

# GENERAL STRUCTURAL NOTES

## DESIGN CODES:

WISCONSIN COMMERCIAL BUILDING CODE CONTAINED IN CHS. SPS 361-366 OF THE STATE'S ADMINISTRATIVE CODE EDITION #811 (2021 IBC AS AMENDED)  
ASCE 7-16 - MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES

## DESIGN LOADS:

20 PSF	ROOF DEAD LOAD UNO
40 PSF	GROUND SNOW LOAD (Pg)
	Ce ---- 1.0
	Ct ---- 1.0 TYPICAL
	Ct ---- 1.2 AT CANOPIES
	I ---- 1.0
	Cs ---- 1.0
28 PSF	ROOF SNOW LOAD (PLUS SNOW DRIFTING) TYPICAL
34 PSF	ROOF SNOW LOAD (PLUS SNOW DRIFTING) AT CANOPIES
108 MPH	ULTIMATE DESIGN WIND SPEED
84 MPH	NOMINAL DESIGN WIND SPEED
II	RISK CATEGORY
B	EXPOSURE CATEGORY
+ 0.18	INTERNAL PRESSURE COEFFICIENT
7 FT	END ZONE WIDTH (a)

COMPONENT AND CLADDING	ULTIMATE WIND PRESSURES (PSF)				
	EFFECTIVE AREA	20 FT <sup>2</sup>	50 FT <sup>2</sup>	100 FT <sup>2</sup>	PARAPET
ZONE		WINDWARD	LEEWARD		
INTERIOR ZONE	17.6	16.6	16.0	50.7	29.9
END ZONE	21.0	19.1	17.6	64.9	34.2

1.0	SEISMIC IMPORTANCE FACTOR (I)
II	RISK CATEGORY
D	ASSUMED SITE CLASS
A	SEISMIC DESIGN CATEGORY
0.051g	MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETER (SS)
0.039g	MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETER (S1)
0.055g	DESIGN SPECTRAL RESPONSE ACCELERATION PARAMETER (SDS)
0.063g	DESIGN SPECTRAL RESPONSE ACCELERATION PARAMETER (SD1)
4000 PSF	ALLOWABLE SOIL BEARING PRESSURE

## NOTES:

- IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DESIGN AND PROVIDE ADEQUATE TEMPORARY BRACING AND LATERAL BRACING FOR ALL STRUCTURAL ELEMENTS INCLUDING WALLS, COLUMNS, BEAMS, LINTELS, ETC. FOR WIND, SOIL AND CONSTRUCTION LATERAL AND GRAVITY LOADS. THE BRACING MUST REMAIN IN PLACE UNTIL PERMANENT STRUCTURAL SUPPORTS, INCLUDING ROOF AND FLOOR DIAPHRAGMS ARE INSTALLED AND HAVE REACHED THEIR ALLOWABLE SERVICE STRESSES.
- IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO ENSURE THAT CONSTRUCTION CONFORMS TO THE LATEST OSHA SAFETY REQUIREMENTS. NOTIFY THE ARCHITECT AND ENGINEER IMMEDIATELY IF ANY REVISIONS ARE REQUIRED TO THE DRAWINGS DUE TO OSHA SAFETY REQUIREMENTS.
- GEOTECHNICAL ENGINEER SHALL FIELD VERIFY SOIL BEARING CAPACITY. NOTIFY ARCHITECT AND ENGINEER IF ANY DISCREPANCIES ARE FOUND.
- THIS PROJECT IS NOT DESIGNED FOR FUTURE CONSTRUCTION.
- THE ROOF STRUCTURE HAS BEEN REVIEWED FOR PONDING INSTABILITY BASED ON A 15 MINUTE DURATION/100 YEAR RAINFALL INTENSITY OF 7.0 IN/HR AND A MAXIMUM STATIC HEAD OF 3" AND HYDRAULIC HEAD OF 5" AT THE SECONDARY DRAINAGE INLET. THE PLUMBING CONTRACTOR'S ENGINEER MUST CONFIRM THAT THE PROPOSED SECONDARY DRAINAGE SIZES ARE ADEQUATE TO MAINTAIN SECONDARY DRAINAGE FLOW AS NOTED ABOVE.

## SUBMITTALS:

ALL SUBMITTED STRUCTURAL SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL CONFORMANCE WITH THE INFORMATION GIVEN AND DESIGN CONCEPTS EXPRESSED IN THE CONTRACT DOCUMENTS. REVIEW OF SUCH SUBMITTALS IS NOT CONDUCTED FOR THE PURPOSE OF DETERMINING THE ACCURACY AND COMPLETENESS OF DETAILS SUCH AS DIMENSIONS AND QUANTITIES AS REQUIRED BY THE CONTRACT DOCUMENTS.

## DEFERRED SUBMITTALS:

- THE FOLLOWING ITEMS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL AS DEFERRED SUBMITTALS PER IBC SECTION 107.3.4.1:
- METAL LADDERS AND RAILINGS AND THEIR CONNECTION TO THE BUILDING STRUCTURE
  - METAL CANOPIES AND SUNSHADES AND THEIR CONNECTION TO THE BUILDING STRUCTURE
  - COLD FORMED STEEL FRAMING

FOR ALL DEFERRED STRUCTURAL SUBMITTALS, INCLUDE CALCULATIONS PREPARED AND CERTIFIED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED. SEE PROJECT SPECIFICATIONS FOR ADDITIONAL SHOP DRAWING REQUIREMENTS.

## SOILS AND FOUNDATIONS:

- FOUNDATIONS, SLABS ON GRADE AND OTHER ITEMS RELATED TO THE SOILS ARE DESIGNED AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE RECOMMENDATIONS OF CHOSEN VALLEY TESTING, INC. GEOTECHNICAL EVALUATION REPORT 26630.25.WIL DATED MARCH 11, 2026
- MINIMUM DEPTH FROM EXTERIOR GRADE TO BOTTOM OF BUILDING PERIMETER FOOTINGS SHALL BE 48".
- MINIMUM DEPTH FROM EXTERIOR GRADE TO BOTTOM OF FOOTINGS AT UNHEATED STRUCTURES SHALL BE 60".
- ALL FOOTINGS ARE CENTERED UNDER WALLS, PILASTERS AND COLUMNS ABOVE UNO.
- ALL FOUNDATION WALLS SHALL BE Laterally BRACED BEFORE BACKFILLING AND COMPACTING.
- DO NOT BACKFILL AROUND STEEL COLUMNS UNTIL CORROSION RESISTANT ASPHALTIC COATING HAS BEEN INSTALLED BY THE STRUCTURAL STEEL ERECTOR.
- COORDINATE ALL UNDERGROUND PIPING OR DUCTWORK LOCATIONS AND ELEVATIONS WITH THE MECHANICAL CONTRACTOR. ADJUST FOOTING ELEVATIONS AND PROVIDE SLEEVES PER WALL FOOTING AT UNDERGROUND LINES DETAIL. FOR PIPING OR DUCTWORK PARALLEL TO FOOTING, BOTTOM OF FOOTING ELEVATION SHOULD BE NO HIGHER THAN THE PIPE OR DUCTWORK INVERT ELEVATION PLUS THE HORIZONTAL DISTANCE BETWEEN EDGE OF FOOTING AND EDGE OF PIPING OR DUCTWORK. CONTRACTOR SHALL NOTIFY ARCHITECT AND ENGINEER FOR APPROVAL OF ANY DEVIATIONS PRIOR TO COMMENCING WORK.

## CAST-IN-PLACE CONCRETE:

- DESIGN CODES: IBC CHAPTER 19 & ACI 318-19, BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE & ACI DETAILING MANUALS LATEST EDITION.
- MINIMUM 28-DAY COMPRESSIVE STRENGTH SHALL BE:
 

TYPE	STRENGTH	LOCATION
A	f <sub>c</sub> =3000 PSI	MASONRY CORE FILL AND BOND BEAMS WITH 3/8" MAX AGGREGATE SIZE
B	f <sub>c</sub> =4000 PSI	INTERIOR SLABS ON GRADE WITH 3/4" MAX AGGREGATE SIZE
C	f <sub>c</sub> =3000 PSI	FOOTINGS
D	f <sub>c</sub> =4000 PSI	EXTERIOR PIERS & FOUNDATION WALLS - AIR ENTRAINED 6% +/- 1.5% - MAX WATER/CEMENT RATIO = 0.50
E	f <sub>c</sub> =4000 PSI	INTERIOR PIERS & FOUNDATION WALLS
F	f <sub>c</sub> =4500 PSI	EXPOSED EXTERIOR CONCRETE - AIR ENTRAINED 6% +/- 1.5% - MAX WATER/CEMENT RATIO = 0.45

