL CONTRACTOR |
| (GFI) | GROUND FAULT INTERRUPTER BREAKER LOCATED IN ELECTRICAL PANEL |
| GND | GROUND |
| GSP | GYM SWITCH PANEL |
| GUH | GAS UNIT HEATER |
| HF | HEAT FAN |
| HP | HORSE POWER |
| HVAC | HEATING, VENTILATING AND AIR CONDITIONING |
| IDF | INTERMEDIATE DISTRIBUTION FRAME |
| JB | JUNCTION BOX |
| MAU | MAKEUP AIR UNIT |
| MB | MOTORIZED BACKBOARD |
| MC | MECHANICAL CONTRACTOR |
| MDF | MAIN DISTRIBUTION FRAME |
| MOD | MOTOR OPERATED DAMPER |
| MTD | MOUNTED |
| MTS | MANUAL TRANSFER SWITCH |
| NL | NIGHT LIGHT |
| NIC | NOT IN CONTRACT |
| NTS | NOT TO SCALE |
| OHD | OVERHEAD DOOR |
| OC | ON CENTER |
| PDB | PUNCH DOWN BLOCK |
| PNL | PANELBOARD |
| PS | POWER SUPPLY |
| RCP | RELAY CONTROL PANEL |
| RTU | ROOF TOP UNIT |
| SAP | SECURITY ANNUNCIATOR PANEL |
| SCP | SECURITY CONTROL PANEL |
| SD | SMOKE DAMPER |
| SJB | SOUND SYSTEM JUNCTION BOX |
| SPD | SURGE PROTECTION DEVICE |
| SS | STAINLESS STEEL |
| SV | SOLENOID VALVE |
| TC | TELECOMMUNICATION CLOSET |
| TCP | TEMPERATURE CONTROL PANEL |
| TGB | TELECOMMUNICATION GROUND BAR MASTER |
| TJB | TERMINAL JUNCTION BOX |
| TRN | TRANSFORMER |
| TRS | TRANSFER SWITCH |
| TTB | TELEPHONE TERMINAL BOARD |
| UC | UNDERCOUNTER |
| UPS | UNINTERRUPTED POWER SUPPLY |
| UH | UNIT HEATER |
| VFD | VARIABLE FREQUENCY DRIVE |
| W | WELDER |
| WE | WALL EXHAUST |
| WG | WIRE GUARD |
| WH | WATER HEATER |
| WP | WEATHER PROOF |
| WP/GFI | WEATHER PROOF / GROUND FAULT INTERRUPTER |
| XP | CLASS 1, DIV. 1 EQUIPMENT |

## ELECTRICAL SHEET LIST

| Sheet Number | Sheet Name |
|---|---|
| E0.0 | ELECTRICAL SYMBOLS AND ABBREVIATIONS |
| E0.1 | ELECTRICAL SITE PLAN |
| E1.0 | OVERALL FLOOR PLANS |
| E1.1AL | UNIT A - FIRST FLOOR PLAN - LIGHTING |
| E1.1AP | UNIT A - FIRST FLOOR PLAN - POWER |
| E1.1BL | UNIT B - FIRST FLOOR PLAN - LIGHTING |
| E1.1BP | UNIT B - FIRST FLOOR PLAN - POWER |
| E1.2AL | UNIT A - SECOND FLOOR PLAN - LIGHTING |
| E1.2AP | UNIT B - SECOND FLOOR PLAN - POWER |
| E1.3.AL | UNIT A - THIRD FLOOR PLAN - LIGHTING |
| E1.3AP | UNIT A - THIRD FLOOR PLAN - POWER |
| E2.0 | ENLARGED KITCHEN PLAN & SCHEDULES |
| E3.0 | FLOOR PLANS - FIRE ALARM |
| E3.1 | FIRE ALARM DETAILS |
| E4.0 | DATA ROUTING PLANS AND DETAILS |
| E4.1 | LOCAL AREA NETWORK RISER |
| E5.0 | FLOOR PLANS - SECURITY |
| E5.1 | SECURITY DETAILS |
| E6.0 | ONE LINE DIAGRAM |
| E6.1 | PANEL SCHEDULES |
| E7.0 | FIXTURE & ELECTRICAL MOTOR STARTER SCHEDULE |
| E8.0 | ELECTRICAL DETAILS |
| E8.1 | ELECTRICAL DETAILS |


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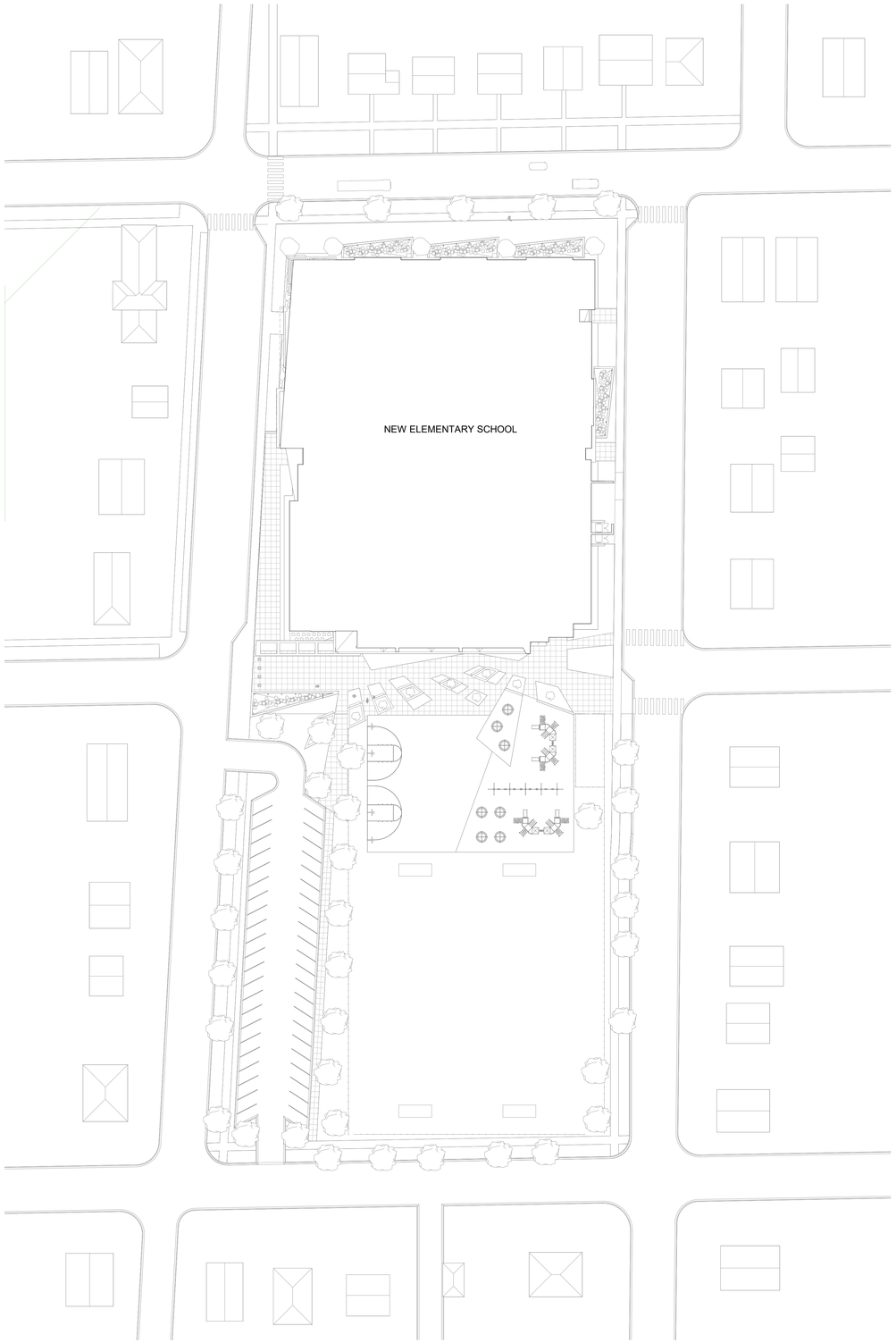
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4A E0.1 ELECTRICAL SITE PLAN
Scale: 1" = 40'-0"

## GENERAL NOTES

1. UNLESS SHOWN OTHERWISE, ALL WIRING SHOWN IS (2)#10 AND #10 GROUND IN 1" PVC.
2. INSTALL PULL CORD IN ALL EMPTY CONDUITS.
3. PROVIDE CONCRETE BASE ROUGH-IN FOR ALL TYPE _____ FIXTURES PER DETAIL 4/E8.0.
4. UNLESS SHOWN OTHERWISE, ALL CONDUITS BURIED 2'-6" BELOW FINISHED GRADE.
5. PROVIDE SITE LIGHTING CONTROL PANEL PER DETAIL 5/E8.0.
6. BRING SITE EXCAVATION AND TOPPING BACK TO ORIGINAL CONDITION IF TRENCHING IS DONE ON COMPACTED SURFACES.
7. ALL CONDUITS STUBBED OUT OF BUILDINGS SHALL BE DONE PER DETAIL 2/E8.0.

## PLAN NOTES (X)

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**ELECTRICAL SITE PLAN**

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**E0.1**

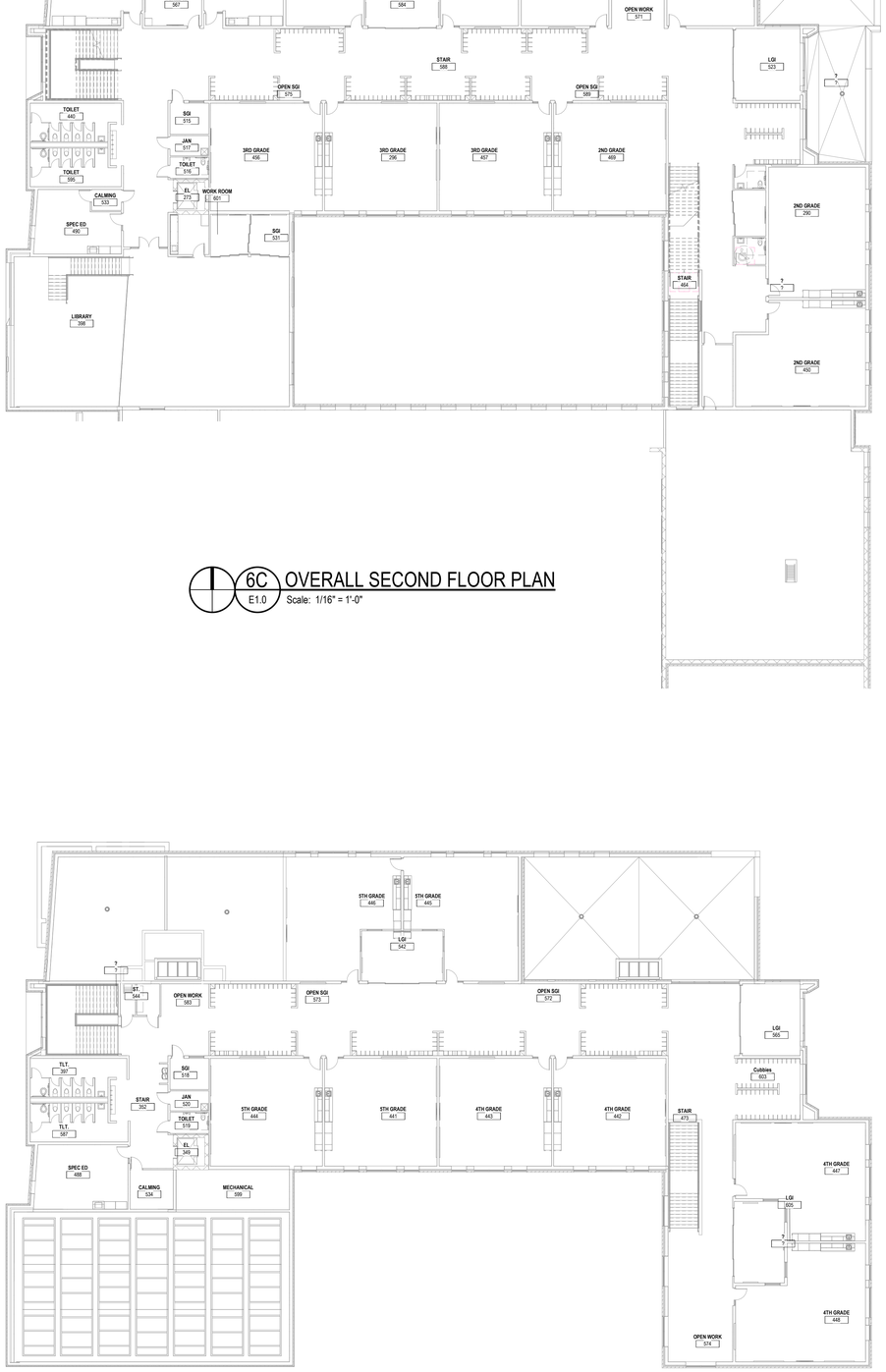
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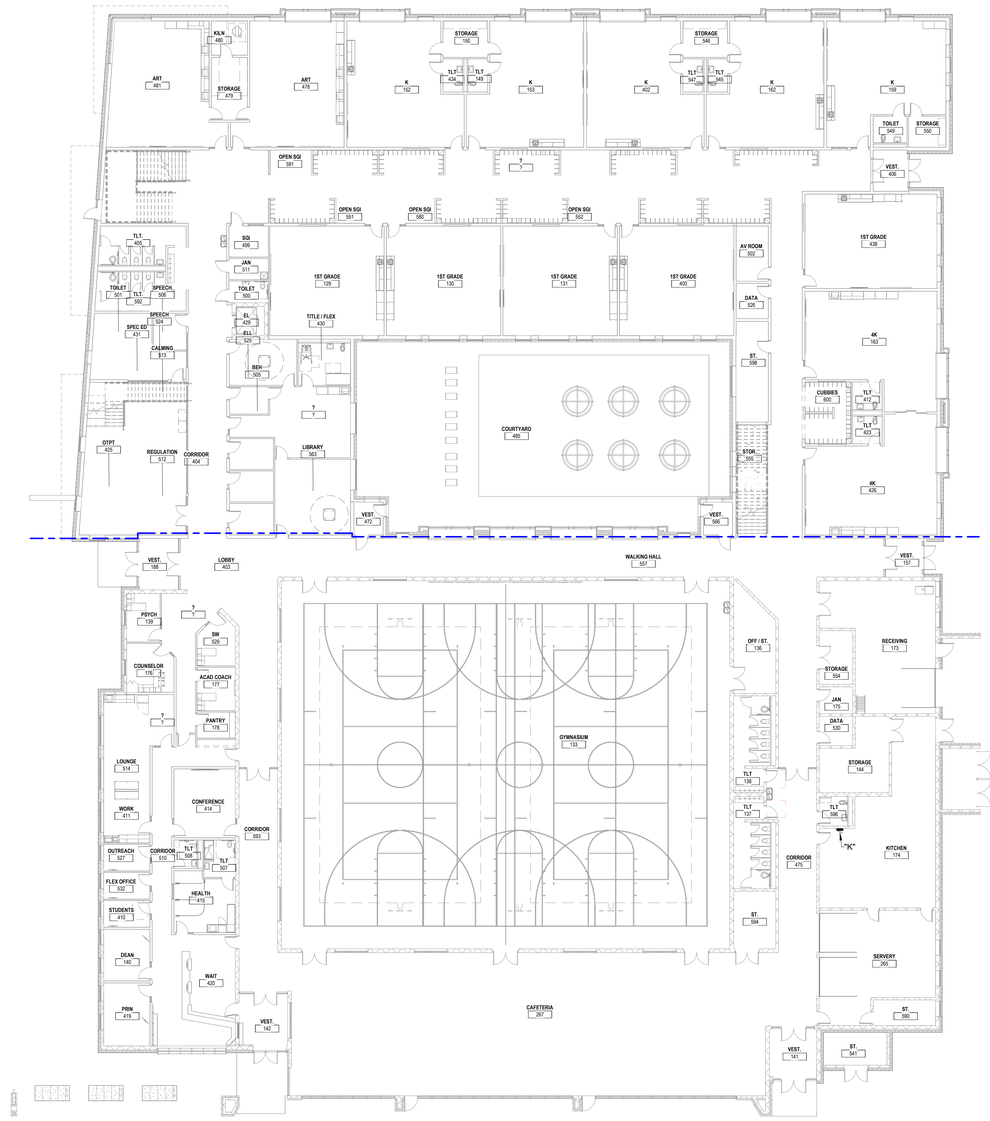
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6C OVERALL SECOND FLOOR PLAN
E1.0 Scale: 1/16" = 1'-0"

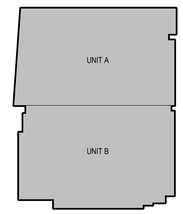



6A OVERALL THIRD FLOOR PLAN
E1.0 Scale: 1/16" = 1'-0"




2B OVERALL FIRST FLOOR PLAN
E1.0 Scale: 1/16" = 1'-0"

PLAN NOTES (X)




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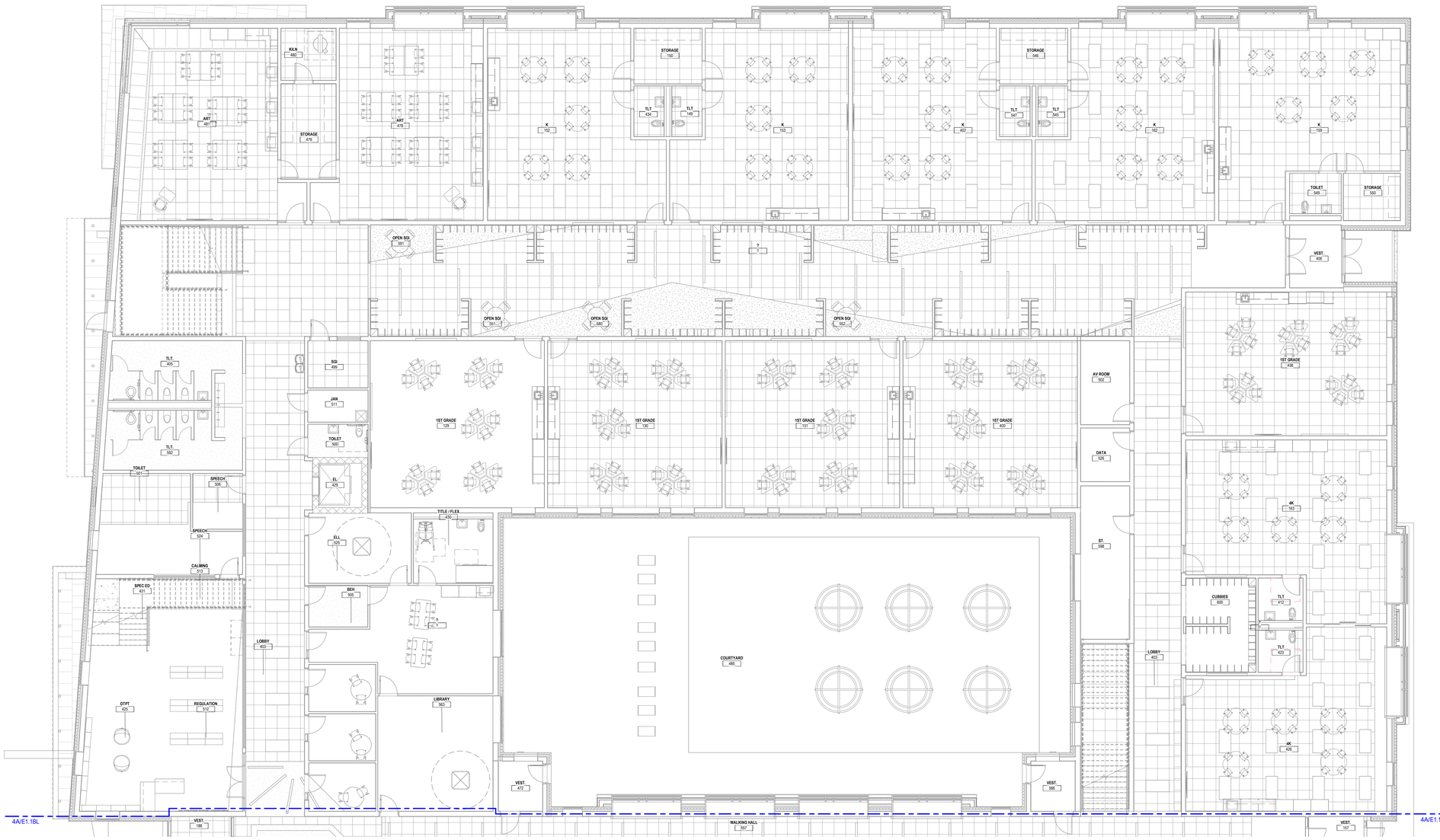


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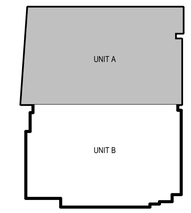
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6. SEE ARCHITECTURAL REFLECTED CEILING PLANS FOR EXACT LOCATION OF CEILING MOUNTED LIGHT FIXTURES.

## PLAN NOTES (X)




4A / E1.1AL UNIT A - FIRST FLOOR PLAN - LIGHTING
Scale: 1/8" = 1'-0"




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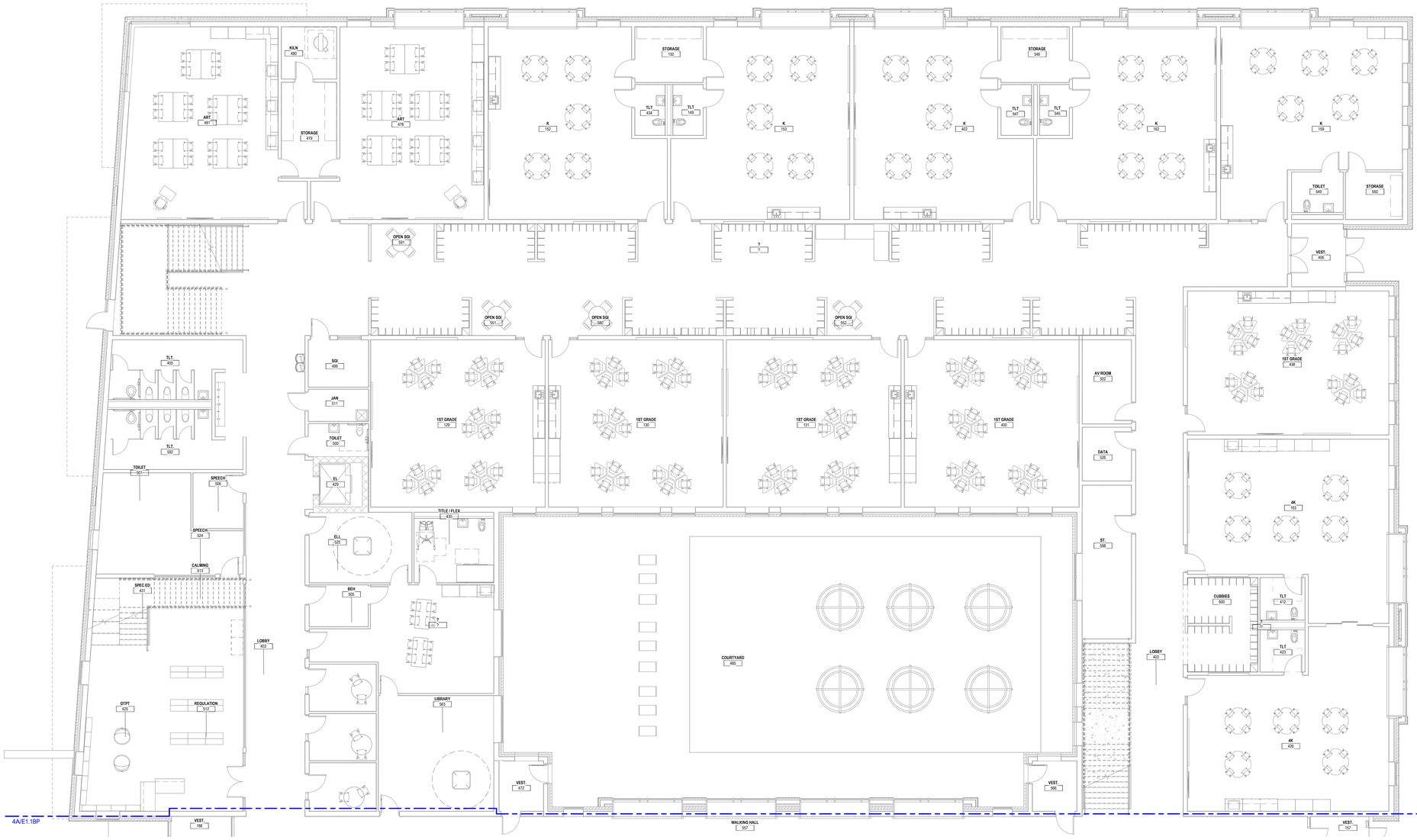


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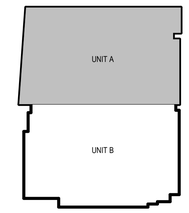
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## PLAN NOTES (X)




4A / E1.1AP UNIT A - FIRST FLOOR PLAN - POWER
Scale: 1/8" = 1'-0"




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Sheet Number: **E1.1AP**

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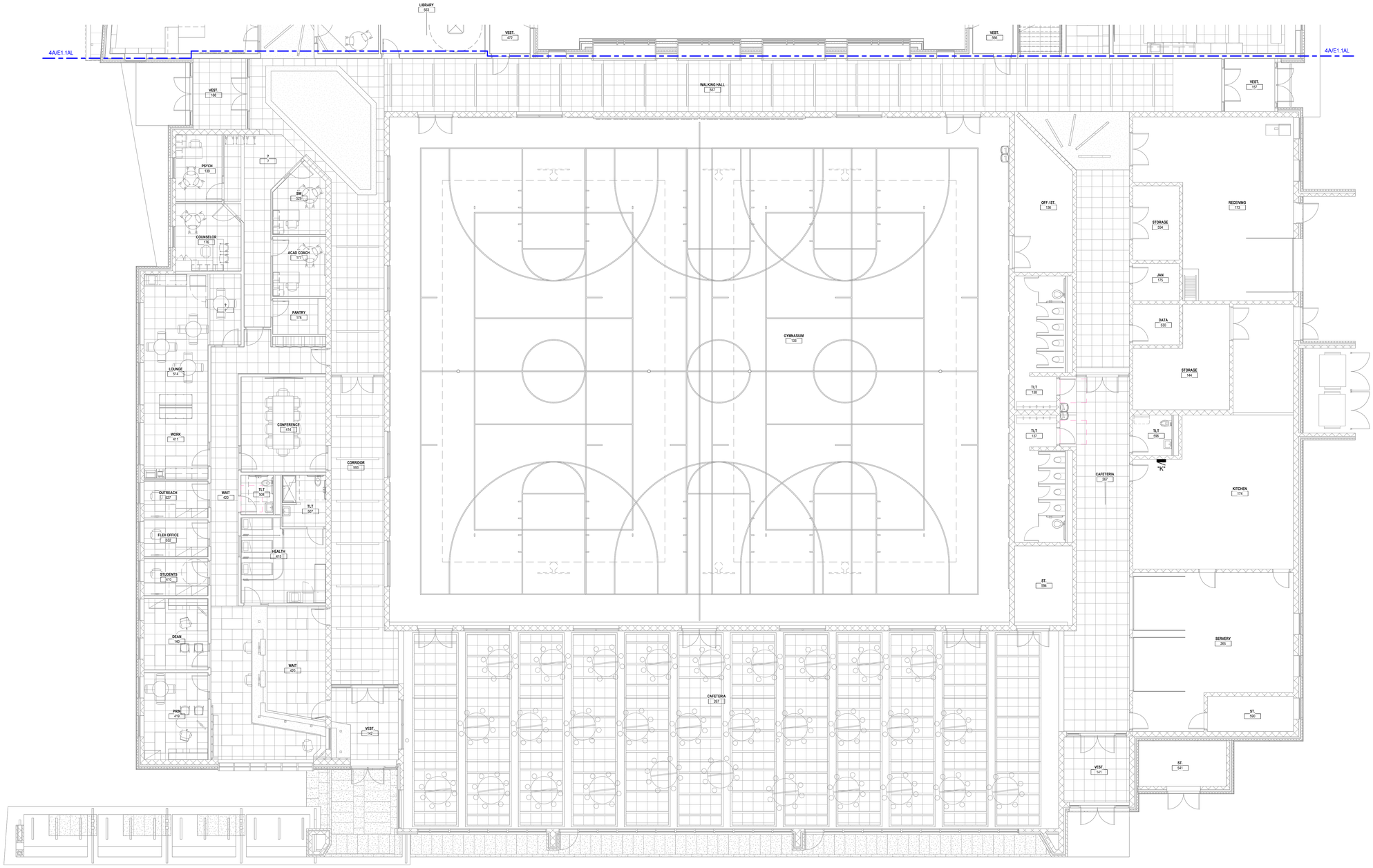


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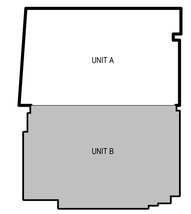
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## PLAN NOTES (X)




4A E1.1BL UNIT B - FIRST FLOOR PLAN - LIGHTING
Scale: 1/8" = 1'-0"




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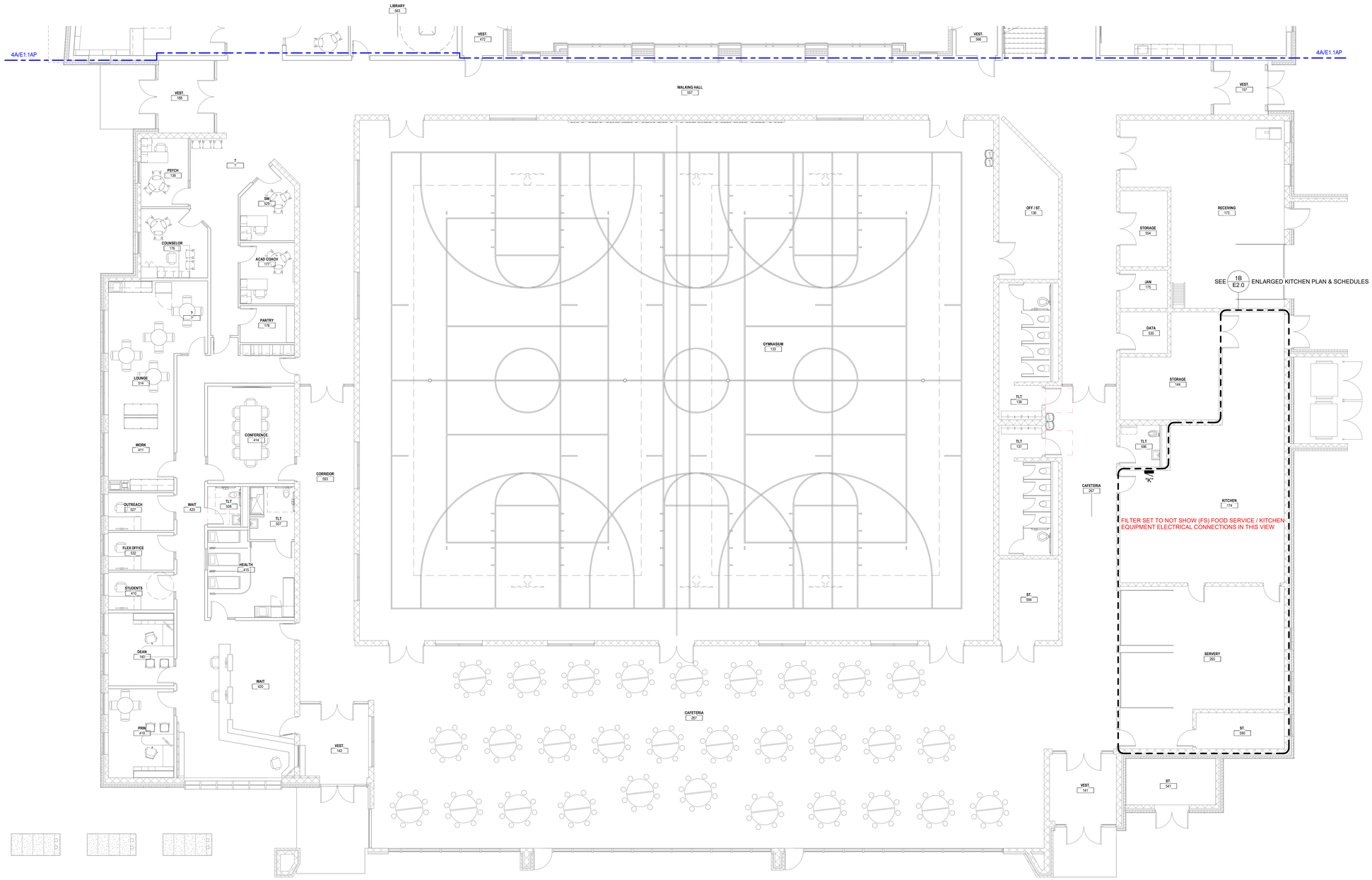


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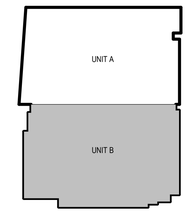
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## PLAN NOTES (X)




4A/E1.1BP UNIT B - FIRST FLOOR PLAN - POWER
Scale: 1/8" = 1'-0"




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TYPICAL CLASSROOM LIGHTING LAYOUT



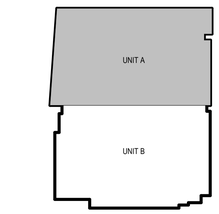

5A / E1.2AL UNIT A - SECOND FLOOR - LIGHTING

Scale: 1/8" = 1'-0"

## GENERAL NOTES

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## PLAN NOTES (X)




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**UNIT A - SECOND FLOOR PLAN - LIGHTING**

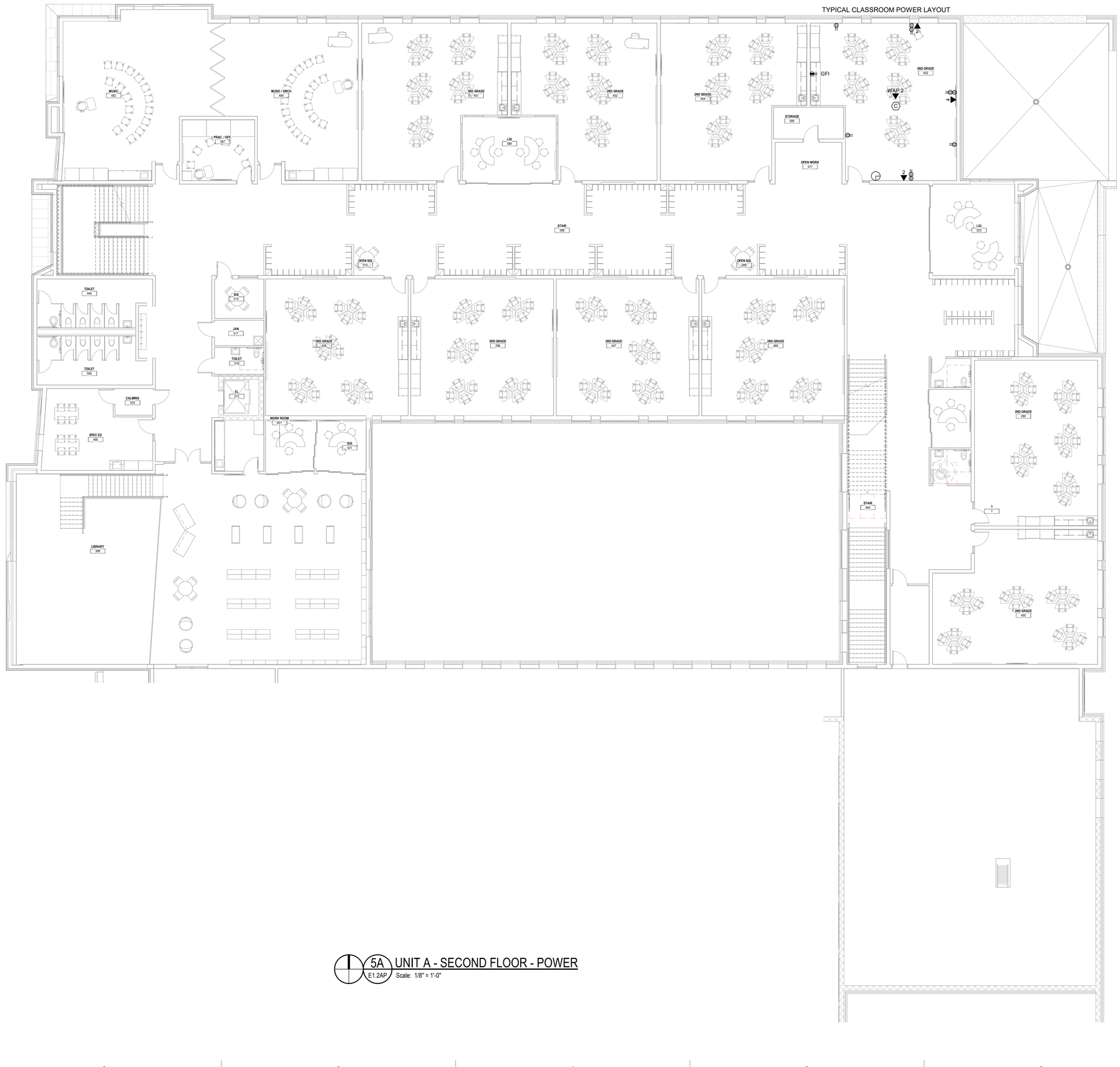
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TYPICAL CLASSROOM POWER LAYOUT