- REINFORCING STEEL PROPERTIES:
  - DEFORMED BARS - ASTM A615 GRADE 60 UNO
  - WELDED WIRE FABRIC - ASTM A1064
- ALL REINFORCING SHALL BE DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH THE LATEST ACI MANUALS.
  - FOR PIERS WITH A CROSS SECTION LARGER THAN 18"x18" AND WITH MORE THAN 4 LONGITUDINAL BARS, ADDITIONAL CROSS TIES (TYPE 19 PER ACI 315) OF SAME SIZE AND SPACING AS MAIN TIES, SHALL BE PROVIDED TO ENSURE THAT ALL NON CORNER BARS ARE Laterally SUPPORTED BY THE CORNER OF A TIE
- SHOP DRAWINGS FOR REINFORCING STEEL AND CONCRETE MIX DESIGNS SHALL BE SUBMITTED FOR APPROVAL.
- PLACE ALL ITEMS PER APPROVED SHOP DRAWINGS AND APPROVED MIX DESIGNS.
- PROVIDE ALL ACCESSORIES, CHAIRS, SPACER BARS AND SUPPORTS NECESSARY TO SECURE REINFORCING PER "ACI DETAILING MANUAL" NO OTHER METHODS OR MATERIALS WILL BE ACCEPTABLE.
- PROVIDE PLASTIC CHAIRS AND SUPPORTS IN ALL AREAS OF EXPOSED CONCRETE.
- PROVIDE MINIMUM CONCRETE COVER FOR ALL REINFORCING AS FOLLOWS UNO:
 

BAR SIZE	6" CMU	8" CMU	10" CMU & WIDER
#4	24"	24"	24"
#5	30"	30"	30"
#6	38"	36"	36"
#7	42"	42"	42"

BARS CENTERED IN WALL				
BAR SIZE	6" CMU	8" CMU	10" CMU & WIDER	
#4	24"	24"	24"	
#5	30"	30"	30"	
#6	38"	36"	36"	
#7	42"	42"	42"	

BARS PLACED AT INSIDE OR OUTSIDE FACE OF WALL				
BAR SIZE	6" CMU	8" CMU	10" CMU & WIDER	
#4	24"	24"	24"	
#5	30"	30"	30"	
#6	37"	37"	37"	
#7	43"	43"	43"	

- FOOTING WIDTH THICKNESS LONG REINF TRANSV REINF
 

1'-8" OR LESS	10"	(2) #5	-----
1'-9" TO 2'-8"	12"	(2) #5	-----
2'-9" TO 3'-4"	14"	(3) #5	-----
- PROVIDE FOOTING DOWELS FOR CONCRETE OR MASONRY WALLS AND COLUMNS. DOWELS SHALL BE SAME SIZE AND SPACING AS WALL AND COLUMN VERTICAL REINFORCING AND SHALL PROJECT FROM FOOTINGS TO PROVIDE LAP SPLICES AS INDICATED BELOW OR IN THE MASONRY NOTES. PROVIDE 90 DEGREE STANDARD HOOK IN COLUMN AND FREE STANDING WALL FOOTING DOWELS UNO.
- PROVIDE LAP SPLICES FOR CONCRETE REINFORCING AS FOLLOWS:
  - TYPICAL LAP SPLICES SHALL BE UNO:
 

#5 BARS AND SMALLER	44 BAR DIAMETERS
#7 THROUGH #11 BARS	56 BAR DIAMETERS
  - THE MINIMUM LAP SPLICE LENGTH FOR ALL SPLICES FOR CONCRETE REINFORCING SHALL BE 24"

WALL THICKNESS	VERT REINF	HORIZ REINF
6"	#4 @ 16" O.C. CTRD	#4 @ 16" O.C. CTRD
11"	#4 @ 16" O.C. EF	#4 @ 16" O.C. EF
14" AND 15"	#4 @ 12" O.C. EF	#4 @ 12" O.C. EF
20"	#4 @ 12" O.C. EF	#5 @ 15" O.C. EF

- PROVIDE EXTRA REINFORCING ON ALL SIDES OF ALL CONCRETE WALL AND SLAB OPENINGS EQUAL TO ONE HALF THE INTERRUPTED REINFORCING IN EACH DIRECTION, UNO ON THE DRAWINGS. A MINIMUM OF (1) #5 SHALL BE PROVIDED FOR EACH LAYER OF REINFORCEMENT. EXTEND BARS PAST EDGE OF OPENING THE LAP SPLICE NOTED ABOVE. PROVIDE AN ADDITIONAL #4x4-0" DIAGONAL BAR AT EACH CORNER FOR EACH LAYER OF REINFORCING.
- HOT WEATHER AND COLD WEATHER CONCRETE PROVISIONS OF ACI 308R-10 AND ACI 308R-16 SHALL BE FOLLOWED.
- PROVIDE CONCRETE WALL VERTICAL CONTROL JOINTS FOR WALLS AS FOLLOWS UNO:
 

WALL THICKNESS	MAXIMUM CONTROL JOINT SPACING
6" TO 11"	24'-0" O.C.
14" TO 20"	34'-0" O.C.

## MASONRY:

- DESIGN CODES: IBC CHAPTER 21, TMS 402-16 BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES AND TMS 602-16, SPECIFICATION FOR MASONRY STRUCTURES.
- HOLLOW CONCRETE MASONRY UNITS SHALL BE NORMAL WEIGHT UNITS PER ASTM C90 REQUIREMENTS. F<sub>m</sub>=2000 PSI.
- ALL MORTAR FOR WALLS ABOVE GRADE SHALL BE TYPE 'S'.
- MORTAR PER ASTM C270. MINIMUM 28 DAY COMPRESSIVE STRENGTH: TYPE 'S'=1800 PSI
- COARSE GROUT PER ASTM C476. NO GROUT IS TO BE HAND MIXED ON SITE.
- MASONRY CEMENT SHALL NOT BE USED IN MORTAR OR GROUT.
- REINFORCING STEEL:
  - DEFORMED BARS: ASTM A615 GRADE 60
  - JOINT REINFORCEMENT: ASTM A1064
- SUBMIT MORTAR AND GROUT DESIGN MIXES FOR APPROVAL.
- SUBMIT SHOP DRAWINGS FOR REINFORCING STEEL.
- WIRE JOINT REINFORCING FOR CONCRETE BLOCK WALLS SHALL BE STANDARD CORROSION RESISTANT 9 GAGE LADDER TYPE (PROVIDE TRIPOD LADDER FOR MULTIPLE WALLS). INSTALL JOINT REINFORCING PER MANUFACTURER'S RECOMMENDATIONS AT A MAXIMUM SPACING 16" O.C. UNO ON DRAWINGS OR PROJECT SPECIFICATIONS.
- INSTALL MASONRY VERTICAL AND HORIZONTAL REINFORCING PER IBC REQUIREMENTS AND AS FOLLOWS:
  - SEE PLANS, DETAILS AND SCHEDULES FOR REQUIRED REINFORCING
  - PROVIDE FULL MORTAR BEDDED FACE SHELLS AND WEBS AROUND ALL GROUTED CELLS FOR BEARING AND TO PREVENT LEAKAGE INTO ADJACENT CELLS
  - PLACE VERTICAL REINFORCING PRIOR TO PLACING GROUT.
  - PROVIDE REINFORCEMENT POSITIONERS TO HOLD VERTICAL REINFORCEMENT IN PROPER POSITION AT TOP AND BOTTOM OF ALL LENGTHS OF REINFORCING BARS TO BE COORDINATED WITH CONCRETE AND MASONRY CONTRACTORS
  - GROUT ALL CORES CONTAINING REINFORCING SOLID
  - MASONRY WALL CELLS TO BE GROUTED WITH CONCRETE SHALL BE FILLED IN MAXIMUM 4'-0" LIFTS. REMOVAL ALL OVERHANGING OBSTRUCTION AND DEBRIS FROM THE CELLS PRIOR TO GROUTING
  - CONSOLIDATE ALL GROUT BY RODDING OR VIBRATING
  - PROVIDE REINFORCEMENT LAP SPLICES PER TABLES BELOW:
 

BARS CENTERED IN WALL				
BAR SIZE	6" CMU	8" CMU	10" CMU & WIDER	
#4	24"	24"	24"	
#5	30"	30"	30"	
#6	38"	36"	36"	
#7	42"	42"	42"	

BARS PLACED AT INSIDE OR OUTSIDE FACE OF WALL				
BAR SIZE	6" CMU	8" CMU	10" CMU & WIDER	
#4	24"	24"	24"	
#5	30"	30"	30"	
#6	37"	37"	37"	
#7	43"	43"	43"	

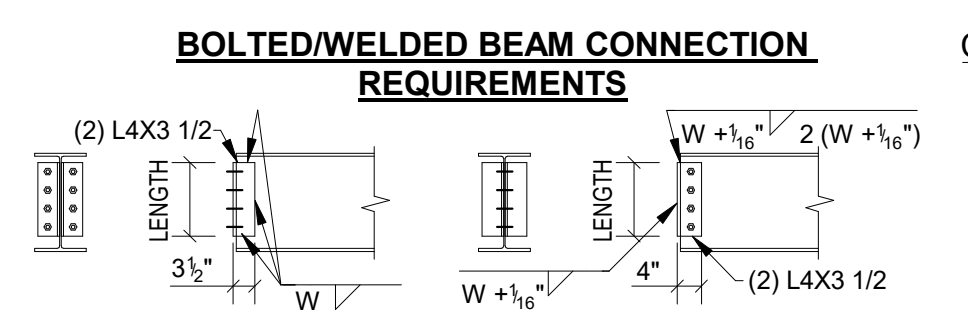
- PROVIDE CORNER BAR WITH 2'-0" x 2'-0" LEGS FOR EACH HORIZONTAL BOND BEAM BAR AT CORNERS AND INTERSECTING WALLS
- MASONRY VERTICAL CONTROL JOINTS:
  - COORDINATE JOINTS AND CONSTRUCTION DETAILS WITH ARCHITECT. MAXIMUM JOINT SPACING IS 24'-0"
  - LOCATE JOINTS 2'-0" FROM ALL CORNERS AND EDGE OF ALL OPENING EXCEPT AS NOTED PER SECTIONS AND DETAILS
  - FOR LOAD BEARING MASONRY, CONTINUE BOND BEAM REINFORCING AND GROUT THROUGH CONTROL JOINTS
  - FORM CONTROL JOINTS IN BLOCK WITH JAMB BLOCK WITH NEOPRENE INSERTS, OR ROD AND CALK. CONTRACTOR OPTION: USE STANDARD BLOCK, LINE ONE SIDE WITH BUILDING PAPER AND GROUT SOLID TO FORM KEYWAY

## STEEL - STRUCTURAL:

- DESIGN CODES: IBC CHAPTER 22, AISC 360-16, SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS AND AISC 303-16, CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIGDES.
- STEEL PROPERTIES:
  - ASTM A992 (F<sub>y</sub>=50 KSI) FOR WIDE FLANGE SHAPES
  - ASTM A500 GRADE C (F<sub>y</sub>=50 KSI) FOR TUBULAR MEMBERS
  - ASTM A36 STEEL (F<sub>y</sub>=36 KSI) FOR ALL OTHER MEMBERS UNO
  - ASTM F3125 GRADE A325 OR GRADE F1952 UNO FOR CONNECTION BOLTS
  - ASTM F1554 GRADE 36 FOR ANCHOR BOLTS UNO
  - WELDS = AMERICAN WELDING SOCIETY (AWS) E70XX ELECTRODES
- PROVIDE 5000 PSI NON-SHRINK GROUT UNDER ALL STEEL BASE AND BEARING PLATES PER THE SECTIONS AND DETAILS. GROUT PLATES PRIOR TO LOADING ROOF MEMBERS WITH PRECAST PLANK OR DECKING MATERIALS.
- PROVIDE CORROSION RESISTANT ASPHALTIC COATING OR EQUAL AROUND ALL STRUCTURAL STEEL BELOW GRADE.
- ALL WELDS SHALL BE MADE BY CERTIFIED WELDERS AND IN ACCORDANCE WITH AMERICAN WELDING SOCIETY (AWS) - D1.1 LATEST EDITION.
- STRUCTURAL STEEL FABRICATOR SHALL FURNISH ALL PLATES AND ANGLES CAST IN CONCRETE AND MASONRY FOR CONNECTING STEEL JOISTS, BEAMS, COLUMNS, DECK, ETC UNO. SEE APPLICABLE NOTES ON THIS SHEET AND COORDINATE WITH MASONRY AND CONCRETE CONTRACTORS.
- ALL COLUMN BASE AND CAP PLATES TO BE WELDED AROUND ALL SIDES.
- PRIME PAINT PER AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS SECTION 6.5, UNO ON ANY CONSTRUCTION DOCUMENTS.
- STRUCTURAL STEEL EXPOSED TO WEATHER SHALL BE HOT-DIP GALVANIZED. TOUCH-UP WELDS AND REPAIR HOT-DIP GALVANIZED COATING PER ASTM A780.
- STRUCTURAL STEEL FABRICATOR SHALL PROVIDE HOLES FOR JOIST ERECTION BEAMS AS REQUIRED BY OSHA. HOLES SHALL BE 9/16" FOR K-SERIES JOISTS.
- STRUCTURAL STEEL FABRICATOR SHALL SUBMIT SHOP DRAWINGS FOR APPROVAL.
- MISCELLANEOUS STEEL FABRICATOR SHALL SUBMIT SHOP DRAWINGS FOR APPROVAL, INCLUDING STRUCTURAL CALCULATIONS FOR STAIRS, LADDERS, RAILINGS AND THEIR CONNECTIONS CERTIFIED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED.

## STEEL - STRUCTURAL (CONT):

- PROVIDE DOUBLE ANGLE SIMPLE SHEAR CONNECTIONS AS SHOWN BELOW. ALTERNATE CONNECTIONS WILL BE ALLOWED PROVIDED THAT CONNECTION DESIGN CALCULATIONS, CERTIFIED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED, ARE SUBMITTED BY THE FABRICATOR FOR REVIEW.



BEAM SIZE	# OF ROWS OF 3/4"Ø BOLTS	CONN ANGLES		WELD SIZE
		THICKNESS	LENGTH	
W36	10	3/8"	2'-5 1/2"	1/4"
W33	9	3/8"	2'-2 1/2"	1/4"
W30	8	3/8"	1'-11 1/2"	1/4"
W27	7	3/8"	1'-8 1/2"	1/4"
W24	6	3/8"	1'-5 1/2"	1/4"
W21	5	3/8"	1'-2 1/2"	1/4"
W18	4	3/8"	0'-11 1/2"	1/4"
W16	4	5/16"	0'-11 1/2"	1/4"
W14	3	5/16"	0'-8 1/2"	1/4"
W12	3	5/16"	0'-8 1/2"	1/4"
W10	2	5/16"	0'-5 1/2"	1/4"
W8	2	5/16"	0'-5 1/2"	1/4"

- NOTES:**
- USE E70XX ELECTRODES.
  - BOLTS SHALL BE TIGHTENED IN PROPERLY ALIGNED HOLES TO A SNUG-TIGHT CONDITION AS DEFINED BY THE LATEST EDITION OF AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS" UNO.
  - USE BENT PLATES AS REQUIRED AT SKEWED BEAMS.
  - CONTRACTOR OPTION TO USE ALL-BOLTED OR ALL-WELDED CONNECTIONS.

## STEEL - JOISTS:

- DESIGN CODE: STEEL JOIST INSTITUTE (SJI) "STANDARD SPECIFICATIONS, LOAD TABLES AND WEIGHT TABLES FOR STEEL JOISTS AND JOIST GIRDERS" LATEST EDITION.
- ALL JOISTS SHALL BE CAMBERED PER SJI SPECIFICATIONS UNO.
- PROVIDE END BEARING DEPTH AND BEARING LENGTH PER SJI SPECIFICATIONS, UNO.
- WHERE JOISTS BEARS ON A BEAM ON ONE SIDE ONLY, BEARING LENGTH TO EQUAL TO THE BEAM FLANGE WIDTH, WHERE JOISTS BEAR ON A BEAM ON BOTH SIDES, BUTT JOISTS OR PROVIDE A 1/2" MAX GAP, BUT MAINTAINING THE REQUIRED MINIMUM JOIST BEARING LENGTH. PROVIDE EXTENDED ENDS WHERE REQUIRED OR SHOWN ON DRAWINGS.
- STEEL JOISTS LOCATED AT COLUMNS AND ALL JOISTS 40'-0" OR GREATER IN LENGTH SHALL BE BOLTED TO BEAMS TO MEET OSHA REQUIREMENTS. ERECT BOLTED JOISTS FIRST AND WELD WHEN OTHER JOISTS ARE INSTALLED. IF JOIST IS NOT LOCATED AT COLUMN CENTER LINE, PROVIDE OSHA BOLTED CONNECTION AT FIRST JOIST EACH SIDE OF COLUMN.
- FIELD WELD ALL STEEL JOISTS TO BEAMS, COLUMNS, ANGLES OR PLATES CAST IN MASONRY OR CONCRETE PER SJI SPECIFICATIONS UNO.
- ALL STEEL JOIST BRIDGING AND ITS ANCHORAGE SHALL BE DESIGNED AND PROVIDED BY JOISTS MANUFACTURER PER SJI SPECIFICATIONS UNO. COORDINATE LOCATIONS WITH MECH CONTRACTOR TO MISS SPRINKLER HEADS AND DUCTWORK.
- THE JOIST MANUFACTURER SHALL BE RESPONSIBLE FOR AND SHALL SUBMIT SHOP DRAWINGS FOR APPROVAL. CALCULATIONS FOR ALL SPECIAL JOISTS SHALL BE CERTIFIED AND SUBMITTED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED.
- THE DESIGN OF ALL JOIST MEMBERS AND ELEMENTS SHOWN ON THE DRAWINGS ARE FOR THE IN-PLACE COMPLETED BUILDING, ALL LOADING CRITERIA AND VERIFICATION OF DESIGN FOR LOADING, SUCH AS HANDLING, TRANSPORTATION AND ERECTION ARE THE SOLE RESPONSIBILITY OF THE JOIST MANUFACTURER.
- JOISTS AND STRUCTURAL STEEL FABRICATORS SHALL SHOW ALL FIELD WELDING REQUIREMENTS ON SHOP DRAWINGS.
- STEEL JOISTS SHALL BE SHOP PRIMED WITH STANDARD PRIMER UNO. VERIFY COLOR WITH ARCHITECTURAL DRAWINGS.
- STEEL ROOF JOISTS ARE DESIGNED FOR THE FOLLOWING SPRINKLER PIPE LOADS:
  - A SINGLE 4" DIAMETER MAIN RUNNING PARALLEL TO JOIST OR A SINGLE 6" MAIN TRAPEZED BETWEEN TWO JOISTS, OR
  - A SINGLE 4" MAIN RUNNING PERPENDICULAR TO JOIST AND HUNG AT A MINIMUM OF EVERY OTHER JOIST OR A SINGLE 6" MAIN RUNNING PERPENDICULAR TO JOIST AND HUNG AT EVERY JOIST
  - FIRE PROTECTION CONTRACTOR SHALL ARRANGE SPRINKLER PIPES TO SATISFY JOIST DESIGN OR PROVIDE SPRINKLER PIPE LAYOUT AND LOADS AS WATER FILLED PIPE TO JOIST SUPPLIER TO DESIGN SPRINKLER PIPES AS ADDED JOIST LOADS