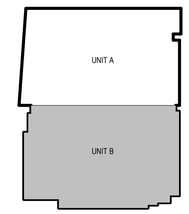

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## PLAN NOTES (X)

5A / E1.2AP UNIT A - SECOND FLOOR - POWER

Scale: 1/8" = 1'-0"




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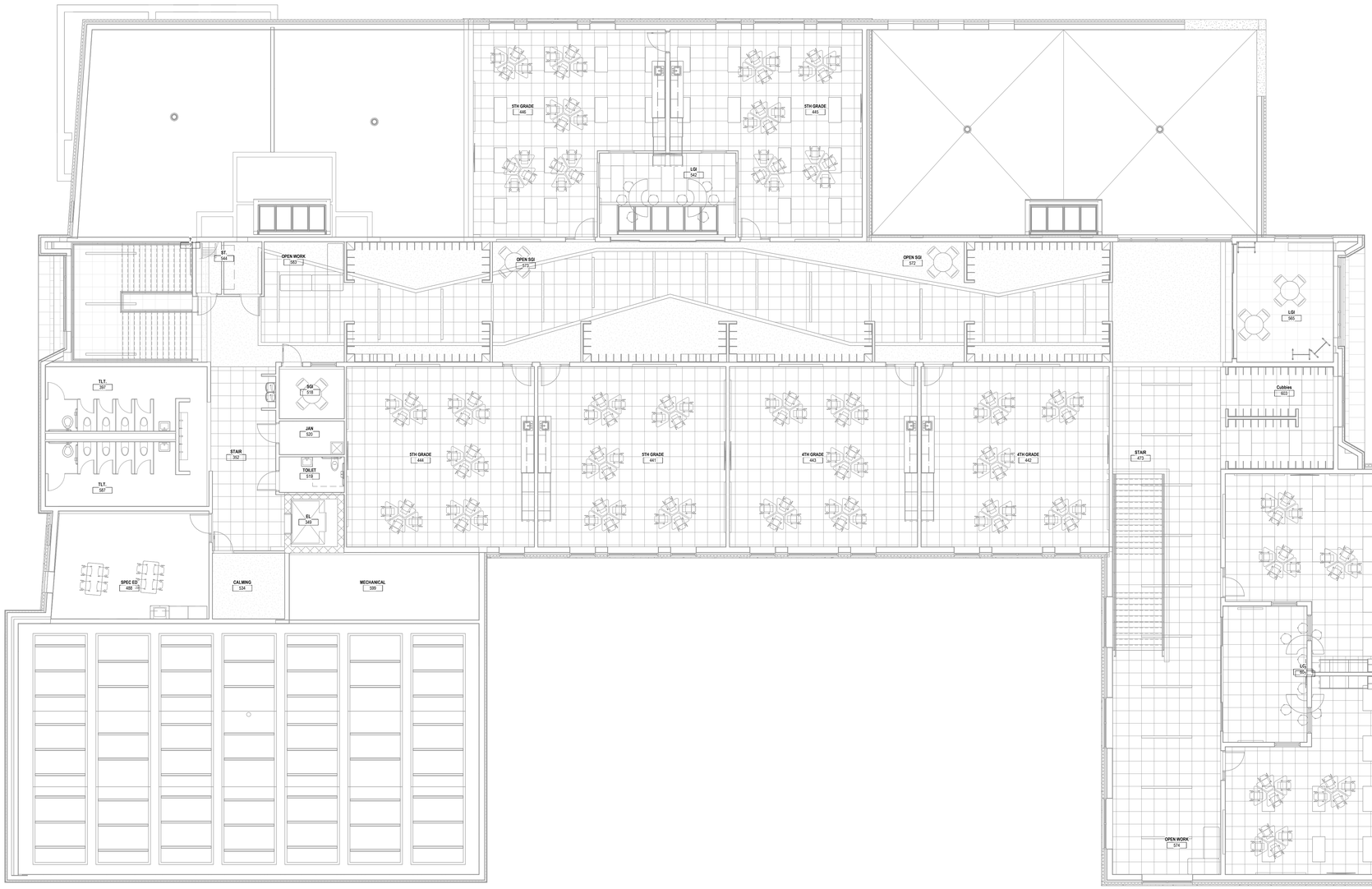


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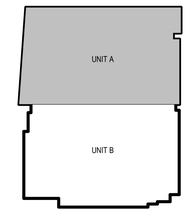
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## PLAN NOTES (X)




4A / E1.3.AL UNIT A - THIRD FLOOR - LIGHTING
Scale: 1/8" = 1'-0"




PLAN NORTH

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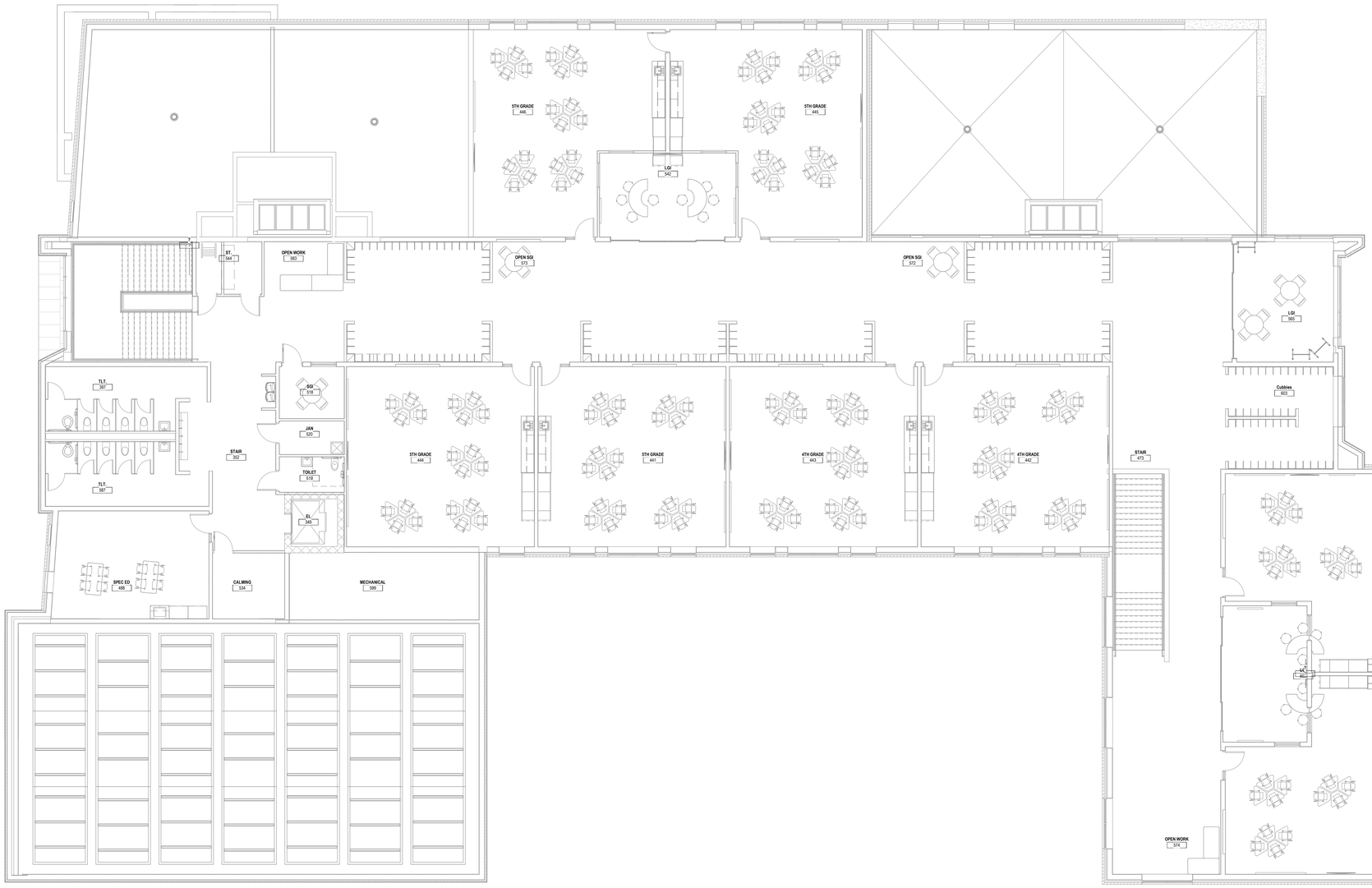


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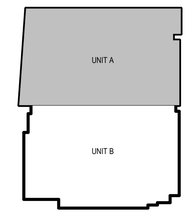
## GENERAL NOTES

1. PROVIDE EXTERIOR RECEPTACLES PER DETAIL 7/E8.0.
2. ALL CONDUITS STUBBED OUT OF BUILDING SHALL BE DONE PER DETAIL 2/E8.0.
3. CONFIRM EXACT LOCATION OF ALL OUTLETS IN OFFICES AND IMC WITH OWNER PRIOR TO ROUGH-IN.
4. ALL RECEPTACLES FOR ELECTRIC WATER COOLERS (EWC) TO BE ROUGHED-IN BEHIND UNIT. CONFIRM EXACT LOCATION PRIOR TO ROUGH-IN. CONFIRM RECEPTACLE OR DIRECT CONNECTION.
5. ALL RECEPTACLES MOUNTED WITHIN 6 FEET OF SINKS SHALL BE GFI TYPE.
6. ALL LOW VOLTAGE CABLING IN AREAS WITH EXPOSED STRUCTURE SHALL BE IN CONDUIT.

## PLAN NOTES (X)




4A / E1.3AP UNIT A - THIRD FLOOR - POWER
Scale: 1/8" = 1'-0"




PLAN NORTH

KEY PLAN

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**La Crosse School District**
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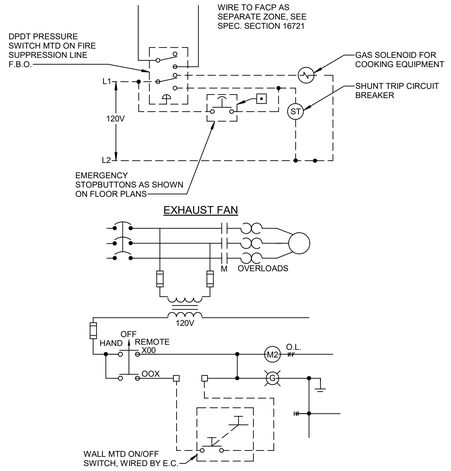
Sheet Title:
**UNIT A - THIRD FLOOR PLAN - POWER**

Sheet Number:
**E1.3AP**

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OPERATION

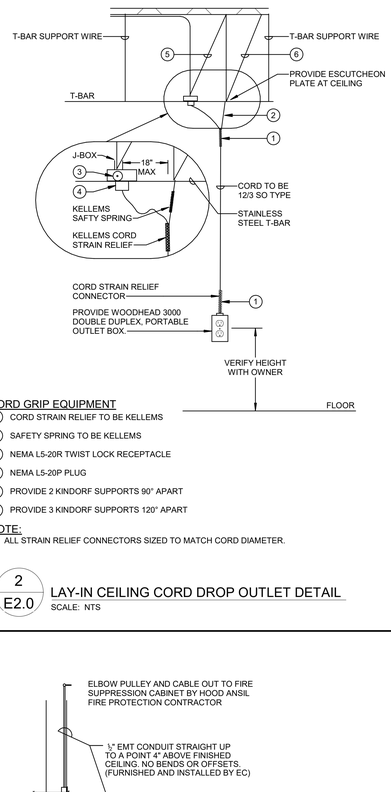
1. WHEN FIRE SUPPRESION ACTIVATES, PRESSURE SWITCH CHANGES POSITION. CIRCUIT BREAKERS TRIP AND GAS SOLENOID CLOSES. EXHAUST FAN CONTINUES TO OPERATE.

NOTES:

1. _____ = FIELD WIRING BY E.C.
2. SEE SPECIFICATIONS FOR SUPPLIER OF STARTERS.
3. EMERGENCY STOP BUTTON SHALL BE HEAVY DUTY OIL TYPE, FLUSH MOUNTED, RED MUSHROOM HEAD.

1 / E2.0 KITCHEN HOOD WIRING DIAGRAM

SCALE: NTS




CORD GRIP EQUIPMENT

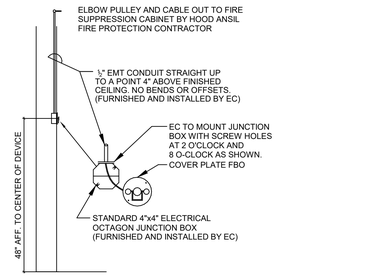
1. CORD STRAIN RELIEF TO BE KELLEMS
2. SAFETY SPRING TO BE KELLEMS
3. NEMA L5-20R TWIST LOCK RECEPTACLE
4. NEMA L5-20P PLUG
5. PROVIDE 2 KINDORF SUPPORTS 90° APART
6. PROVIDE 3 KINDORF SUPPORTS 120° APART

NOTE:

1. ALL STRAIN RELIEF CONNECTORS SIZED TO MATCH CORD DIAMETER.

2 / E2.0 LAY-IN CEILING CORD DROP OUTLET DETAIL

SCALE: NTS




MOUNT MANUAL PULL SWITCH MINIMUM 10'-0" FROM EXHAUST HOOD. MUST BE LOCATED IN PATH OF EGRESS. VERIFY LOCATION WITH LOCAL FIRE AUTHORITY.

3 / E2.0 ANSUL FIRE SUPPRESSION PULL SWITCH DETAIL

SCALE: NTS

FOOD SERVICE EQIPMENT SCHEDULE (FROM FOOD SERVICE VENDOR)

FOOD SERVICE ELECTRICAL CONNECTION SCHEDULE

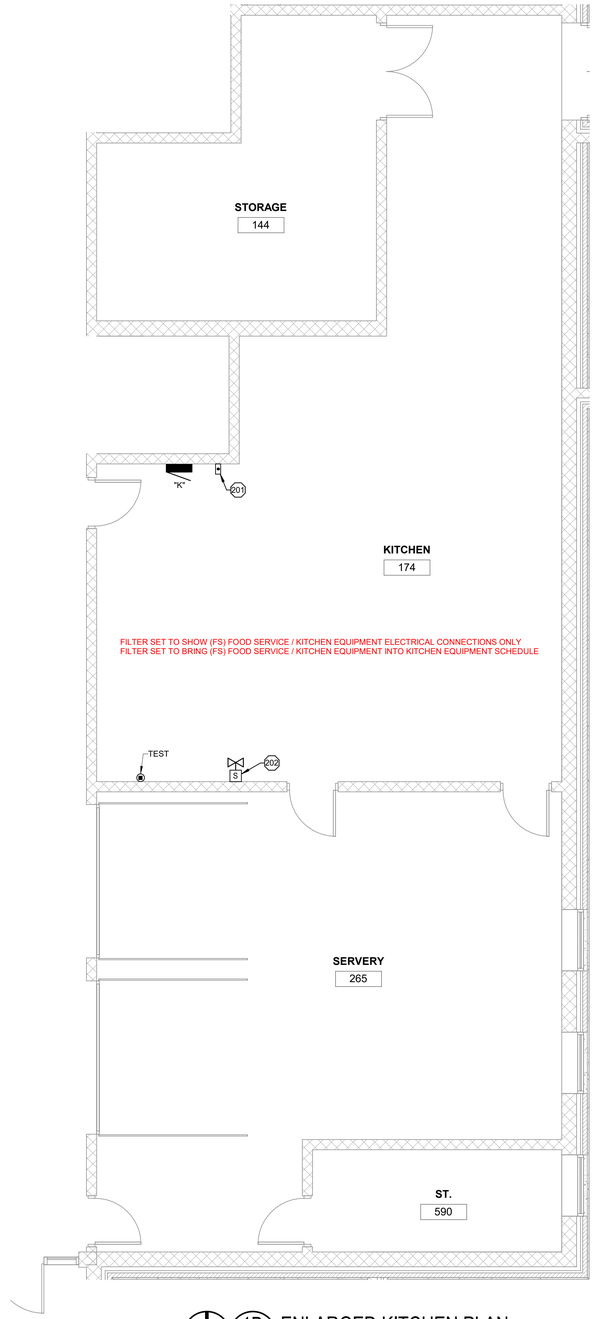
| LOAD NAME | MCA | BREAKER | VOLTAGE | POLE | PANEL | WIRE CONDUIT | REMARKS |
|---|---|---|---|---|---|---|---|
| TEST (FS) | 1.5 A | 20 A | 120 V | 1 | K | (2) #12, #12G IN 3/4"C | |

GENERAL NOTES

1. PROVIDE TAMPER PROOF, GFI TYPE, 120 VOLT, 20 AMP RECEPTACLES.
2. REFER TO KITCHEN PLANS AS PRTT OF THIS BID PACHAGE FOR ADDITIONAL CONNECTION REQUIREMENTS AND LOCATIONS.

REMARKS NOTES

1. FIRE SUPPRESSION SYSTEM, SEE DETAIL 1/E2.0
2. PROVIDE SHUNT TRIP BREAKER AND WIRE THRU ANSUL FIRE SUPPRESSION SYSTEM AS SHOWN PER DETAIL 1/E2.0 .
3. PROVIDE DIRECT CONNECTION TO UNIT.
4. PROVIDE GFI BREAKER.




FILTER SET TO SHOW (FS) FOOD SERVICE / KITCHEN EQUIPMENT ELECTRICAL CONNECTIONS ONLY
FILTER SET TO BRING (FS) FOOD SERVICE / KITCHEN EQUIPMENT INTO KITCHEN EQUIPMENT SCHEDULE

1B / E2.0 ENLARGED KITCHEN PLAN

Scale: 1/4" = 1'-0"

GENERAL NOTES

1. REFER TO FOOD SERVICE EQUIPMENT SCHEDULE AND FOOD SERVICE ELECTRICAL CONNECTION SCHEDULE FOR FOOD SERVICE EQUIPMENT ELECTRICAL REQUIREMENTS. TYPICAL OF ALL NUMBERED FOOD SERVICE ROUGH-IN TAGS.
2. VERIFY ALL PLUG AND RECEPTACLE TYPES WITH KITCHEN SUPPLIER AND FOOD SERVICE CONTRACTOR PRIOR TO ROUGH-IN.
3. VERIFY EXACT MOUNTING HEIGHTS OF ALL RECEPTACLES AND EQUIPMENT CONNECTIONS WITH FOOD SERVICE DRAWINGS PRIOR TO ROUGH-IN.
4. SEE FOOD SERVICE KITCHEN EQUIPMENT DRAWINGS FOR ADDITIONAL ELECTRICAL INFORMATION AND REQUIREMENTS.
5. ALL CONNECTIONS TO DISPOSERS, BOOSTER HEATERS AND DISHWASHERS TO BE SEAL TIGHT WITH WATER TIGHT CONNECTIONS. E.C. TO PROVIDE 4X STAINLESS STEEL WASH DOWN RATED DISCONNECT SWITCHES.
6. ALL WIRING TO BE #12 UNLESS SHOWN OTHERWISE. ALL WIRING FOR KITCHEN EQUIPMENT TO BE COPPER.
7. ELECTRICAL CONTRACTOR IS TO SUPPLY AND INSTALL ALL CONDUIT, WIRING, CORDS, BOXES, SWITCHES, FACEPLATES, BRACKETS, ETC. TO COMPLETE INSTALLATION. ELECTRICAL CONTRACTOR SHALL ALSO MAKE ALL FINAL CONNECTIONS TO FOOD SERVICE EQUIPMENT.
8. PROVIDE STAINLESS STEEL PLATES FOR WIRING DEVICES ON WALL UNDER EXHAUST HOOD.
9. SEE 1/8" SCALE FLOOR PLANS FOR ADDITIONAL ELECTRICAL EQUIPMENT IN KITCHEN SPACES.