## STEEL - DECK:

- DESIGN CODE: STEEL DECK INSTITUTE (SDI) "DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS AND ROOF DECKS" LATEST EDITION.
- STEEL ROOF DECK SHALL CONFORM TO EITHER ASTM A653 OR A1063 FOR DECK THAT IS GALVANIZED AND TO EITHER ASTM A1008 OR A1039 FOR DECK THAT IS NOT GALVANIZED. DECK SHALL HAVE A MINIMUM YIELD STRENGTH OF 50 KSI. DECK SIZE, TYPE AND GAUGE TO BE AS INDICATED ON PLANS.
- FASTEN STEEL DECK TO ALL INTERIOR STEEL SUPPORTS AS SHOWN ON PLANS. PROVIDE FASTENERS AT 6" O.C. TO PLATE, ANGLE OR BEAM AT BUILDING PERIMETER UNO. SIDELAP FASTENING SHALL BE AS INDICATED ON PLANS.
- ALL WELDS SHALL BE MADE BY CERTIFIED WELDERS AND IN ACCORDANCE WITH AMERICAN WELDING SOCIETY (AWS) - D1.3 LATEST EDITION. ALL HILTI PAF'S SHALL BE FASTENED BY TRAINED AND CERTIFIED INSTALLERS ON THE GENERAL USE OF POWDER-ACTUATED TECHNOLOGY FOR ATTACHMENT OF THE STEEL DECK.
- SPAN DECK SECTION MINIMUM (3) SPANS CONTINUOUS, UNO.
- THE STEEL DECK FABRICATOR SHALL SUBMIT SHOP DRAWINGS FOR APPROVAL.

## STEEL - DECK (CONT):

- STEEL ROOF DECK TO BE SHOP PRIMED WITH STANDARD PRIMER UNO. VERIFY COLOR WITH ARCHITECTURAL DRAWINGS.
- STEEL ROOF DECK SHALL HAVE GALVANIZED FINISH EQUAL TO G30 MINIMUM.

## COLD FORMED STEEL FRAMING:

- THE INFORMATION SHOWN ON THE STRUCTURAL DRAWINGS IS ONLY CONCEPTUAL AND IS INTENDED TO EXPRESS MINIMUM COLD-FORMED STEEL SYSTEM MATERIAL DESIGN AND CONSTRUCTION PERFORMANCE CRITERIA. THE COLD-FORMED STEEL SYSTEM SUPPLIER SHALL PROVIDE AND SUBMIT FOR APPROVAL THE FINAL STRUCTURAL ENGINEERING DESIGN WHICH INCLUDES DETAILED SHOP DRAWINGS AND CALCULATIONS OF THE MEMBER SIZES, THICKNESS, SPACING AND ALL CONNECTIONS. ALL SUBMITTALS MUST BE CERTIFIED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT.
- DESIGN CODES:
  - IBC CHAPTER 22, SECTION 2210 AND 2211-COLD FORMED STEEL AND LIGHT-FRAMING CONSTRUCTION. SEE PLANS FOR SIZE AND THICKNESS OF STUDS
  - 2016 AISI NORTH AMERICAN SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS
- SEE ARCH FOR REQUIRED SIZE AND MINIMUM THICKNESS OF STUDS. YIELD STRESS SHALL BE 33KSI FOR 33 MIL AND 43 MIL MEMBERS AND 50KSI FOR ALL OTHER MEMBERS.
 

MEMBER DEPTH IN HUNDREDS OF INCHES (EX: 600=6")	FLANGE WIDTH OF MEMBER IN HUNDREDS OF INCHES (EX: 162=1' 5/8", 200=2')
600S162-43	
- ALL STEEL STUD FASTENERS SHALL BE WELDED CONNECTIONS, OR SHALL BE #10-16 "TEK" SCREWS MANUFACTURED BY "ITW BUILD-EX" OR APPROVED EQUAL UNO. ALL WELDS SHALL BE MADE BY CERTIFIED WELDERS AND IN ACCORDANCE WITH AMERICAN WELDING SOCIETY (AWS)-D13 LATEST EDITION. THE CENTER-TO-CENTER SPACING AND EDGE DISTANCE FOR ALL SCREW FASTENERS SHALL BE AT LEAST 3X (NOMINAL DIAMETER) UNO.
- ALL POWDER ACTUATED FASTENERS (PAFS) SHALL BE HILTI "X-UNO" OR APPROVED EQUAL UNO.
- SPICES IN WALL STUDS, JOISTS OR HEADERS SHALL NOT BE PERMITTED UNLESS SPICE CONNECTIONS ARE DESIGNED AND DETAILED BY SUPPLIER.
- ALL FRAMING COMPONENTS SHALL BE CUT SQUARELY FOR ATTACHMENT TO PERPENDICULAR MEMBERS, DO NOT TORCH CUT. MEMBERS SHALL BE HELD POSITIVELY IN PLACE UNTIL PROPERLY FASTENED.
- STUDS SHALL BE PLUMBED, ALIGNED AND SECURELY ATTACHED TO THE FLANGE OR WEB OF BOTH UPPER AND LOWER TRACKS. STUDS PRIOR TO STUD AND BEARING AGAINST INSIDE TRACK WEB SHOULD BE HELD AGAINST TRACK ATTACHMENT.
- WALL STUD BRIDGING SHALL BE ATTACHED IN A MANNER TO PREVENT STUD ROTATION. BRIDGING ROWS SHALL BE SPACED ACCORDING TO THE MANUFACTURER'S RECOMMENDATION. THE MINIMUM BRIDGING SHALL BE 6" ON C. FOR WIND LOADED WALLS AND 4'-0" O.C. FOR AXIAL LOADED WALLS.
- CONTINUOUS BOTTOM RUNNER TRACKS FOR ALL EXTERIOR WALLS SHALL BE ANCHORED TO FOUNDATION WALLS WITH POWDER ACTUATED FASTENERS (PAF), MAXIMUM SPACING TO BE 16" O.C.
- CONTRACTOR IS RESPONSIBLE FOR AVOIDING THE OVERLOAD OF COLD FORMED STEEL MEMBERS DURING ERECTION AND FOR PROVIDING ADEQUATE TEMPORARY BRACING UNTIL THE PERMANENT BRIDGING AND SHEATHING ARE IN PLACE.
- TOUCH-UP WELDS AND REPAIR HOT-DIP GALVANIZED COATING PER ASTM A780.

MILS TO GAUGE CONVERSION CHART		
MILS		GAUGE
33		20 GA
43		18 GA
54		16 GA
68		14 GA
97		12 GA

- SPECIAL INSPECTION AND TESTING:
  - SPECIAL INSPECTION AND TESTING IS REQUIRED IN ACCORDANCE WITH THE LATEST EDITION OF IBC, CHAPTER 17, FOR THE FOLLOWING PORTIONS OF CONSTRUCTION:
    - CONCRETE
    - SOILS: GEOTECHNICAL ENGINEER SHALL FIELD VERIFY SOIL BEARING CAPACITY AND COMPACTION PER THE REQUIREMENTS OF THE GEOTECHNICAL REPORT
    - CAST-IN-PLACE ANCHORS IN CONCRETE (PER PRODUCT ICC-ES REQUIREMENTS)
    - REINFORCING STEEL IN CONCRETE
    - REINFORCING STEEL IN MASONRY - LEVEL 2
    - STRUCTURAL WELDING - VISUAL TESTING (UNO)
    - STRUCTURAL WELDING - ULTRASONIC OR RADIOGRAPHIC
    - TESTING (ALL FULL OR PARTIAL PENETRATION WELDS)
    - HIGH STRENGTH BOLTING

PROJECT

**MAYO EMPLOYEES  
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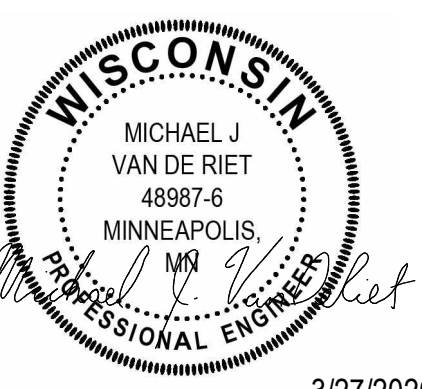
NEW BUILDING

605 WEST AVE S.  
LA CROSSE, WI

ISSUED SET 3/27/2026

REVISIONS

DATE NO



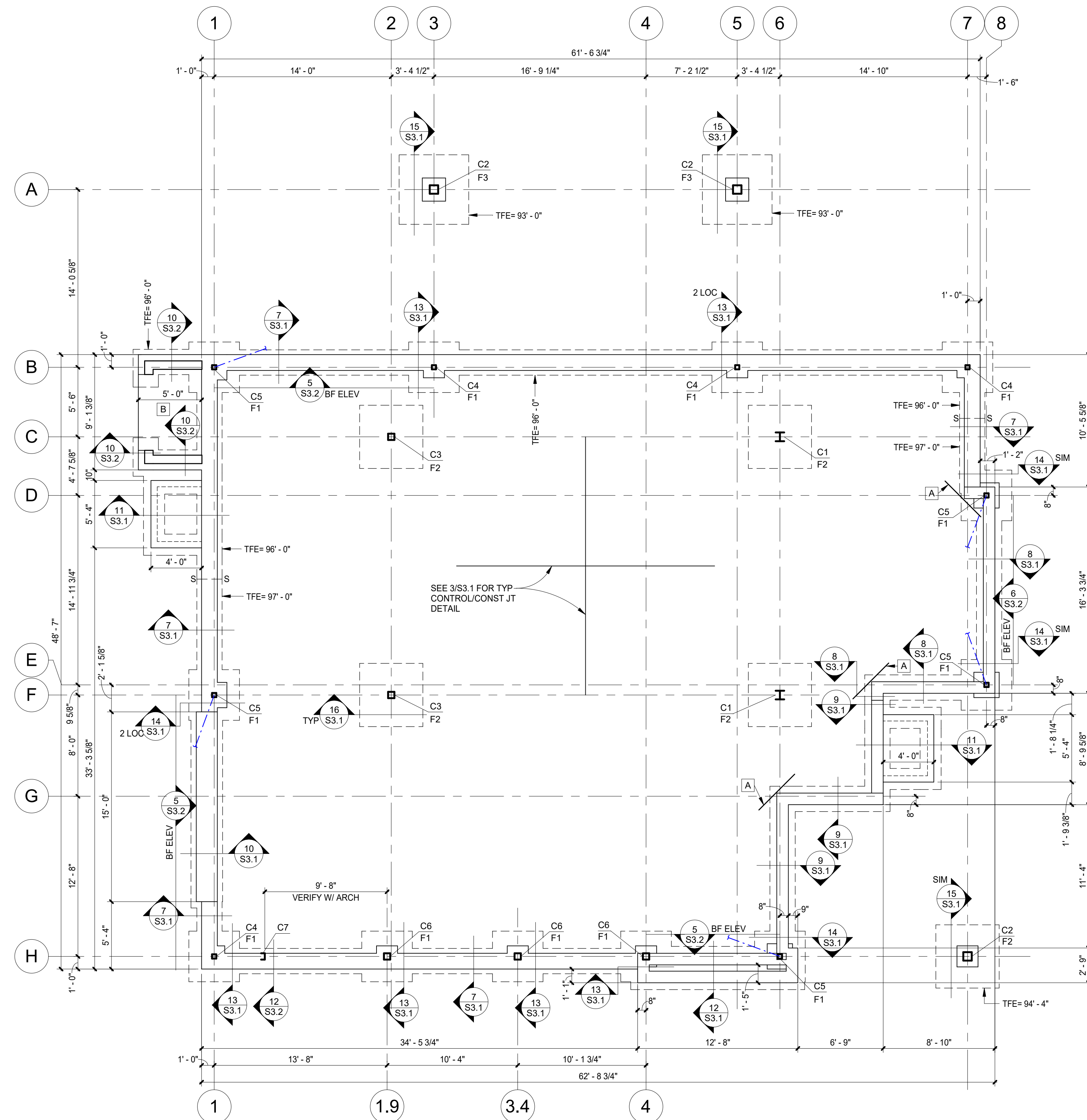
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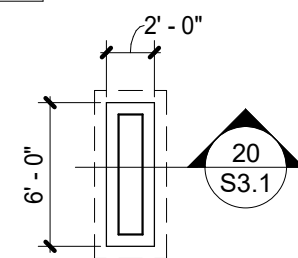
FOUNDATION PLAN

Drawn By: JMW Checked By: BRP

# S1.1



SEE ARCH CIVIL FOR EXACT LOCATION ON SITE



2 MONUMENT SIGN FOUNDATION PLAN  
S1.1 1/8" = 1'-0"

FOUNDATION PLAN NOTES:

- VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
- SEE ARCH DRAWINGS FOR SIZE AND LOCATION OF ALL WALL OPENINGS.
- FLOOR SLAB SHALL BE 4" MINIMUM CONCRETE SLAB ON GRADE WITH 6X6-W1.4XW1.4 WWF. FINISHED FLOOR ELEVATION (FFE) = 100'-0". VERIFY ALL DEPRESSED AND SLOPED SLABS WITH ARCH DRAWINGS AND MECH/ELEC CONTRACTORS.
- ALL CONTINUOUS WALL FOOTINGS SHALL BE 2'-0"W X 1'-0"D WITH (2) #5 CONTINUOUS BOTTOM UNO.
- TOP OF EXTERIOR FOOTING ELEVATION (TFE) = 97'-0" UNO.
- TOP OF INTERIOR FOOTING ELEVATION (TFE) = 99'-0" UNO.
- S---S ON PLAN DENOTES STEPPED FOOTING. SEE SECTION 1/S3.1.
- SEE DETAIL 2/S3.1 FOR DROPPED FOOTING AT ALL UNDERGROUND MECHANICAL LINES ENTERING BUILDING.
- SEE DETAILS 4/S3.1, 5/S3.1 AND 6/S3.1 FOR TYPICAL FOUNDATION WALL JOINTS AND CORNER REINFORCING.
- "CX" ON PLAN DENOTES STEEL COLUMN SIZE. SEE STEEL COLUMN SCHEDULE ON THIS SHEET.
- "FX" ON PLAN DENOTES FOOTING SIZE. SEE FOOTING SCHEDULE ON THIS SHEET.
- SEE SECTIONS AND DETAILS FOR SIZE OF CAST-IN-PLACE CONCRETE FOUNDATION WALLS. SEE GENERAL NOTES FOR REINFORCING REQUIREMENTS.
- EXTEND ALL FOOTINGS MINIMUM 1'-0" BEYOND END OF FOUNDATION WALLS UNO.

FOUNDATION KEYNOTES:

- [A] PROVIDE (2) #4 X 4'-0" DIAGONAL BARS IN SLAB ON GRADE AT INSIDE CORNERS.
- [B] 6" MINIMUM CONCRETE SLAB ON GRADE WITH #4 @ 12" O.C. EACH WAY CENTERED. SEE ARCH FOR TOP OF SLAB ELEVATION (TSE) AND SLOPES.

PLAN TRUE NORTH  
1 FOUNDATION PLAN  
S1.1 3/16" = 1'-0"

STEEL COLUMN SCHEDULE			
MARK	TYPE	BASE PLATE	REMARKS
C1	W8X40	3/4"X14"X14"	
C2	HSS8X8X1/4	1"X16"X16"	
C3	HSS6X6X3/8	3/4"X12"X12"	
C4	HSS4X4X1/4	3/4"X10"X10"	
C5	HSS4X4X1/4	3/4"X12"X12"	
C6	HSS6X6X1/4	3/4"X12"X12"	
C7	MCSX15.3	1/2"X6"X6"	

FOOTING SCHEDULE			
MARK	SIZE	BOT REINF EW	REMARKS
F1	4'-0"X4'-0"X12"	(5) #5	
F2	5'-0"X5'-0"X12"	(6) #5	
F3	5'-6"X5'-6"X12"	(6) #5	