PLAN NOTES (X)

| | |
|---|---|
| 201 | E.C. TO PROVIDE BOX MOUNTED AT 48"AFF TO CENTER FOR ANSUL PULL STATION. COORDINATE INSTALLATION WITH FOOD SERVICE EQUIPMENT CONTRACTOR. SEE DETAIL . 3/E2.0 |
| 202 | PROVIDE WIRING FROM EXHAUST HOOD FIRE SUPPRESSION SYSTEM TO GAS SOLENOID VALVE TO SHUT OFF GAS SUPPLY WHERE FIRE SUPPRESSION SYSTEM IS ACTIVATED, SEE DETAIL . 3/E2.0 |

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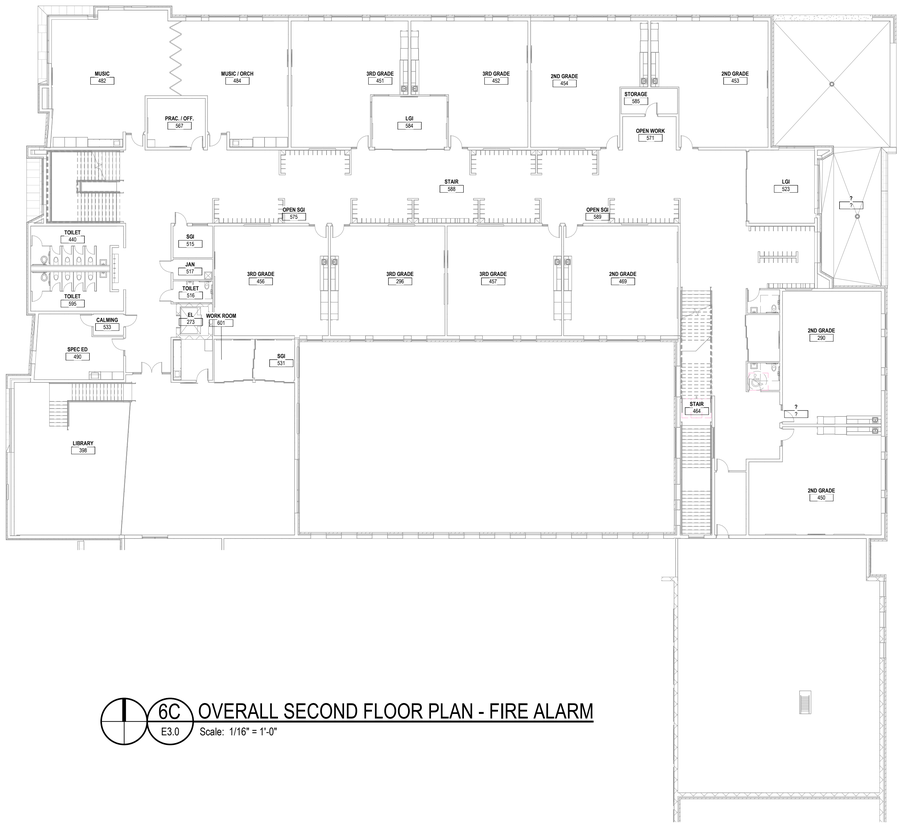
Sheet Title: ENLARGED KITCHEN PLAN & SCHEDULES

Sheet Number: E2.0

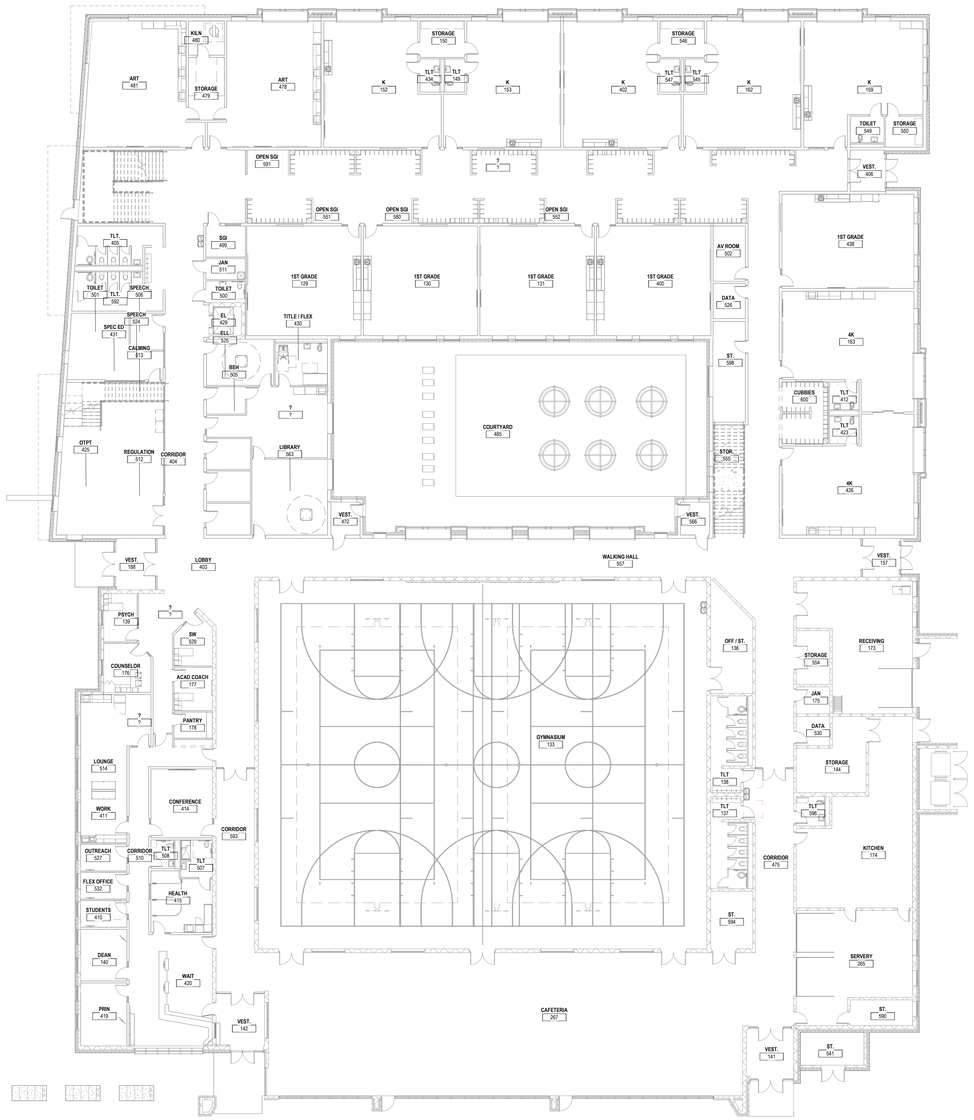
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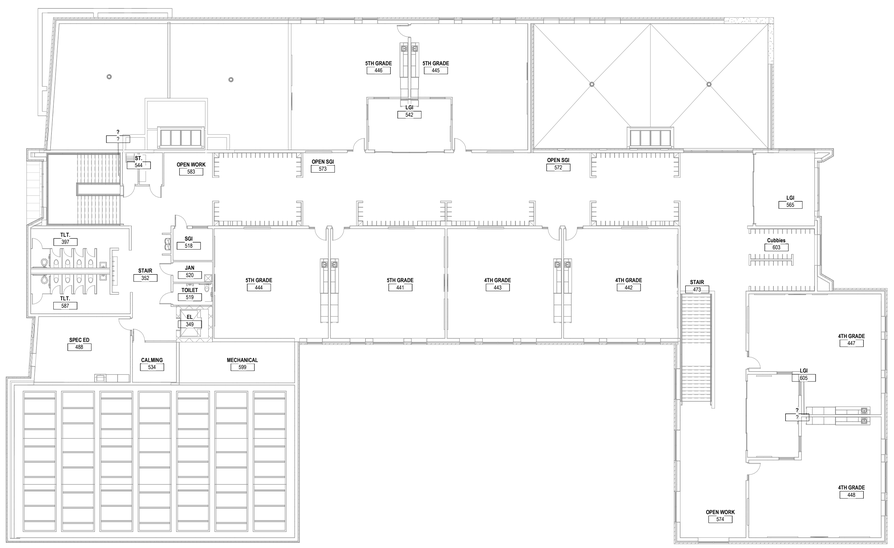
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6C E3.0 OVERALL SECOND FLOOR PLAN - FIRE ALARM
Scale: 1/16" = 1'-0"




2B E3.0 OVERALL FIRST FLOOR PLAN - FIRE ALARM
Scale: 1/16" = 1'-0"

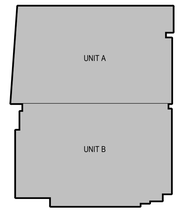



6A E3.0 OVERALL THIRD FLOOR PLAN - FIRE ALARM
Scale: 1/16" = 1'-0"

## GENERAL NOTES

1. SEE POWER PLANS FOR FIRE ALARM CONTROL PANEL AND POWER SUPPLY CIRCUITS.
2. ALL DEVICES SHOWN THIS SHEET ARE APPROXIMATE LOCATIONS. VERIFY EXACT LOCATIONS ON SITE TO ACCOMMODATE FURNITURE, LIGHT FIXTURES, ETC. ENSURE STROBE COVERAGE REQUIRED BY NFPA 72 IS MAINTAINED IF STROBES ARE RELOCATED FOR COORDINATION.
3. ALL FIRE ALARM CABLING SHALL BE SUPPORTED SEPARATELY FROM OTHER LOW VOLTAGE CABLING ABOVE LAY-IN CEILINGS. FIRE ALARM WIRING IN AREAS WHERE THERE IS NO LAY-IN CEILINGS SHALL BE IN CONDUIT.
4. SEE DETAIL 1 / E3.1 FOR INSTALLATION DETAILS.
5. LOCATE SMOKE DETECTOR MINIMUM 3'-0" FROM HVAC DIFFUSERS. ENSURE COVERAGE IS MAINTAINED IF SMOKES ARE RELOCATED FOR COORDINATION.
6. SEE ARCHITECTURAL EXTERIOR ELEVATIONS FOR MOUNTING HEIGHTS OF EXTERIOR FIRE ALARM HORN AND STROBE DEVICES.
7. ALL DEVICES IN MECHANICAL ROOMS SHALL BE LAID OUT ON-SITE AND DETERMINED BY THE MECHANICAL EQUIPMENT IN THE ROOM.
8. ALL FIRE ALARM PULL, JUNCTION BOXES, AND THEIR COVERS SHALL BE PAINTED RED AND HAVE "FIRE ALARM" WRITTEN ON THE COVER IN LARGE BLACK NON-WASHABLE INK. THE LETTERING SHALL BE SUCH THAT IT CAN BE READ FROM 10' AWAY. NOTE THAT THIS REQUIREMENT IS IN ADDITION TO THE NEC REQUIREMENTS WHICH REQUIRE THAT THE BOX ITSELF BE MARKED IN RED.

PLAN NOTES (X)




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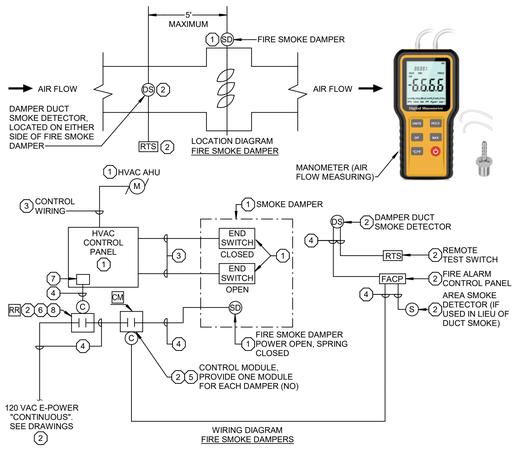
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Sheet Number: E3.0

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GENERAL NOTES:

1. MOUNT REMOTE TEST STATION ON NEAREST WALL THAT PROVIDES ACCESS. IN INSTALLATIONS WHERE TWO OR MORE FIRE SMOKE DAMPERS ARE INSTALLED TOGETHER, MOUNT ALL TEST SWITCHES IN ONE AREA.
2. PROVIDE WIRING PER MANUFACTURE SPECIFICATIONS.
3. AT EACH RETURN AIR DUCT SMOKE DETECTOR AND/OR DETECTOR SERVING A SMOKE DAMPER, MEASURE AND RECORD THE PRESSURE DIFFERENTIAL AT EACH DEVICE TO VERIFY COMPLIANCE WITH THE UNIT MANUFACTURER REQUIREMENTS FOR PROPER AIRFLOW. ALL PRESSURE DIFFERENTIAL READINGS SHALL BE COMPILED IN ONE TABLE AND SUBMITTED WITH THE O&M MANUAL AT THE END OR THE PROJECT. PROVIDE THESE READINGS TO THE LOCAL AHJ AND/OR FIRE INSPECTOR UPON REQUEST PRIOR TO THE SUBMITTAL OF THE O&M MANUAL.

2 E3.1 FIRE ALARM DUCT DETECTOR INSTALLATION FOR FIRE SMOKE DAMPER POWER OPEN SPRING CLOSED

SCALE: NTS

**SYSTEM OPERATION**

**NORMAL OPERATION OF THE AHU:**

1. WHEN THE HVAC SYSTEM IS CALLING FOR THE "AHU TO OPERATE. THE TEMPERATURE CONTROL PANEL SHALL PROVIDE A CONTACT "CLOSURE" SIGNAL TO FIRE SMOKE DAMPER(S) THROUGH THE RIB RELAY.
2. ONCE THE FIRE SMOKE DAMPER IS IN THE FULLY OPEN POSITION, THE END SWITCH OF THE SMOKE DAMPER WILL SEND A SIGNAL TO THE TEMPERATURE CONTROL PANEL STATING IT IS FULLY OPEN THUS ALLOWING THE AHU TO OPERATE.
3. WHEN THE AHU IS TURNED OFF THE TEMPERATURE CONTROL PANEL WILL TURN OFF THE "AHU WILL OPEN" THE CONTACT, OF THE RIB RELAY, ALLOWING THE FIRE SMOKE DAMPER TO CLOSE MANUALLY VIA SPRING RETURN.

**DUCT SMOKE DETECTOR SENSES SMOKE:**

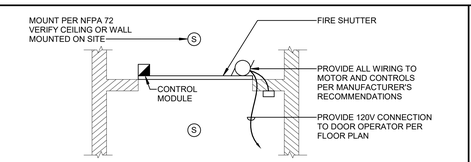
1. UNDER NORMAL OPERATION WHERE THE DUCT SMOKE DETECTOR DOES NOT SENSE SMOKE THE FIRE ALARM CONTROL PANEL SHALL CLOSE THE NORMALLY "OPEN" CONTACT (FAIL SECURE) OF THE FIRE ALARM CONTROL MODULE ALLOWING THE AHU TO OPERATE NORMALLY AS STATED ABOVE. THE TEMPERATURE CONTROL PANEL WILL OPEN THE SMOKE DAMPER THROUGH THE RIB RELAY WHICH WILL FEED ALL OF THE CONTROL MODULES ASSOCIATED WITH THE AHU. NOTE THERE WILL BE ONE CONTROL MODULE FOR EACH FIRE SMOKE DAMPER. WHEN MULTIPLE CONTROL MODULES ARE USED CONNECT THE 120 VOLT CONTACTS IN PARALLEL BETWEEN THE RIB RELAY AND THE SMOKE FIRE DAMPER(S).
2. IF THE DUCT SMOKE DETECTOR SENSES SMOKE IT WILL THEN DE-ENERGIZE OR "OPEN" ALL OF THE CONTROL MODULES FOR THE SMOKE DAMPERS ASSOCIATED WITH THIS AHU. IF MULTIPLE CONTROL MODULES ARE SHOWN CONNECT THEM IN SERIES.

**NOTES:**

1. THE FIRE SMOKE DAMPER SHALL BE CLOSED ANY TIME THE AHU IS NOT IN OPERATION.
2. *THE ABOVE REFERENCES "AHU" REFERENCES DIFFERENT PIECES OF HVAC EQUIPMENT. SEE DRAWINGS FOR ACTUAL HVAC DEVICE NAME TAG.

PLAN NOTE: (X)

1. EQUIPMENT PROVIDED AND INSTALLED BY HVAC CONTRACTOR.
2. EQUIPMENT PROVIDED AND INSTALLED BY ELECTRICAL CONTRACTOR.
3. WIRING PROVIDED AND INSTALLED BY HVAC CONTRACTOR.
4. WIRING PROVIDED AND INSTALLED BY ELECTRICAL CONTRACTOR.
5. SEE FLOOR PLANS FOR QUANTITY.
6. EC TO PROVIDE RIB RELAY. VERIFY COIL VOLTAGE 24 VOLT OR 120 VAC WITH HVAC CONTRACTOR.
7. CONNECTION IN HVAC CONTROL PANEL BY HVAC CONTRACTOR. EC TO COORDINATE WITH HVAC CONTRACTOR.
8. MOUNT RIB RELAY IN A NEMA 1 ENCLOSURE, LABEL WHICH DAMPER(S) THIS RELAY CONTROLS. LOCATE ENCLOSURE IN ACCESSIBLE LOCATION SUCH AS ADJACENT FROM HVAC CONTROL PANEL, DUCT DETECTOR REMOTE TEST, OR 120V PANEL FEEDING SMOKE DAMPERS.

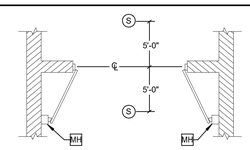



NOTES:

1. PROVIDE ALL FLUSH MOUNTED OUTLET BOXES WHERE WALL IS NEW OF WALL IS HOLLOW AND CAN BE FISHED.
2. IF EXISTING WALL CAN NOT BE FISHED, PROVIDE SURFACE METAL RACEWAY FOR CABLING. PROVIDE FIRE ALARM INTERFACE IN DOOR CONTROLLER ABOVE.
3. PROGRAM FIRE ALARM SYSTEM SUCH THAT DOOR CLOSES ONLY WHEN LOCAL SMOKE DETECTORS ARE ACTIVATED.