PROJECT

**MAYO EMPLOYEES  
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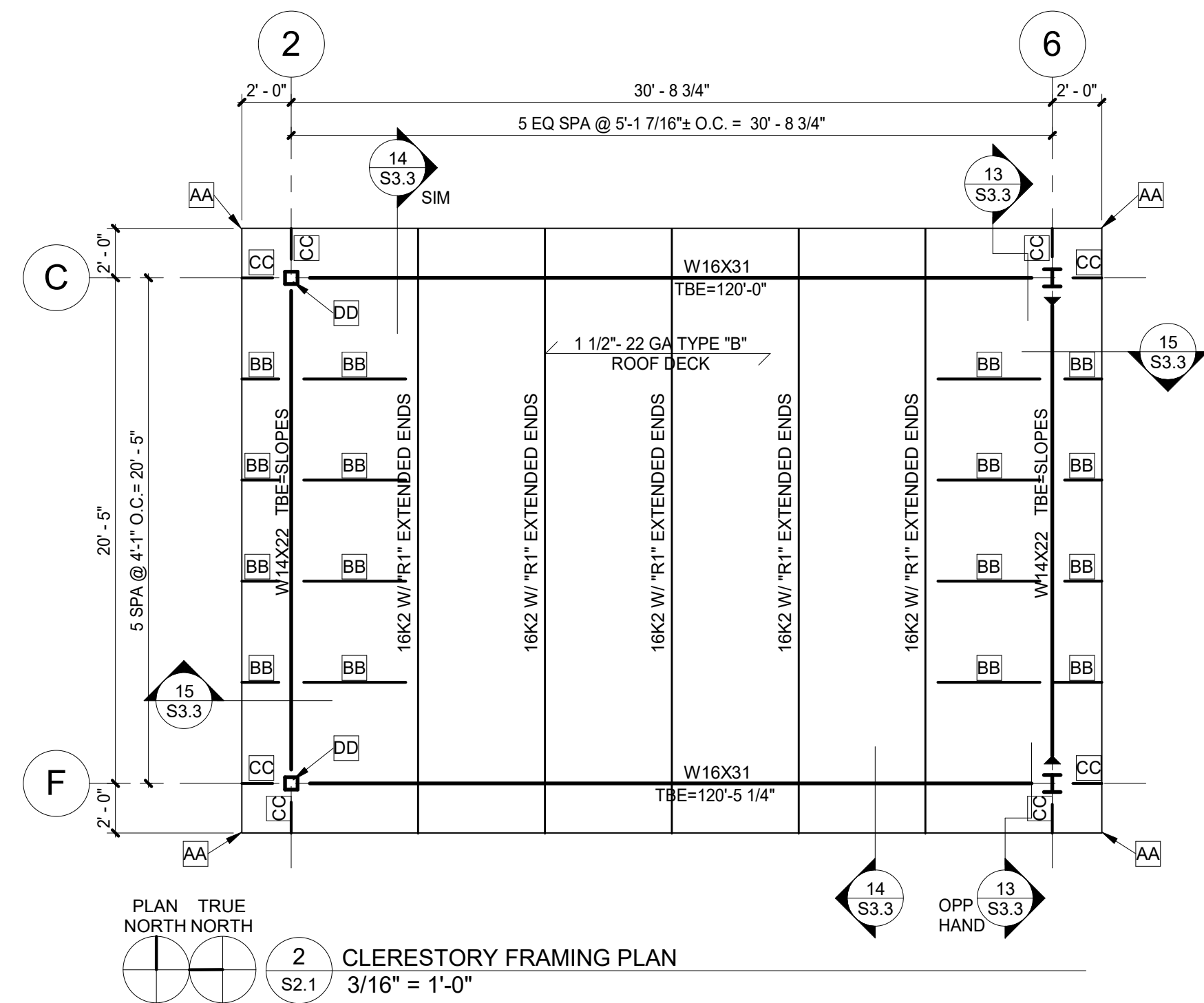
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ROOF FRAMING PLAN

Drawn By: JMW Checked By: BRP

# S2.1

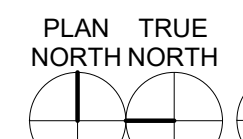
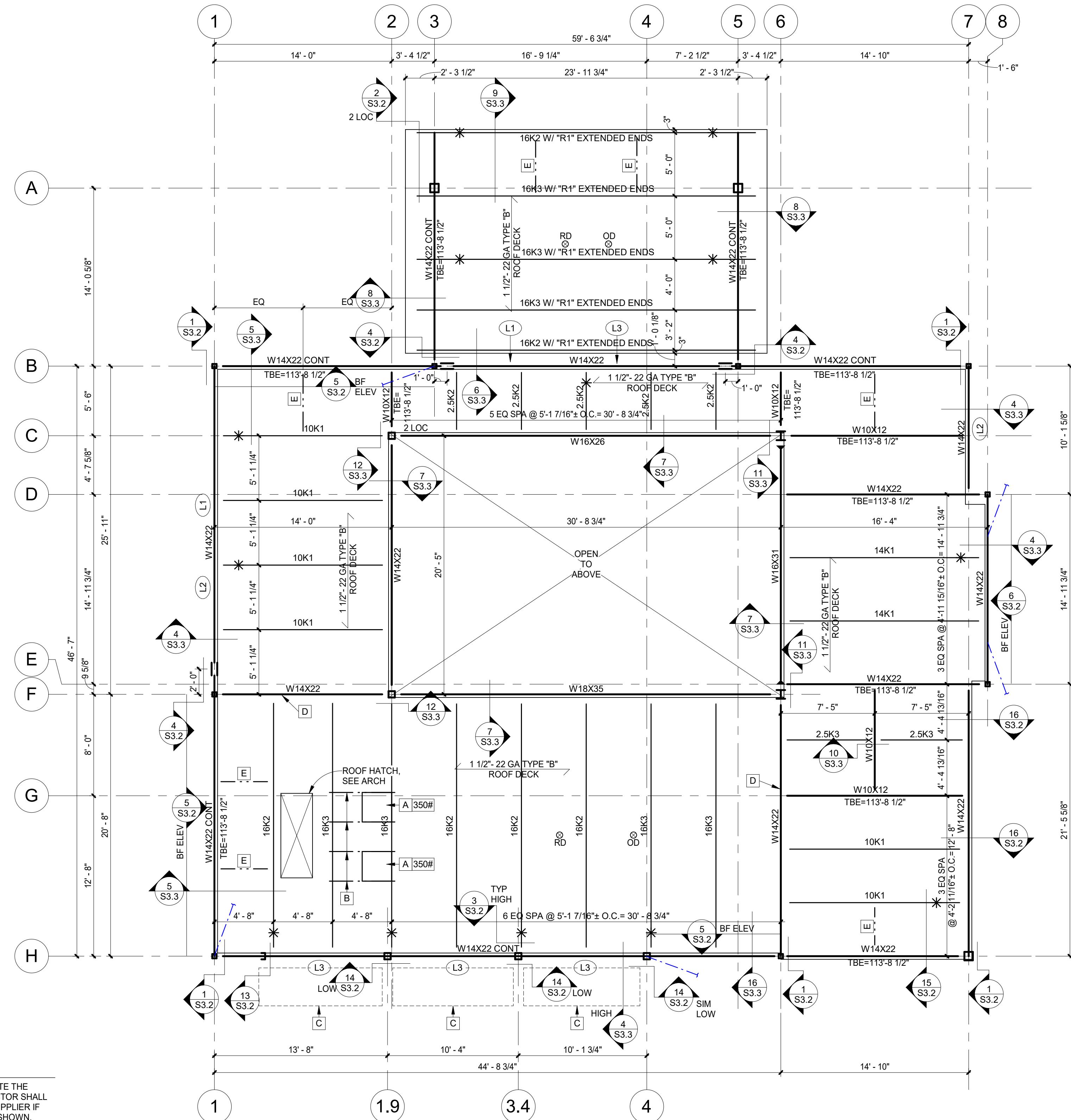


**ROOF FRAMING PLAN NOTES:**

- SEE FOUNDATION PLAN FOR COLUMN SCHEDULE.
- VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
- TOP OF BEAM ELEVATION (TBE) = 113'-6" UNO ON PLAN OR IN SECTIONS.
- SEE DETAIL 1/S3.3 FOR HANGER REQUIREMENTS AT STEEL JOISTS.
- SEE DETAIL 2/S3.3 FOR ROOF OPENING SUPPORT FRAME DETAIL AT ROOF OPENINGS (INCLUDING ROOF HATCHES, DRAINS AND MECHANICAL OPENINGS (MO)).
- SEE DETAIL 3/S3.3 FOR MECHANICAL UNIT SUPPORT DETAIL.
- ON PLAN DENOTES JOIST BOTTOM CHORD ANGLE EXTENSION TO BEAM BOTTOM FLANGE. SEE 10/S3.3. JOIST SUPPLIER SHALL CHECK JOIST BOTTOM CHORD FOR 2K AXIAL FORCE.
- DESIGN ROOF JOISTS FOR A NET SERVICE UPLIFT OF 10 PSF.
- (DF-) ON PLAN DENOTES ROOF DECK FASTENING. SEE ROOF DECK FASTENING SCHEDULE ON THIS SHEET.
- ROOF DECK FASTENING SHALL BE (DF1) UNO ON PLAN.
- (L-) ON PLAN DENOTES LINTEL. SEE LINTEL SCHEDULE ON THIS SHEET.

**ROOF FRAMING KEYNOTES:**

- A MECHANICAL UNIT WEIGHTS AND SIZES SHOWN ON PLAN INDICATE THE MAXIMUM WEIGHT AND SIZE PERMITTED. MECHANICAL CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD AND COORDINATE WITH JOIST SUPPLIER IF ANY UNIT WEIGHTS, SIZES OR LOCATIONS DIFFER FROM THOSE SHOWN.
- B MECHANICAL UNIT SUPPORT CHANNEL. SEE 3/S3.3.
- C SUNSHADE AND CONNECTION TO STRUCTURAL STEEL FRAMING BY OTHERS, SEE ARCH.
- D AT DECK DIRECTION TRANSITION, PROVIDE CONTINUOUS WT3X4.5, FIELD WELD TO ROOF JOISTS WITH 3/16" FILLET WELDS EACH SIDE. PROVIDE HSS2 1/2X2 1/2X3/16 X 5" SHIMS AS REQUIRED TO SUPPORT ENDS OF WT3X4.5.
- E ANGLE KICKERS, SEE SECTIONS.



PLAN TRUE NORTH NORTH  
 1 ROOF FRAMING PLAN  
 S2.1 3/16" = 1'-0"

MARK	REFERENCE	MEMBER	REMARKS
L1	--	L4X4X3/8	
L2	--	L7X4X3/8 LLV	
L3	11/S3.2	L7X4X3/8 LLV + HSS8X2X3/16 LSV	

**LINTEL SCHEDULE NOTES:**

- VERIFY ALL SPANS OF OPENINGS AND WALL THICKNESSES WITH ARCH DRAWINGS. NOTIFY ARCHITECT/ENGINEER OF DEVIATIONS PRIOR TO FABRICATION AND ERECTION.
- LOOSE BRICK LINTELS SHALL BE HOT DIP GALVANIZED AND SHALL BEAR 6" MINIMUM AT EACH END OF OPENING.

MARK	ROOF DECK FASTENING SCHEDULE			
	FASTENING OPTION	TYPICAL FASTENING	PERIMETER FASTENING	SIDLAP CONNECTION
(DF1)	WELDED OPTION	5/8"Ø PUDDLE WELDS W/ 3/64 PATTERN	5/8"Ø PUDDLE WELDS @ 6" O.C.	(1) #10 TEK SCREWS AT MID-SPAN BTWN SUPPORTS
	MECHANICAL FASTENER OPTION	HILTI PAF FASTENERS W/ 3/64 PATTERN (SEE NOTE #1)	HILTI PAF FASTENERS @ 6" O.C. (SEE NOTE #1)	(1) HILTI SLC SCREW AT MID-SPAN BTWN SUPPORTS (SEE NOTE #2)

**ROOF DECK FASTENING SCHEDULE NOTES:**

- HILTI PAF FASTENERS SHALL BE HILTI X-HSN 24. IF THE BASE STEEL THICKNESS IS GREATER THAN 3/8", USE ENP-19 FASTENERS IN LIEU OF X-HSN 24.
- HILTI SLC SCREWS SHALL BE S-SLC 01 M HWH OR S-SLC 02 M HWH SCREWS.

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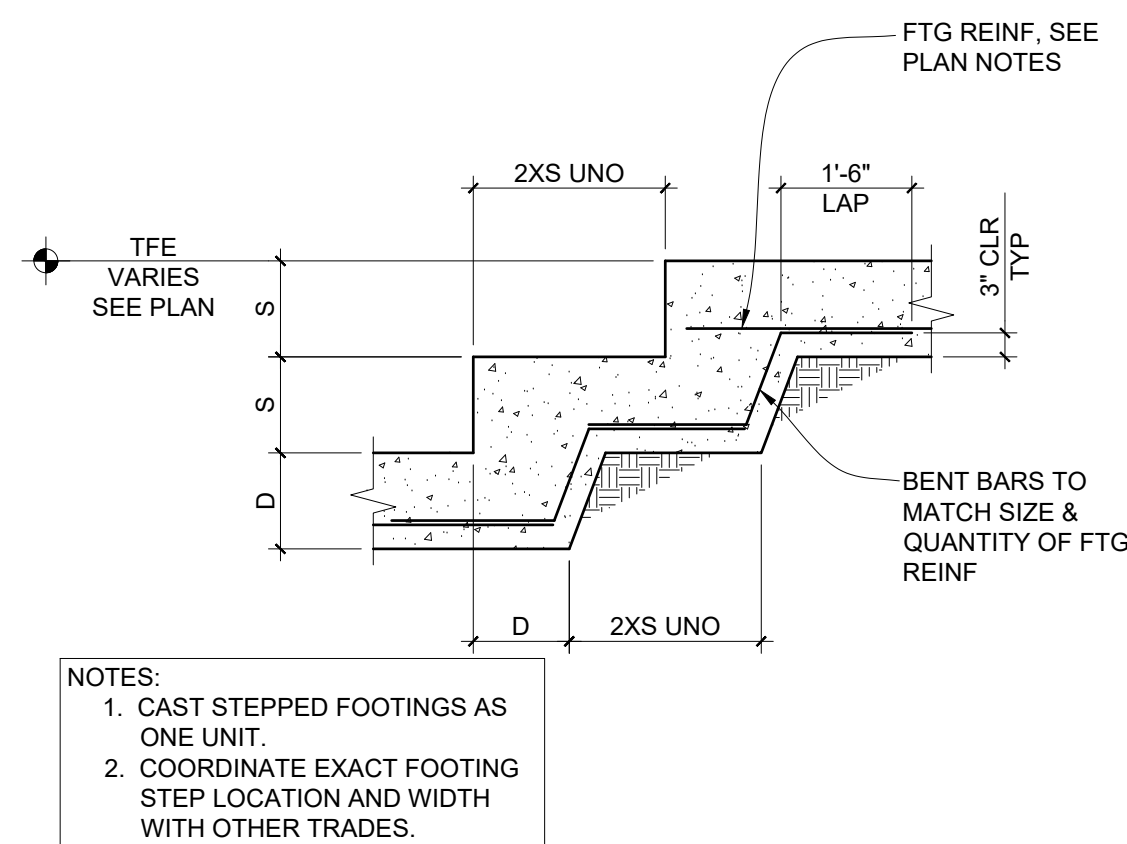
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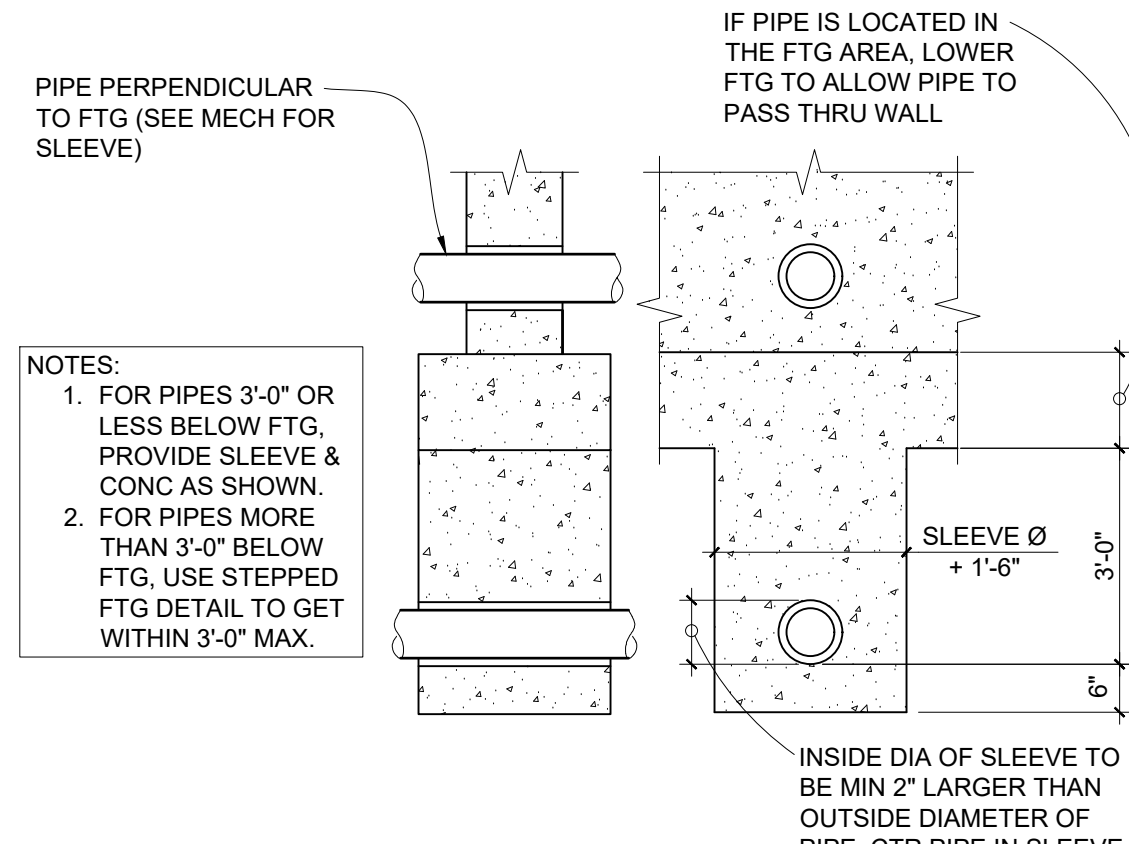
## SECTIONS AND DETAILS