4 E3.1 FIRE SHUTTER INSTALLATION DETAIL

SCALE: NTS

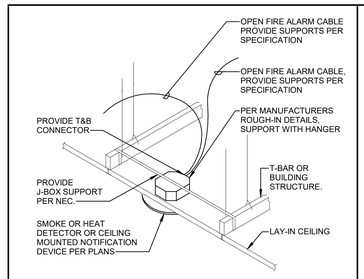



NOTES:

1. DOUBLE DOOR SHOWN, SINGLE DOORS SIMILAR, EXCEPT ONLY ONE MAGNETIC DOOR HOLDER.
2. PROVIDE ALL FLUSH MOUNTED OUTLET BOXES WHERE WALL IS NEW OR WALL IS HOLLOW AND CAN BE FISHED.
3. CONNECT TO FIRE ALARM SYSTEM.
4. IF EXISTING WALL CAN NOT BE FISHED, PROVIDE SURFACE METAL RACEWAY FOR CABLING.

3 E3.1 MAGNETIC DOOR HOLD DETAIL - ADDRESSABLE FACP

SCALE: NTS

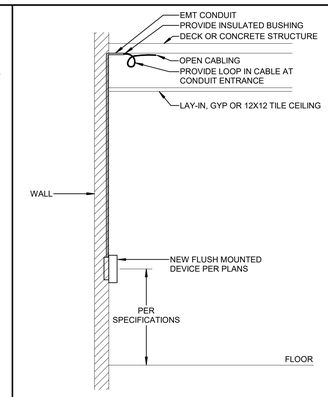



GENERAL NOTE:

1. BOX TO BE ACCESSIBLE VIA CEILING TILE, AND HAVE FRONT COVER WITH PAINTED RED "FA".

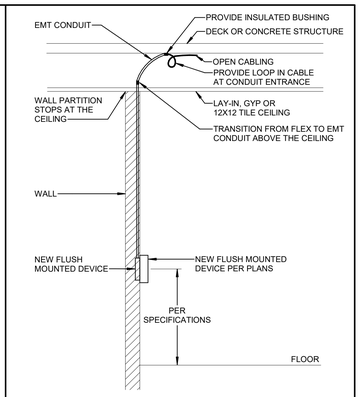
DETAIL ILLUSTRATING NEW FLUSH MOUNTED CEILING DEVICE, IN LAY-IN CEILING

NTS



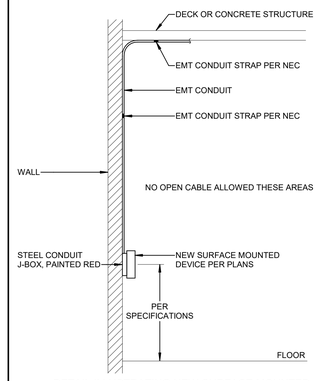

DETAIL ILLUSTRATING NEW FLUSH MOUNTED WALL DEVICE

NTS



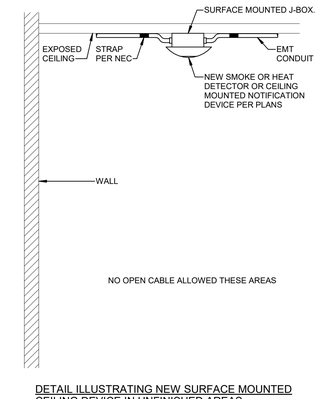

DETAIL ILLUSTRATING NEW WALL MOUNTED DEVICE IN EXISTING PARTITION WALLS

NTS




DETAIL ILLUSTRATING NEW SURFACE MOUNTED WALL DEVICE IN MECHANICAL SPACES

NTS

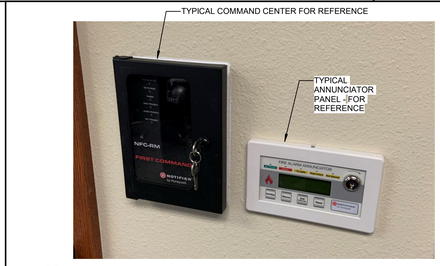



DETAIL ILLUSTRATING NEW SURFACE MOUNTED CEILING DEVICE IN UNFINISHED AREAS

NTS

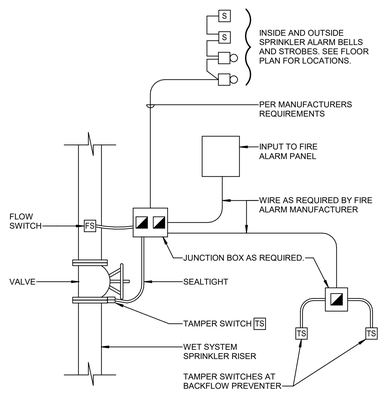
1 E3.1 FIRE ALARM DEVICE INSTALLATION DETAILS (NEW FACILITIES)

SCALE: NTS




7 E3.1 TYPICAL COMMAND CENTER AND ANNUNCIATOR PANEL

SCALE: NTS

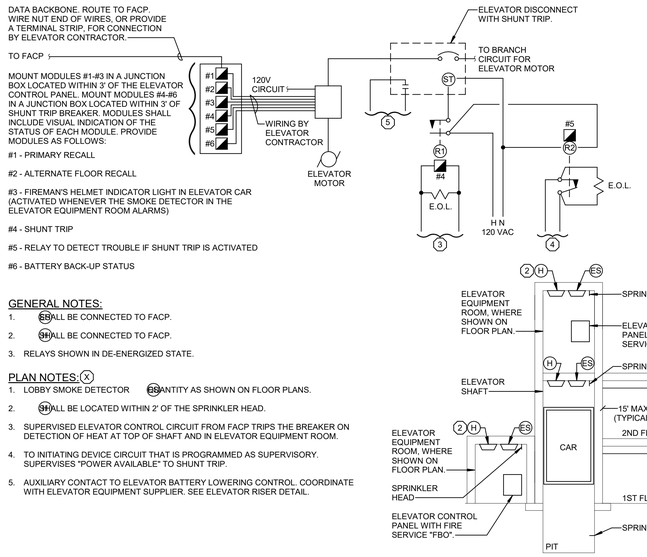



NOTES:

1. SEE FLOOR PLANS FOR ACTUAL NUMBER OF FLOW SWITCHES AND TAMPER SWITCHES. PROVIDE QUANTITY OF MODULES AS REQUIRED.
2. MOUNT MODULES WITH-IN 3'-0".

6 E3.1 SPRINKLER VALVE DIAGRAM

SCALE: NTS




GENERAL NOTES:

1. (SD) SHALL BE CONNECTED TO FACP.
2. (SH) SHALL BE CONNECTED TO FACP.
3. RELAYS SHOWN IN DE-ENERGIZED STATE.

PLAN NOTES: (X)

1. LOBBY SMOKE DETECTOR (SD) QUANTITY AS SHOWN ON FLOOR PLANS.
2. (SH) SHALL BE LOCATED WITHIN 2' OF THE SPRINKLER HEAD.
3. SUPERVISED ELEVATOR CONTROL CIRCUIT FROM FACP TRIPS THE BREAKER ON DETECTION OF HEAT AT TOP OF SHAFT AND IN ELEVATOR EQUIPMENT ROOM.
4. TO INITIATING DEVICE CIRCUIT THAT IS PROGRAMMED AS SUPERVISORY. SUPERVISES "POWER AVAILABLE" TO SHUNT TRIP.
5. AUXILIARY CONTACT TO ELEVATOR BATTERY LOWERING CONTROL. COORDINATE WITH ELEVATOR EQUIPMENT SUPPLIER. SEE ELEVATOR RISER DETAIL.

5 E3.1 ELEVATOR SMOKE/HEAT DETECTOR DETAIL

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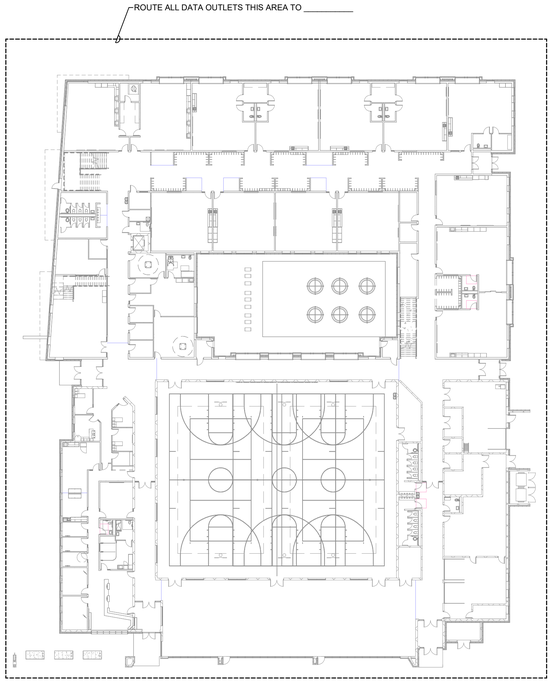
Sheet Title: **FIRE ALARM DETAILS**

Sheet Number: **E3.1**

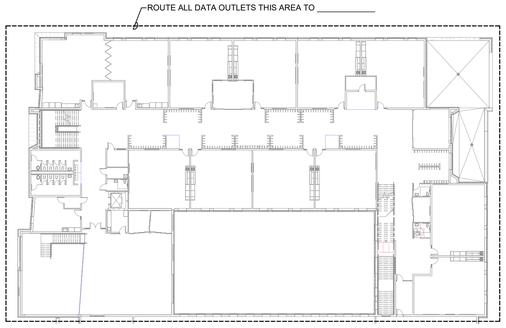
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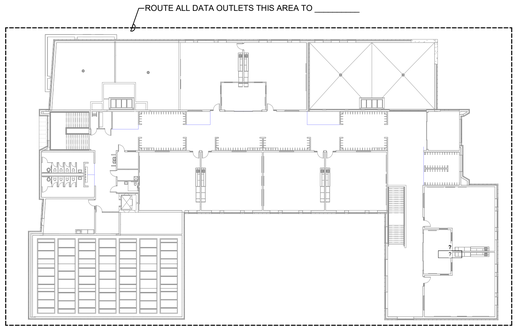
Autodesk Docs://3732 - LaCrosse School 2024 Referendum/0037724_LCSD_New Elementary School_ELECT_R24.rvt
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FIRST FLOOR PLAN




SECOND FLOOR PLAN

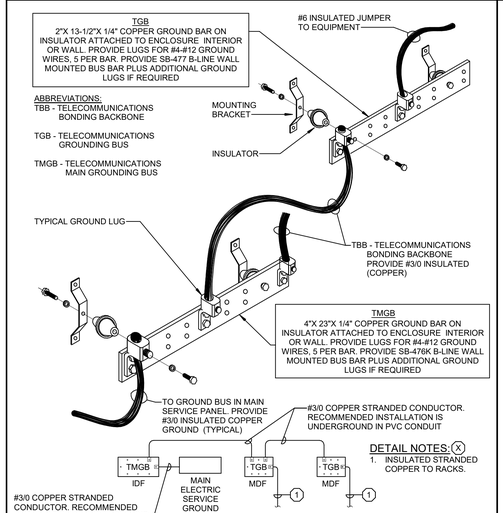



THIRD FLOOR PLAN

GENERAL NOTES

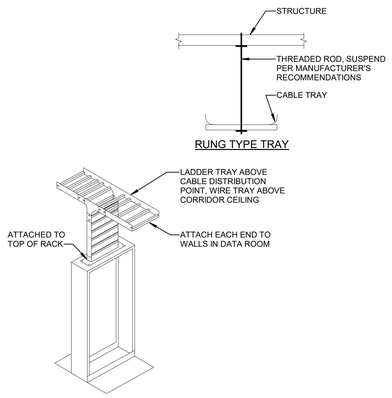
1. FOR DATA ROUTING SEE LOCAL AREA NETWORK RISER DETAIL 1/E4.1.

1 E4.0 TELECOMMUNICATIONS ROUTING DETAIL
SCALE: NTS




GROUNDING RISER
NTS

6 E4.0 TELECOMMUNICATIONS SYSTEM GROUNDING DETAIL WITH DATA GROUNDING RISER
SCALE: NTS



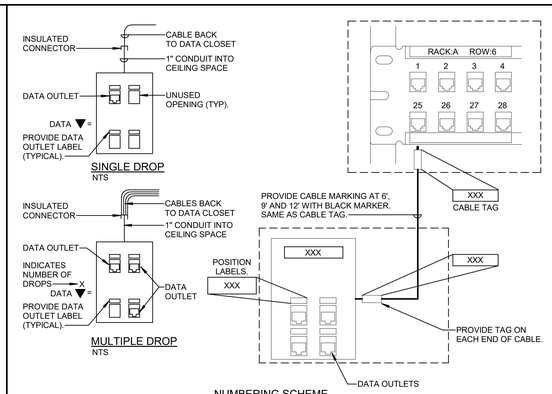

RUNG TYPE TRAY

TYPICAL OPEN RACK DETAIL

NOTES:

1. COORDINATE WITH DUCT WORK AND OTHER OBSTRUCTIONS ABOVE CEILING.
2. INSTALL SO TRAY CAN BE ACCESSED FROM CEILING BELOW.
3. PROVIDE MULTIPLE RUNS WHERE REQUIRED TO ACHIEVE 50% SPARE CAPACITY AND WHERE INDICATED ON DRAWING.

5 E4.0 CABLE TRAY DETAIL
SCALE: NTS




SINGLE DROP
NTS

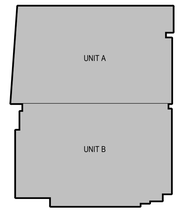
MULTIPLE DROP
NTS

NUMBERING SCHEME
NTS

NOTES:

1. ALL BOXES TO BE 4" SQUARE DEEP WITH 1 OR 2-GANG RING.
2. PROVIDE DATA OUTLETS IN QUANTITY SHOWN ON DRAWINGS.
3. PROVIDE MINIMUM 4 UNIT PLATE. CLOSE ALL UNUSED OPENINGS.
4. PROVIDE OUTLET LABELING AS DIRECTED BY OWNER.
5. IF SHOWN NEXT TO ABOVE COUNTER RECEPTACLE, ASSUME DEVICE MOUNTED ABOVE COUNTER.
6. SEE FLOOR PLANS FOR OTHER DEVICES THAT MAY BE MOUNTED IN CONJUNCTION WITH DATA OUTLETS.

4 E4.0 TYPICAL TELECOMMUNICATIONS OUTLET CONFIGURATIONS
SCALE: NTS




KEY PLAN

BRAY ARCHITECTS
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Milwaukee, Wisconsin 53204
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MSA
(800) 362-4505 • www.msa-ps.com

Project Title:
New Building for
New Elementary School
La Crosse School District
807 East Ave S
La Crosse, WI 54601

REVISIONS:

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|---|---|---|

NOT FOR CONSTRUCTION

Project Number:
3752

Issued For:
SD Set
03/26/25

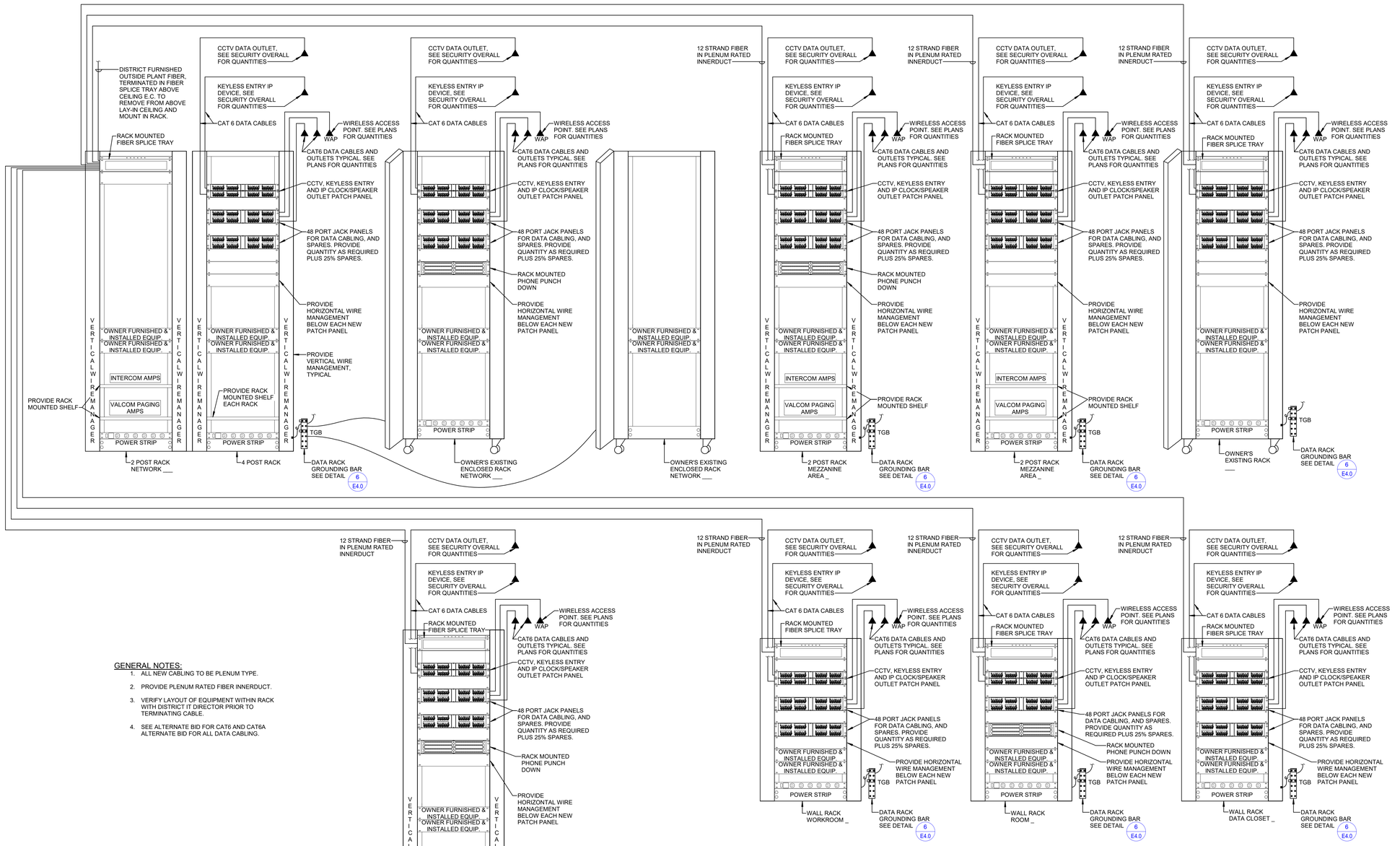
Sheet Title:
DATA ROUTING PLANS AND DETAILS

Sheet Number:
E4.0

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GENERAL NOTES:

1. ALL NEW CABLING TO BE PLENUM TYPE.
2. PROVIDE PLENUM RATED FIBER INNERDUCT.
3. VERIFY LAYOUT OF EQUIPMENT WITHIN RACK WITH DISTRICT IT DIRECTOR PRIOR TO TERMINATING CABLE.
4. SEE ALTERNATE BID FOR CAT6 AND CAT6A ALTERNATE BID FOR ALL DATA CABLING.

1 / E4.1 LOCAL AREA NETWORK RISER
SCALE: NTS

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La Crosse, WI 54601

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NOT FOR CONSTRUCTION

Project Number: 3752

Issued For: SD Set 03/26/25

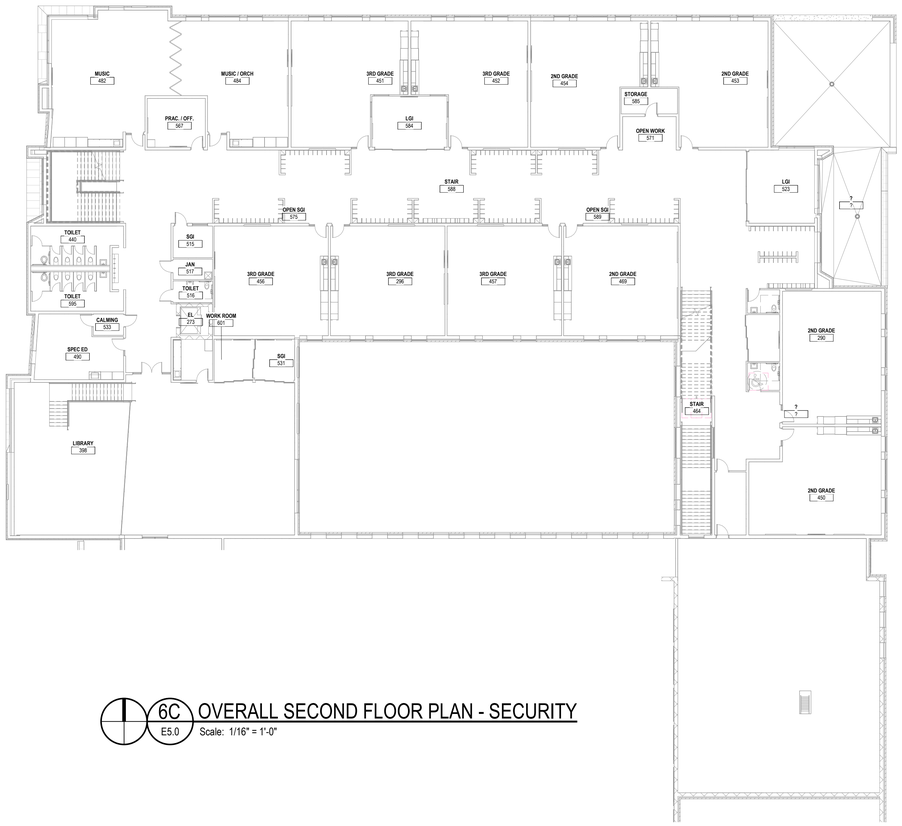
Sheet Title: LOCAL AREA NETWORK RISER

Sheet Number: E4.1

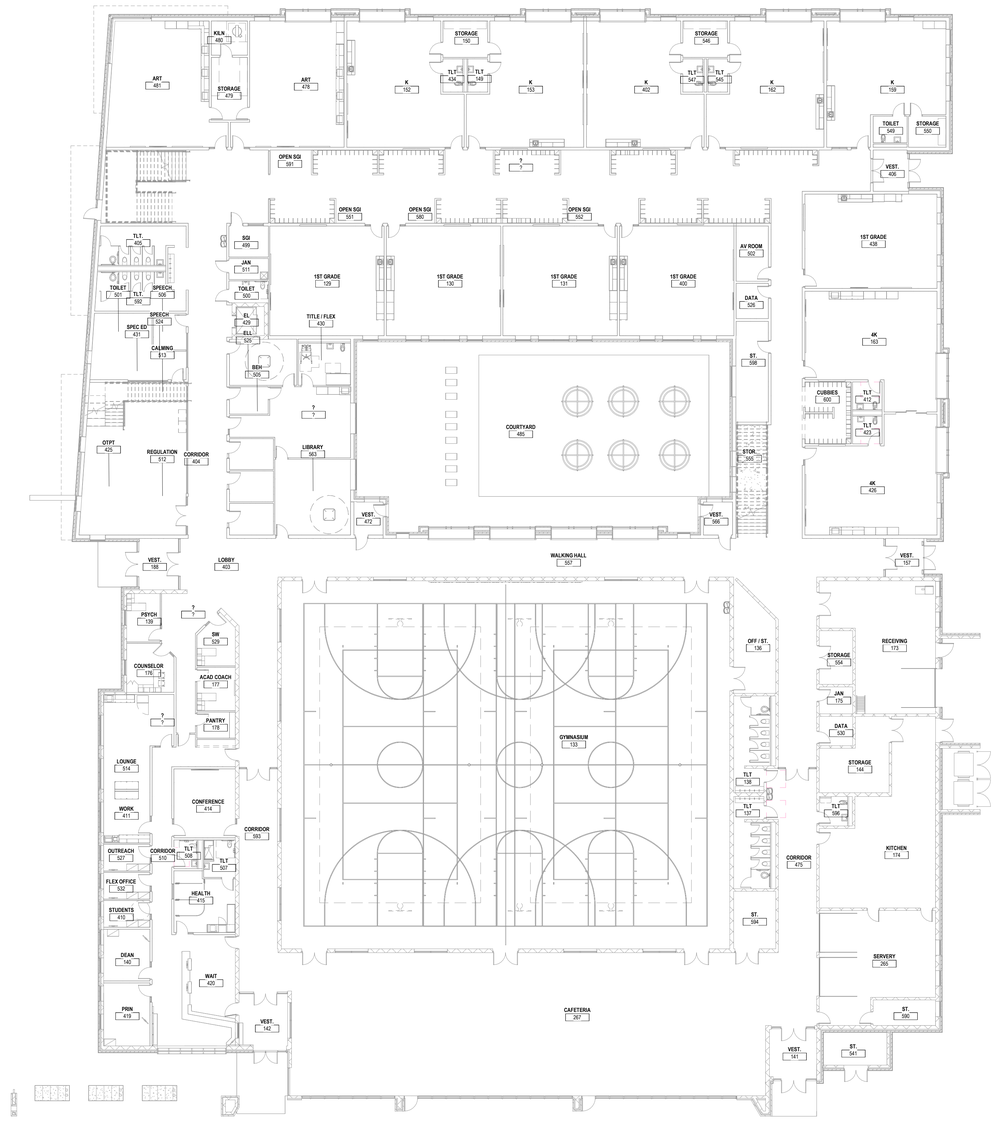
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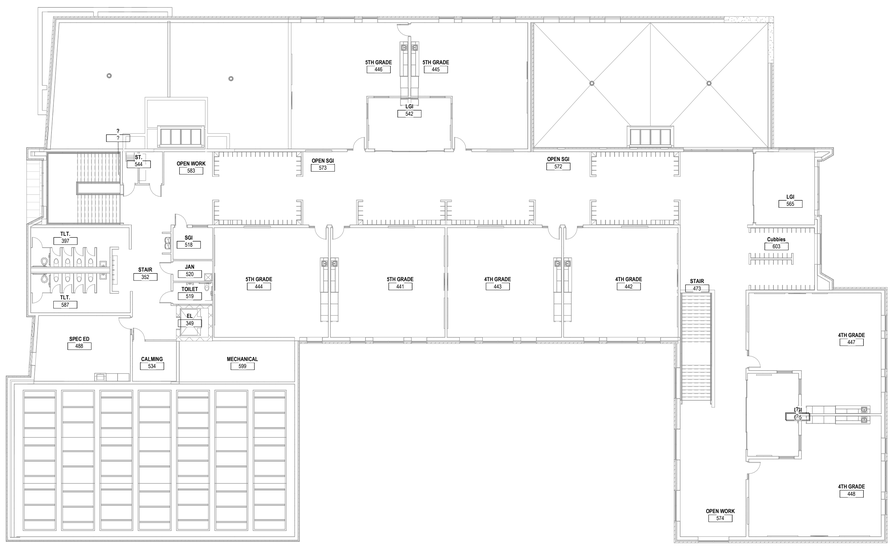
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6C E5.0 OVERALL SECOND FLOOR PLAN - SECURITY
Scale: 1/16" = 1'-0"




2B E5.0 OVERALL FIRST FLOOR PLAN - SECURITY
Scale: 1/16" = 1'-0"

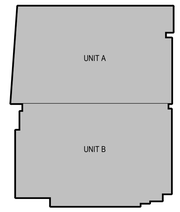



6A E5.0 OVERALL THIRD FLOOR PLAN - SECURITY
Scale: 1/16" = 1'-0"

## GENERAL NOTES

1. ALL CONDUIT AND CABLE FOR EXTERIOR CAMERAS SHALL BE INSTALL PER DETAIL 1 / E5.1 .
2. SEE POWER PLANS FOR DOOR CONTROLLER AND POWER SUPPLY CIRCUITS. ANY ADDITIONAL 120V POWER REQUIRED FOR KEYLESS ENTRY SYSTEM OR VIDEO SURVEILLANCE SYSTEM SHALL BE PROVIDED FROM PANEL "___", "___", "___", OR "___". PROVIDE ADDITIONAL 20A/1P BREAKERS.
3. SEE OVERALL DATA ROUTING PLAN FOR ROUTING OF CABLING.
4. ALL LOW VOLTAGE CABLING IN AREAS WITH EXPOSED STRUCTURE SHALL BE IN CONDUIT.
5. VERIFY ALL LOCATIONS AND MOUNTING HEIGHTS WITH OWNER'S SECURITY CAMERA VENDOR PRIOR TO ROUGH-IN AND INSTALLATION OF CABLING.

PLAN NOTES (X)




PLAN NORTH
KEY PLAN

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Project Number: 3752

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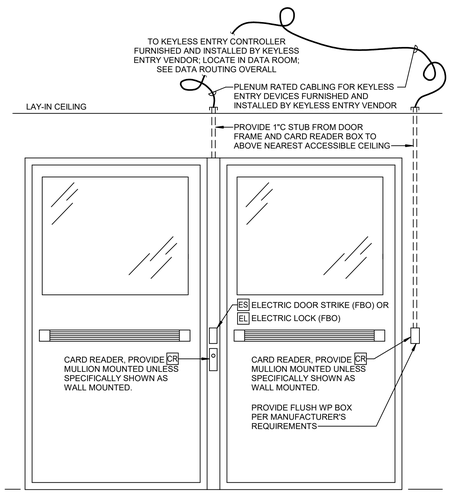
Sheet Title: FLOOR PLANS - SECURITY

Sheet Number: E5.0

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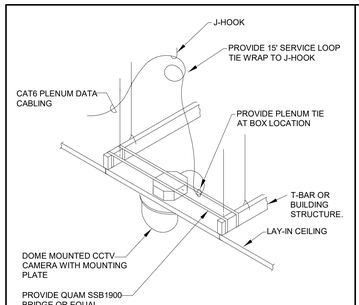
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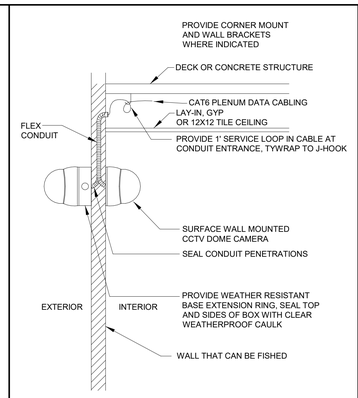

GENERAL NOTES:

1. ALL WIRING AND CONDUIT MUST BE CONCEALED TO DOOR FRAME.
2. COORDINATE SPECIFIC ROUGH-IN REQUIREMENTS WITH KEYLESS ENTRY VENDOR.

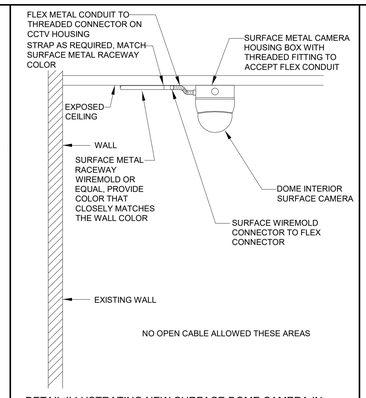
2 / E5.1 DOOR STRIKE, CARD READER, & DOOR CONTACT INSTALLATION DETAIL
SCALE: NTS



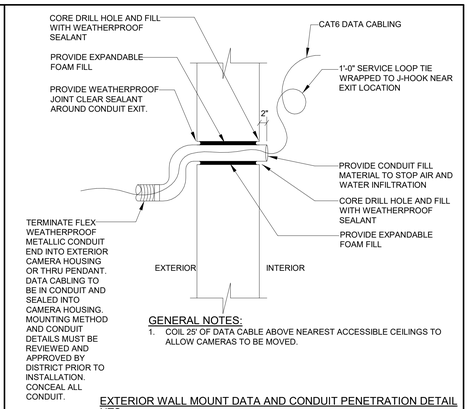

DETAIL ILLUSTRATING NEW DOME LAY-IN CEILING CAMERA
NTS




DETAIL ILLUSTRATING NEW FLUSH MOUNTED WALL CAMERA WITHOUT ARM IN NEW FACILITIES
NTS




DETAIL ILLUSTRATING NEW SURFACE DOME CAMERA IN FINISHED AREAS WITHOUT ACCESSIBLE CEILINGS
NTS




GENERAL NOTES:

1. COIL 25' OF DATA CABLE ABOVE NEAREST ACCESSIBLE CEILINGS TO ALLOW CAMERAS TO BE MOVED.

EXTERIOR WALL MOUNT DATA AND CONDUIT PENETRATION DETAIL
NTS




DETAIL ILLUSTRATING NEW SURFACE MOUNTED DOME CAMERA IN UNFINISHED AREAS
NTS




DETAIL ILLUSTRATING NEW SURFACE MOUNTED DOME CAMERA IN UNFINISHED AREAS
NTS




TYPICAL CABLE SUPPORT DETAIL
NTS

GENERAL NOTES:

1. J HOOK SHALL BE SIZED FOR CABLE SIZE AND LOAD.
2. SPACING BETWEEN J HOOKS 4 FEET ON CENTER.
3. INSTALL ADDITIONAL J HOOKS AS REQUIRED TO ACCOMMODATE NEW CABLING.

DETAIL NOTES: (X)

1. THIS HOOK IS DEDICATED FOR DATA CABLING.
2. FLEX CONDUIT IN WALL.
3. WALL SLEEVE.
4. FLOOR OR ROOF STRUCTURE ABOVE.
5. HANGER SUPPORTED FROM STRUCTURE OF FLOOR ABOVE OR WALL.
6. LAY-IN CEILING.
7. J HOOK, TYPICAL.

1 / E5.1 SECURITY ROUGH-IN DETAILS
SCALE: NTS

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**807 East Ave S**
**La Crosse, WI 54601**

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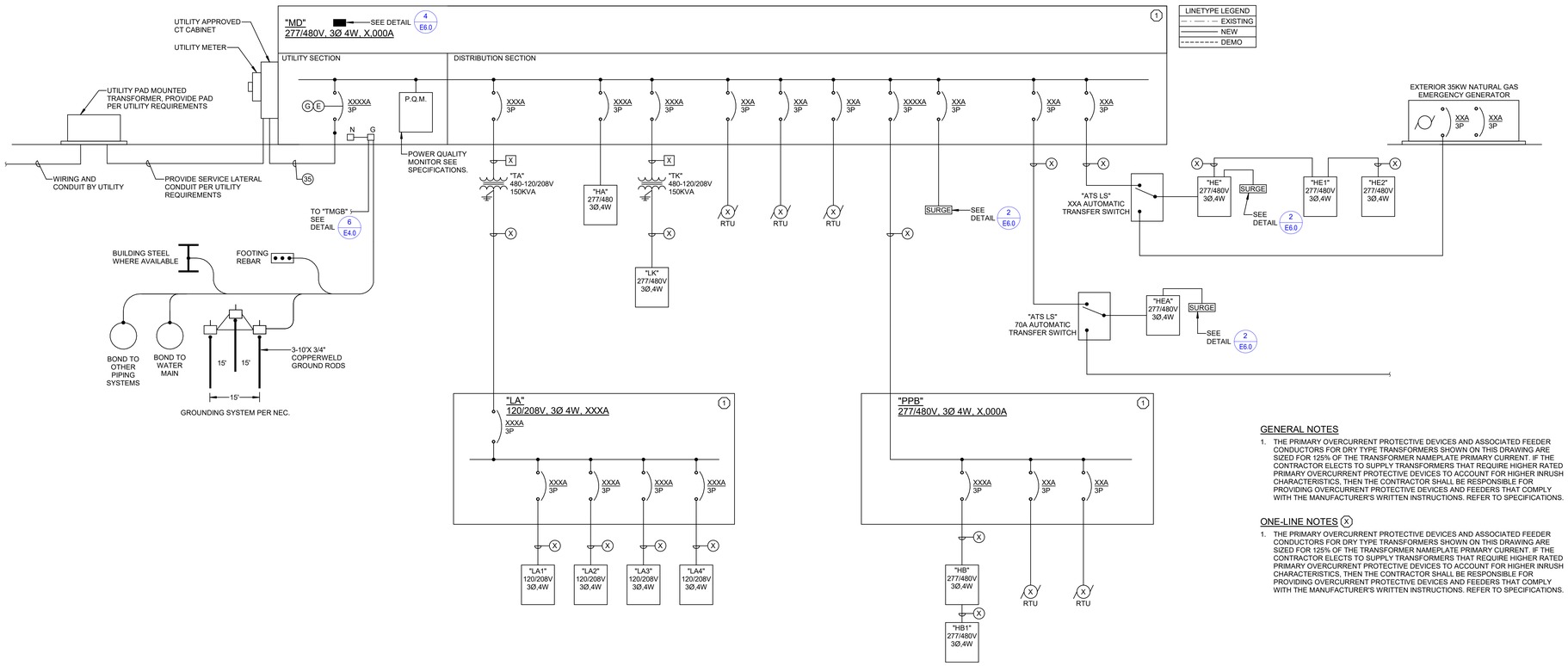
Sheet Title: **SECURITY DETAILS**

Sheet Number: **E5.1**

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EDIT PER PROJECT REQUIREMENTS

## ONE-LINE DIAGRAM

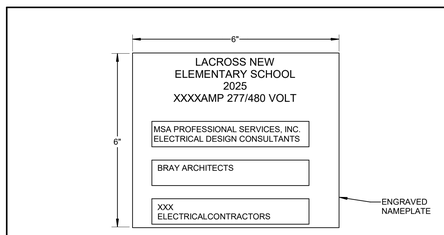
NTS

### GENERAL NOTES

1. THE PRIMARY OVERCURRENT PROTECTIVE DEVICES AND ASSOCIATED FEEDER CONDUCTORS FOR DRY TYPE TRANSFORMERS SHOWN ON THIS DRAWING ARE SIZED FOR 125% OF THE TRANSFORMER NAMEPLATE PRIMARY CURRENT. IF THE CONTRACTOR ELECTS TO SUPPLY TRANSFORMERS THAT REQUIRE HIGHER RATED PRIMARY OVERCURRENT PROTECTIVE DEVICES TO ACCOUNT FOR HIGHER INRUSH CHARACTERISTICS, THEN THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING OVERCURRENT PROTECTIVE DEVICES AND FEEDERS THAT COMPLY WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS. REFER TO SPECIFICATIONS.

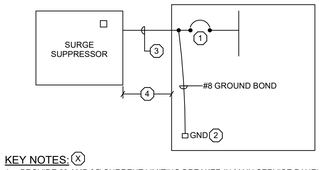
### ONE-LINE NOTES (X)

1. THE PRIMARY OVERCURRENT PROTECTIVE DEVICES AND ASSOCIATED FEEDER CONDUCTORS FOR DRY TYPE TRANSFORMERS SHOWN ON THIS DRAWING ARE SIZED FOR 125% OF THE TRANSFORMER NAMEPLATE PRIMARY CURRENT. IF THE CONTRACTOR ELECTS TO SUPPLY TRANSFORMERS THAT REQUIRE HIGHER RATED PRIMARY OVERCURRENT PROTECTIVE DEVICES TO ACCOUNT FOR HIGHER INRUSH CHARACTERISTICS, THEN THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING OVERCURRENT PROTECTIVE DEVICES AND FEEDERS THAT COMPLY WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS. REFER TO SPECIFICATIONS.




NOTES:
1. INSTALL ON MAIN SERVICE PANEL.

4 / E6.0 NAMEPLATE DETAIL
SCALE: NTS




KEY NOTES: (X)
1. PROVIDE 60 AMP 3Ø CURRENT LIMITING BREAKER IN MAIN SERVICE PANEL. PROVIDE 30 AMP 3Ø CURRENT LIMITING BREAKER IN MAIN SERVICE PANEL.
2. MAIN SERVICE PANEL OR DISTRIBUTION PANEL, SEE DRAWINGS.
3. 4#6 & #6 GND. IN 1 1/4" CONDUIT FOR 60 AMP MAIN SERVICE SUPPRESSION 4#10 & #8 GND. IN 1" CONDUIT FOR 30 AMP BUILDING INTERIOR PANEL SURGE SUPPRESSION.
4. CONDUIT AND FEEDER DISTANCE NOT TO EXCEED 5 FEET.

2 / E6.0 SURGE SUPPRESSION CONNECTION
SCALE: NTS

### FEEDER SCHEDULE

| IDENTIFIER | AMPACITY | 1Ø TWO-WIRE CIRCUIT | | 1Ø OR 3Ø THREE-WIRE CIRCUIT | | 3Ø FOUR-WIRE CIRCUIT | | EQUIPMENT GROUNDING CONDUCTOR |
|---|---|---|---|---|---|---|---|---|
| | | CONDUIT | CIRCUIT CONDUCTORS | CONDUIT | CIRCUIT CONDUCTORS | CONDUIT | CIRCUIT CONDUCTORS | |
| COPPER CONDUCTORS | | | | | | | | |
| 1 | 10 | 3/4" | (2) #12 | 3/4" | (3) #12 | 3/4" | (4) #12 | #12 |
| 2 | 15 | 3/4" | (2) #12 | 3/4" | (3) #12 | 3/4" | (4) #12 | #12 |
| 3 | 20 | 3/4" | (2) #12 | 3/4" | (3) #12 | 3/4" | (4) #12 | #12 |
| 4 | 25 | 3/4" | (2) #10 | 3/4" | (3) #10 | 3/4" | (4) #10 | #10 |
| 5 | 30 | 3/4" | (2) #10 | 3/4" | (3) #10 | 3/4" | (4) #10 | #10 |
| 6 | 35 | 3/4" | (2) #8 | 3/4" | (3) #8 | 3/4" | (4) #8 | #10 |
| 7 | 40 | 3/4" | (2) #8 | 3/4" | (3) #8 | 3/4" | (4) #8 | #10 |
| 8 | 45 | 3/4" | (2) #8 | 3/4" | (3) #8 | 3/4" | (4) #8 | #10 |
| 9 | 50 | 3/4" | (2) #8 | 3/4" | (3) #8 | 3/4" | (4) #8 | #10 |
| 10 | 60 | 3/4" | (2) #6 | 3/4" | (3) #6 | 1" | (4) #6 | #10 |
| 11 | 70 | 1" | (2) #4 | 1" | (3) #4 | 1-1/4" | (4) #4 | #8 |
| 12 | 80 | 1" | (2) #4 | 1" | (3) #4 | 1-1/4" | (4) #4 | #8 |
| 13 | 90 | 1" | (2) #3 | 1" | (3) #3 | 1-1/4" | (4) #3 | #8 |
| 14 | 100 | 1-1/4" | (2) #3 | 1-1/4" | (3) #3 | 1-1/4" | (4) #3 | #8 |
| ALUMINUM CONDUCTORS | | | | | | | | |
| 15 | 110 | 1-1/4" | (2) #1/0 | 1" | (3) #1/0 | 2" | (4) #1/0 | #4 |
| 16 | 125 | 1-1/2" | (2) #2/0 | 2" | (3) #2/0 | 2" | (4) #2/0 | #4 |
| 17 | 150 | 1-1/2" | (2) #3/0 | 2" | (3) #3/0 | 2" | (4) #3/0 | #4 |
| 18 | 175 | N/A | N/A | 2" | (3) #4/0 | 2-1/2" | (4) #4/0 | #4 |
| 19 | 200 | N/A | N/A | 2-1/2" | (3) 250KCMIL | 2-1/2" | (4) 250KCMIL | #4 |
| 20 | 225 | N/A | N/A | 2-1/2" | (3) 300KCMIL | 2-1/2" | (4) 300KCMIL | #2 |
| 21 | 250 | N/A | N/A | 2-1/2" | (3) 350KCMIL | 3" | (4) 350KCMIL | #2 |
| 22 | 300 | N/A | N/A | 3" | (3) 500 KCMIL | 3" | (4) 500KCMIL | #2 |
| 23 | 350 | N/A | N/A | (2) 2" | 2 SETS OF (3) #4/0 | (2) 2-1/2" | 2 SETS OF (4) #4/0 | #1 |
| 24 | 400 | N/A | N/A | (2) 2-1/2" | 2 SETS OF (3) 250KCMIL | (2) 2-1/2" | 2 SETS OF (4) 250KCMIL | #1 |
| 25 | 450 | N/A | N/A | (2) 2-1/2" | 2 SETS OF (3) 300KCMIL | (2) 2-1/2" | 2 SETS OF (4) 300KCMIL | #1/0 |
| 26 | 500 | N/A | N/A | (2) 2-1/2" | 2 SETS OF (3) 350KCMIL | (2) 3" | 2 SETS OF (4) 350KCMIL | #1/0 |
| 27 | 600 | N/A | N/A | (2) 3" | 2 SETS OF (3) 500KCMIL | (2) 3 1/2" | 2 SETS OF (4) 500KCMIL | #2/0 |
| 28 | 700 | N/A | N/A | (3) 2-1/2" | 3 SETS OF (3) 350KCMIL | (3) 3" | 3 SETS OF (4) 350KCMIL | #3/0 |
| 29 | 800 | N/A | N/A | (4) 2-1/2" | 4 SETS OF (3) 250KCMIL | (4) 2-1/2" | 4 SETS OF (4) 250KCMIL | #3/0 |
| 30 | 900 | N/A | N/A | (4) 2-1/2" | 4 SETS OF (3) 300KCMIL | (4) 2-1/2" | 4 SETS OF (4) 300KCMIL | #4/0 |
| 31 | 1000 | N/A | N/A | (4) 2-1/2" | 4 SETS OF (3) 350KCMIL | (4) 3" | 4 SETS OF (4) 350KCMIL | #4/0 |
| 32 | 1200 | N/A | N/A | (4) 3" | 4 SETS OF (3) 500KCMIL | (4) 3-1/2" | 4 SETS OF (4) 500KCMIL | 250KCMIL |
| 33 | 1600 | N/A | N/A | (6) 2-1/2" | 6 SETS OF (3) 400KCMIL | (6) 3" | 6 SETS OF (4) 400KCMIL | 350KCMIL |
| 34 | 1800 | N/A | N/A | (6) 3" | 6 SETS OF (3) 500KCMIL | (6) 3-1/2" | 6 SETS OF (4) 500KCMIL | 400KCMIL |
| 35 | 2000 | N/A | N/A | (6) 3" | 6 SETS OF (3) 600KCMIL | (6) 3-1/2" | 6 SETS OF (4) 600KCMIL | 400KCMIL |
| 36 | 2500 | N/A | N/A | (8) 3" | 8 SETS OF (3) 600KCMIL | (8) 3-1/2" | 8 SETS OF (4) 600KCMIL | 600KCMIL |
| 37 | 3000 | N/A | N/A | (10) 3-1/2" | 10 SETS OF (3) 500KCMIL | (10) 3-1/2" | 10 SETS OF (4) 500KCMIL | 600KCMIL |
| 38 | 4000 | N/A | N/A | (12) 3-1/2" | 12 SETS OF (3) 600KCMIL | (12) 4" | 12 SETS OF (4) 600KCMIL | 800KCMIL |

PLAN NOTATION:
- ◇ SINGLE-PHASE, TWO-WIRE FEEDER, NUMBER IS THE FEEDER IDENTIFIER
- □ SINGLE OR THREE-PHASE, THREE-WIRE FEEDER, NUMBER IS THE FEEDER IDENTIFIER
- ○ THREE-PHASE, FOUR-WIRE FEEDER, NUMBER IS THE FEEDER IDENTIFIER


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Project Number: 3752

Issued For: SD Set 03/26/25

Sheet Title: ONE LINE DIAGRAM

Sheet Number: E6.0

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Sheet Title:
**PANEL SCHEDULES**

Sheet Number:
E6.1

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## ELECTRICAL MOTOR STARTER SCHEDULE (Option 1)

| LOAD NAME | FLA | BREAKER | VOLTAGE | POLES | PANEL | WIRE (CU) | NOTES |
|---|---|---|---|---|---|---|---|

GENERAL NOTES
1. SEE HVAC PLANS FOR STARTER REQUIREMENTS.
2. SEE HVAC PLANS FOR LOCATIONS OF ALL HVAC EQUIPMENT.
3. PROVIDE ALL DISCONNECTS REQUIRED BY CODE.
4. ALL CONDUCTORS SHALL BE COPPER.

REMARKS NOTES
1. HVAC CONTRACTOR TO SUPPLY VFD FOR THIS UNIT. E.C. TO INSTALL.
2. PROVIDE WP/GFI DUPLEX RECEPTACLE ON ROOF WITHIN 20' OF EQUIPMENT PER NEC. CONNECT TO NORMAL POWER CIRCUIT FEEDING NEAREST RECEPTACLES.
3. MULTIPLE HVAC MOTORS ON THIS CIRCUIT.
4. INTERCONNECT RAC AND OU OF SAME NUMBER AS REQUIRED BY MANUFACTURER.

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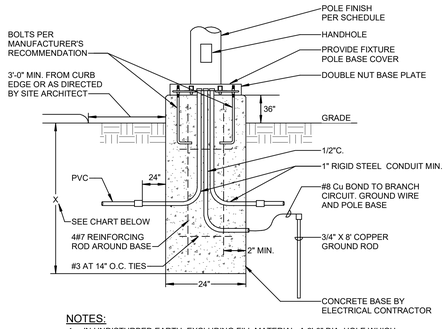
Sheet Title:
**FIXTURE & ELECTRICAL MOTOR STARTER SCHEDULE**

Sheet Number:
E7.0

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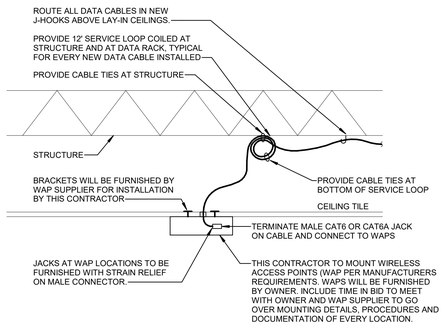



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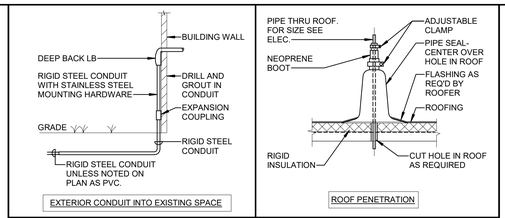
1. IN UNDISTURBED EARTH, EXCLUDING FILL MATERIAL, A 2'-0" DIA. HOLE WHICH SHALL BE USED AS THE FORM FOR THE CONCRETE BASE SHALL BE DRILLED.
2. IN EXCAVATED AREAS OR IN EXISTING SOIL CONTAINING FILL OF OBJECTIONABLE MATERIAL, BACKFILL AROUND CONCRETE BASE WITH COMPACTED GRANULAR BACKFILL A MIN. OF 2'-0" IN ALL DIRECTIONS.

| POLE HEIGHT IN FEET | BASE DEPTH IN INCHES BELOW GRADE |
|---|---|
| 10'-0" | X = 60" (INCHES) |
| 15'-0" | X = 60" (INCHES) |
| 20'-0" | X = 60" (INCHES) |
| 25'-0" | X = 72" (INCHES) |
| 30'-0" | X = 72" (INCHES) |
| 35'-0" | X = 72" (INCHES) |
| 40'-0" | X = 96" (INCHES) |
| 45'-0" | X = 96" (INCHES) |
| 50'-0" | X = 108" (INCHES) |

4 / E8.0 — 36" EXTERIOR LIGHTING CONCRETE BASE DETAIL
SCALE: NTS



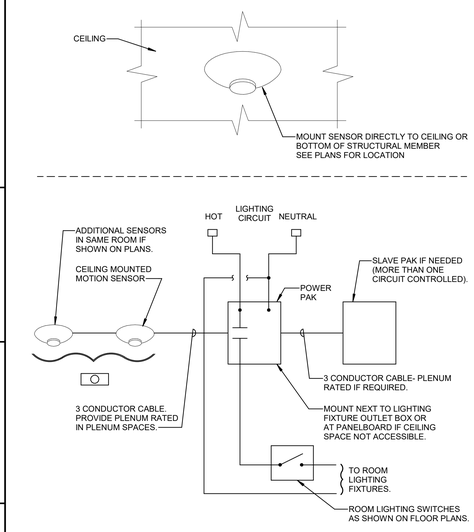

3 / E8.0 — WIRELESS ACCESS POINT MOUNTING DETAIL
SCALE: NTS



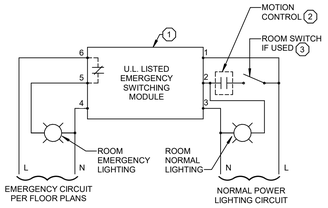

NOTES:

1. E.C. MAY USE PVC COATED RIGID STEEL CONDUIT IN LIEU OF RIGID CONDUIT AND PVC SLEEVE.
2. PVC SLEEVE TO BE AS SNUG AS POSSIBLE ON RIGID CONDUIT.

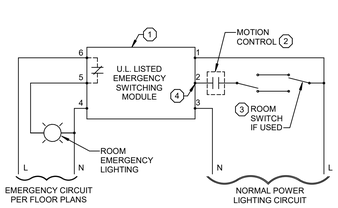
2 / E8.0 — CONDUIT DETAILS
SCALE: NTS




1 / E8.0 — MOTION SENSOR LIGHTING CONTROL
SCALE: NTS

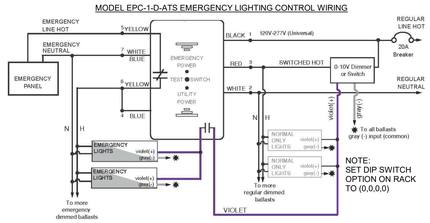



A — CONTROL OF EMERGENCY AND NORMAL LIGHTING WITH SAME SWITCH AND/OR MOTION DETECTOR




B — CONTROL OF EMERGENCY LIGHTING WITH SWITCH AND/OR MOTION DETECTOR OR EMERGENCY LIGHTING TO OPERATE ONLY UPON LOSS OF NORMAL POWER

MODEL EPC-1-D-ATS EMERGENCY LIGHTING CONTROL WIRING




NOTE:
SET DIP SWITCH OPTION ON RACK TO (0,0,0,0)

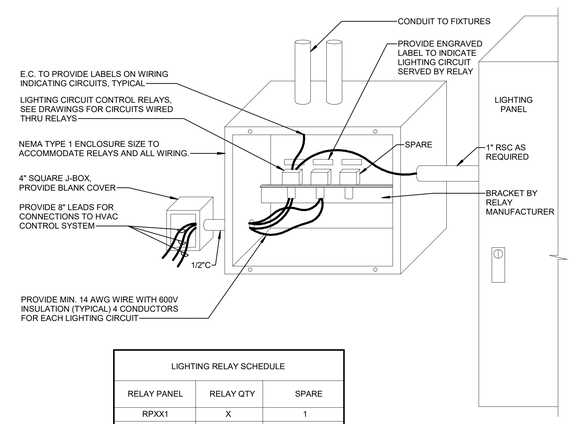
PLAN NOTES:(X)

1. MOUNT IN NEMA 1 VENTED ENCLOSURE, 200% LARGER THAN MODULE(S). MOUNT ADJACENT TO EMERGENCY PANEL. PROVIDE ENGRAVED LABEL ON ENCLOSURE INDICATING FUNCTION IE: "EMERGENCY BYPASS RELAY XXXXXXXX LIGHTING".
2. POWER SUPPLY FOR MOTION DETECTOR ON NORMAL CIRCUIT. DELETE WHEN NOT SHOWN ON DRAWING.
3. SINGLE POLE 3-WAY AND 4-WAY SWITCHES. IF SHOWN ON PLANS.
4. CAP NORMAL SWITCH SENSE LINE WHERE EMERGENCY LIGHTING TO OPERATE ONLY UPON LOSS OF NORMAL POWER.

***NOTE: Normal only lights are optional.***

C — CONTROL OF EMERGENCY LIGHTING WITH 0-10V CONTROLS

6 / E8.0 — EMERGENCY BYPASS DEVICE 0-10V
SCALE: NTS

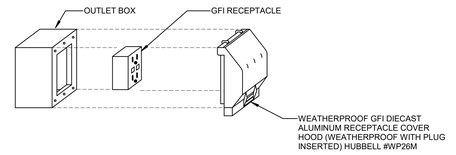



| LIGHTING RELAY SCHEDULE | | |
|---|---|---|
| RELAY PANEL | RELAY QTY | SPARE |
| RPXX1 | X | 1 |
| RPXX2 | X | 1 |
| | | |
| | | |

GENERAL NOTES:

1. ENCLOSURE TO BE NEMA 1 GENERAL PURPOSE.
2. PROVIDE WIRING FROM RELAY COILS AND CONTACTS TO J-BOX ON CONTROL PANEL FOR CONNECTION BY HVAC CONTROL CONTRACTOR. LIGHTING TO BE CONTROLLED THRU HVAC SYSTEM CONTROLS.

5 / E8.0 — RELAY LIGHTING CONTROL
SCALE: NTS




NOTE:
FOR ALL NEW CONSTRUCTION BOX IS TO BE FLUSH MOUNTED.

OUTLET BOX:
FLUSH: 3 1/2" DEEP 1-GANG MASONRY BOX SURFACE: F.S. BOX

7 / E8.0 — GFI/WP RECEPTACLE MOUNTING DETAIL
SCALE: NTS

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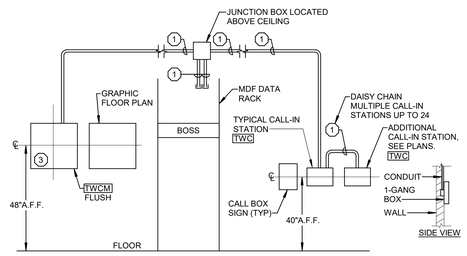
Sheet Title: **ELECTRICAL DETAILS**

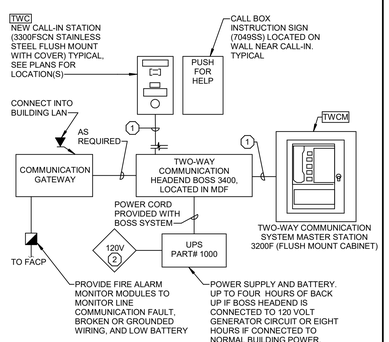
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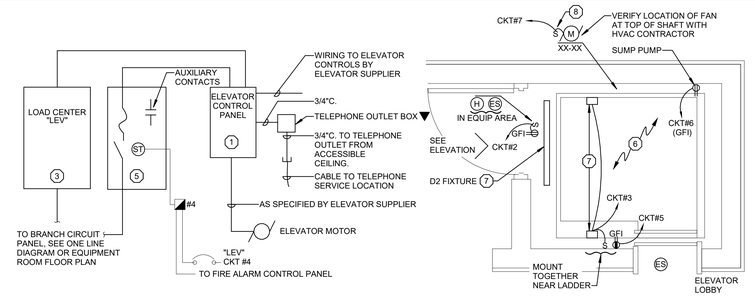


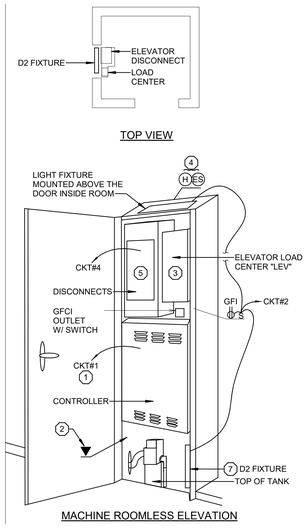

PLAN NOTES: (X) ROUGH-IN REQUIREMENTS

1. 18/4 CABLE IN 3/4" CONDUIT TO CONNECT MASTER PANEL AND STATIONS SUGGESTED.
2. POWER CONNECTION. PROVIDE 3/4" CONDUIT TO POWER SUPPLY.
3. MASTER PANEL OPENING PER SUPPLIERS RECOMMENDATIONS (BOX PROVIDED BY EQUIPMENT SUPPLIER).

2 / E8.1 TWO-WAY COMMUNICATION SYSTEM RATH MASTER
SCALE: NTS




TYPICAL ELEVATOR PIT
NTS




MACHINE ROOMLESS ELEVATION

GENERAL NOTE:

1. THESE PLANS ARE A TYPICAL LAYOUT FOR ELEVATOR EQUIPMENT ROOM AND ELEVATOR PIT AREA. COORDINATE THE EXACT LOCATION OF ALL DEVICES WITH THE ELEVATOR EQUIPMENT SUPPLIER.

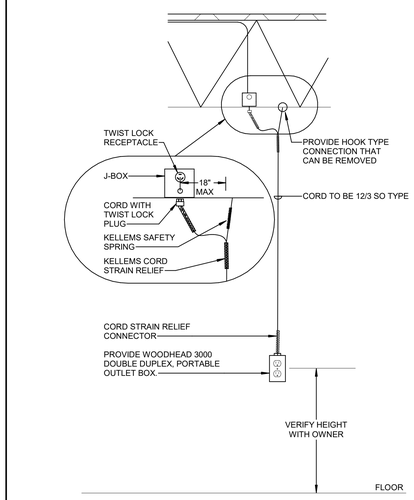
KEY NOTES: (X)

1. CONNECTION TO ELEVATOR CONTROL PANEL, FOR CAR LIGHTS AND RECEPTACLE.
2. WIRING FOR ELEVATOR TELEPHONE. MOUNT BELOW CONTROLLER, WIRE BACK TO MDF.
3. PROVIDE A 8 CIRCUIT, 100 AMP, SINGLE PHASE LOAD CENTER. ATTACH ENGRAVED LABEL INDICATING NAME OF PANEL. PROVIDE TYPED DIRECTORY INSIDE OF HINGED COVER, SEE DIRECTORY ON THIS DETAIL.
4. PROVIDE HEAT AND SMOKE DETECTORS.
5. DISCONNECT SHALL BE "BUSSMANN POWER MODULE SWITCH, MERSEN FUSIBLE SHUNT TRIP SWITCH OR EQUAL. PROVIDE AND INSTALL FUSES. UNIT SHALL HAVE SHUNT TRIP AND AUXILIARY CONTACT FOR BATTERY LOWERING DEVICE. VERIFY EXACT SIZE WITH ELEVATOR PROVIDER.
6. WHEN POSSIBLE MOUNT ALL ELECTRICAL DEVICES AT 48" IN ELEVATOR PIT. IF NOT POSSIBLE PROVIDE WIRING PER NEMA-4 WIRING STANDARDS.
7. PROVIDE TYPE D2 FIXTURE MOUNTED VERTICALLY OR HORIZONTALLY AT 18" TO CENTER ABOVE THE PIT FLOOR. COORDINATE WITH ELEVATOR SUPPLIER.
   METALUX CATALOG NUMBER 2VT2-LD5-5-DR100-W-UNV-L835-CD1-WL-U, OR
   LITHONIA CATALOG NUMBER FEM-L24-6000LM-LPPFL-WD-MVOLT-GZ10-35K-80CRI.
8. DISCONNECT SWITCH PROVIDED WITH UNIT, E.C. TO INSTALL.

LOAD CENTER "LEV":

CIRCUIT #1 - 15A/1P, GFI, WITH LOCK, ELEVATOR LIGHTS AND RECEPTACLE.
CIRCUIT #2 - 20A/1P, ELEVATOR EQUIPMENT AREA LIGHTS AND RECEPTACLE.
CIRCUIT #3 - 20A/1P, ELEVATOR PIT LIGHTS.
CIRCUIT #4 - 20A/1P, SHUNT TRIP.
CIRCUIT #5 - 20A/1P, ELEVATOR PIT REC.
CIRCUIT #6 - SUMP PUMP WITH LOCK.
CIRCUIT #7 - EXHAUST FAN
CIRCUIT #8 - SPARE.

1 / E8.1 TYPICAL ELEVATOR RISER - MACHINE ROOMLESS
SCALE: NTS




NOTE:
1. ALL STRAIN RELIEF CONNECTORS SIZED TO MATCH CORD DIAMETER.

3 / E8.1 OPEN CEILING CORD DROP OUTLET DETAIL
SCALE: NTS

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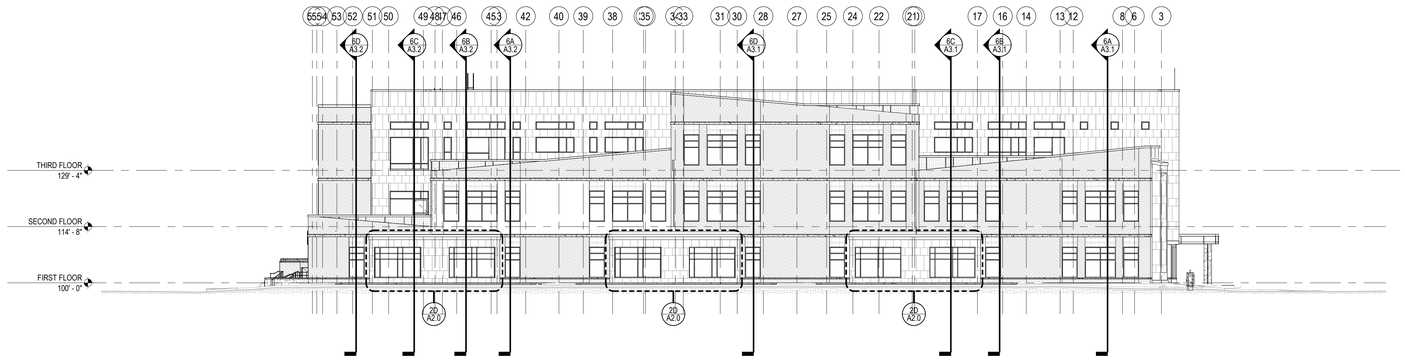
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Sheet Number: E8.1

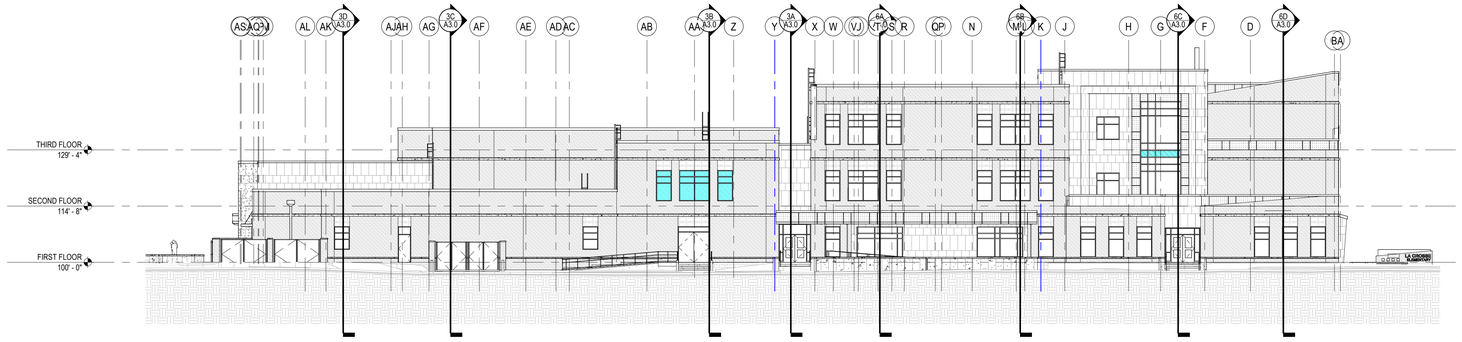
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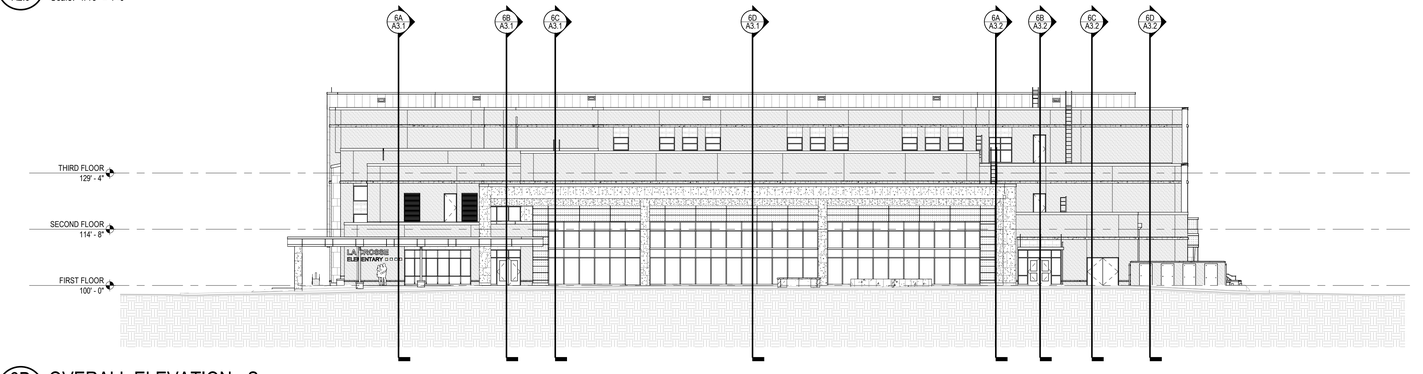
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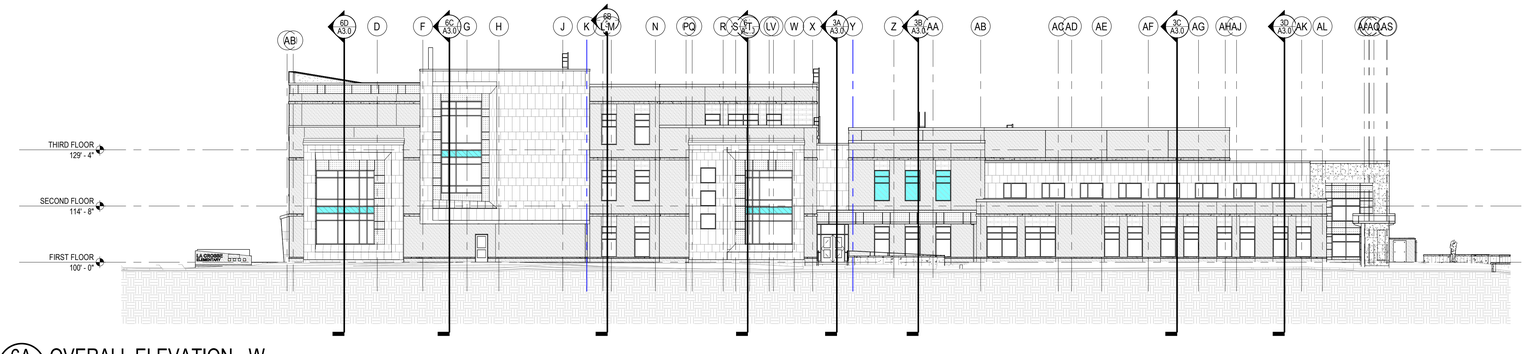

## 6D OVERALL ELEVATION - N
A2.0 Scale: 1/16" = 1'-0"




## 6C OVERALL ELEVATION - E
A2.0 Scale: 1/16" = 1'-0"




## 6B OVERALL ELEVATION - S
A2.0 Scale: 1/16" = 1'-0"




## 6A OVERALL ELEVATION - W
A2.0 Scale: 1/16" = 1'-0"

### FIRST FLOOR ELEVATION GLAZING PERCENTAGE

| | TOTAL FIRST FLOOR ELEVATION SF | TOTAL OPENING SF | OPENING % |
|---|---|---|---|
| NORTH | 3,209 | 896 | 28% |
| EAST | 3,969 | 939 | 24% |
| SOUTH | 3,226 | 1,993 | 64% |
| WEST | 4,155 | 952 | 23% |

*CALCULATION SHOWN FOR LOCAL ZONING REQUIRMENTS

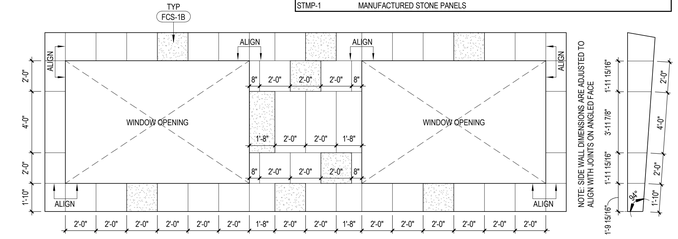
## EXTERIOR MATERIAL LEGEND

- MCM-1A - METAL WALL PANEL, WNX RED
- MCM-1B - METAL WALL PANEL, AMOX BLUE
- MCM-1C - METAL WALL PANEL, BGN GREEN
- MCM-1D - METAL WALL PANEL, EYL YELLOW
- MCM-2 - METAL WALL PANEL, RAL 9001
- FB-1 - FACE BRICK
- BMU-1 - BURNISHED MASONRY UNITS
- FCS-1 - FIBER CEMENT REVEAL PANEL SIDING, PATINA SEE 2B/ A2.0 FOR PATTERN
- STMP-1 - MANUFACTURED STONE PANELS

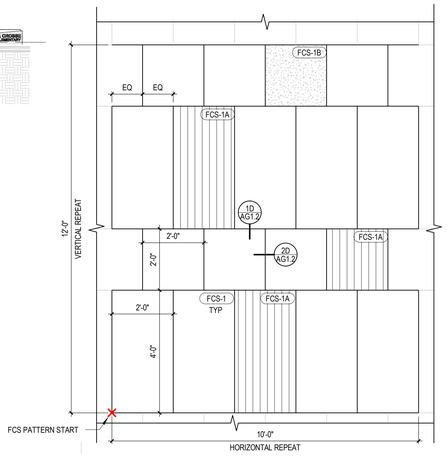
SEE 5E/ A5.6 FOR MASONRY MOVEMENT JOINT DETAILS

## KEYNOTE LEGEND

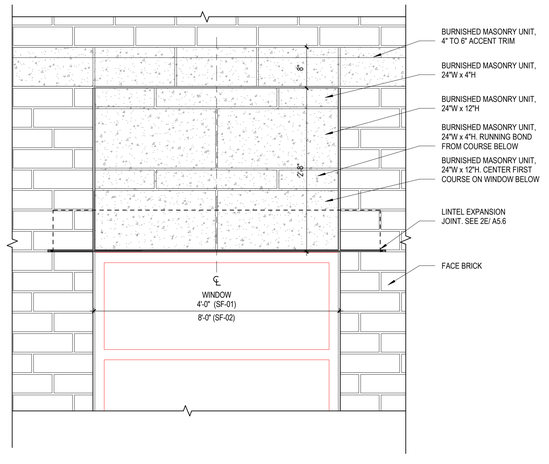
| | |
|---|---|
| BMU-1 | BURNISHED MASONRY UNITS |
| FB-1 | FACE BRICK |
| FCS-1 | FIBER CEMENT REVEAL PANEL SIDING, PATINA |
| FCS-1A | FIBER CEMENT REVEAL PANEL SIDING, PATINA IN LINE |
| FCS-1B | FIBER CEMENT REVEAL PANEL SIDING, PATINA ROUGH |
| MCM-1A | METAL WALL PANEL, WNX RED |
| MCM-1B | METAL WALL PANEL, AMOX BLUE |
| MCM-1C | METAL WALL PANEL, BGN GREEN |
| MCM-1D | METAL WALL PANEL, EYL YELLOW |
| MCM-2 | METAL WALL PANEL, RAL 9001 |
| STMP-1 | MANUFACTURED STONE PANELS |




## 2D FCS PATTERN - WINDOW SEAT DETAIL
A2.0 Scale: 1/4" = 1'-0"




## 2B FCS PATTERN - DETAIL
A2.0 Scale: 1/2" = 1'-0"




## 2A BMU HEADER IN MASONRY WALL
A2.0 Scale: 1" = 1'-0"

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**10/28/2025**

Sheet Title:
**EXTERIOR ELEVATIONS - OVERALL**

Sheet Number:
**A2.0**

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