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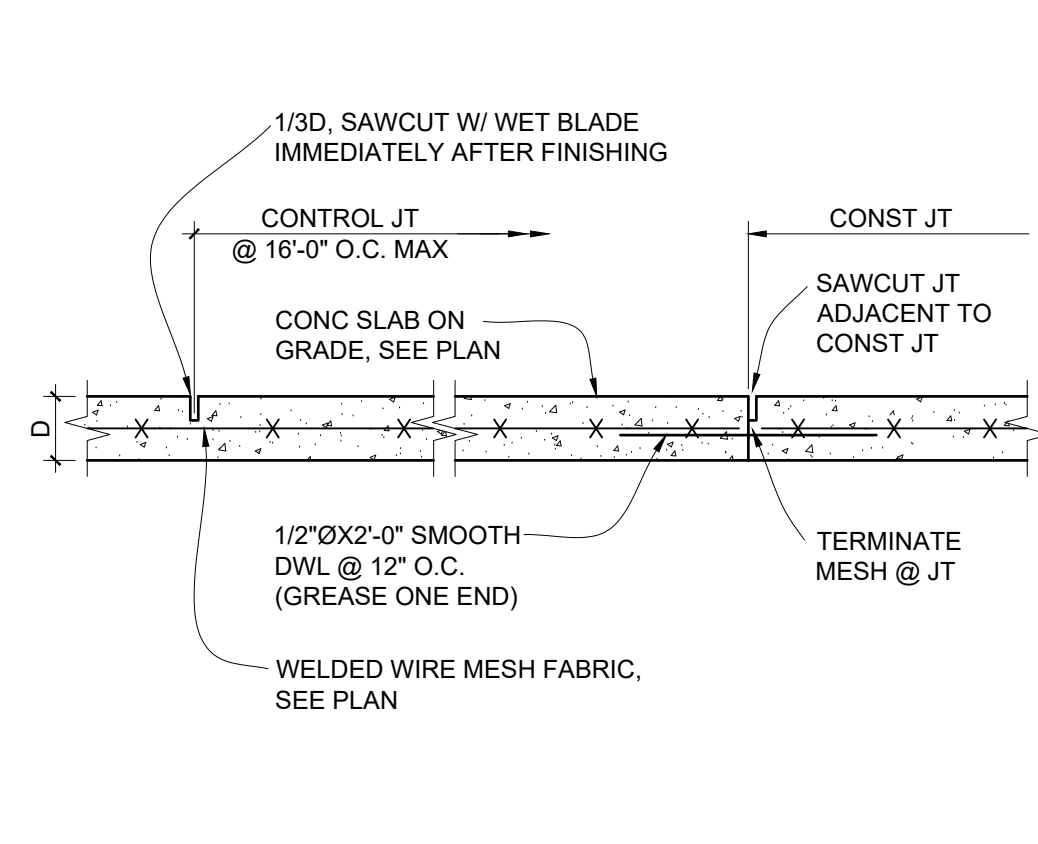
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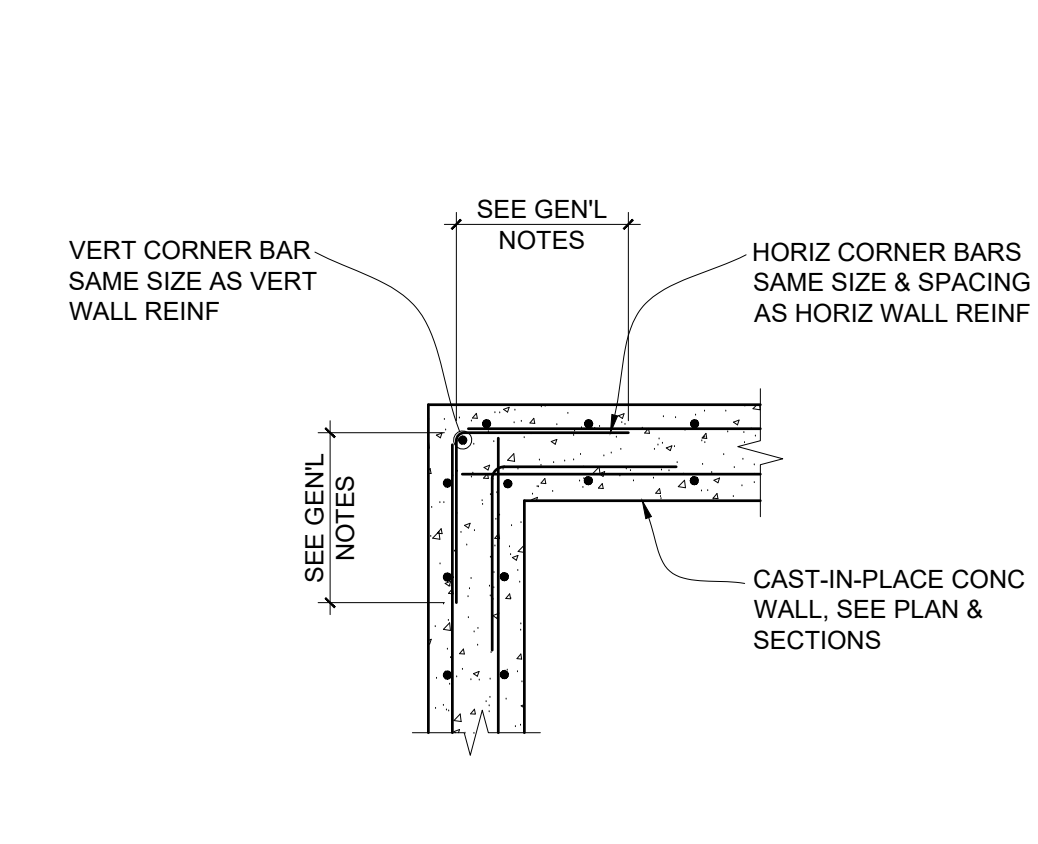
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S3.1 STEPPED FTG NO SCALE



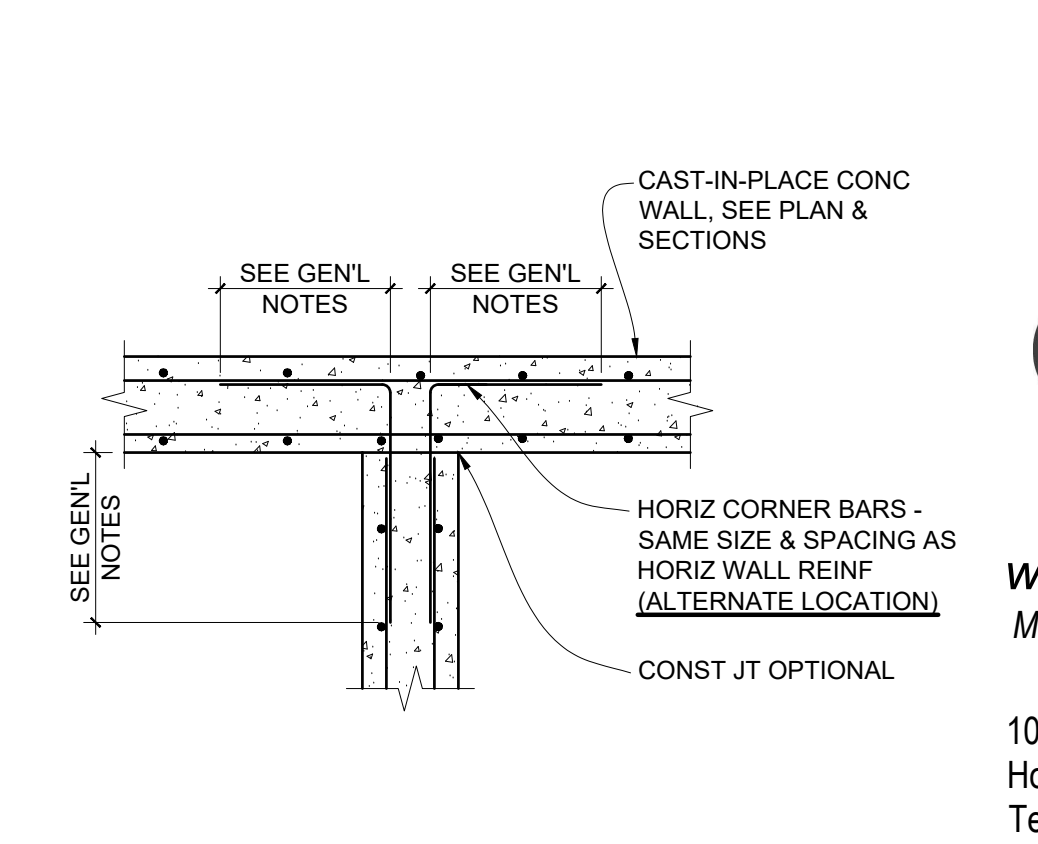
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S3.1 WALL FTG @ UNDERGROUND LINES NO SCALE



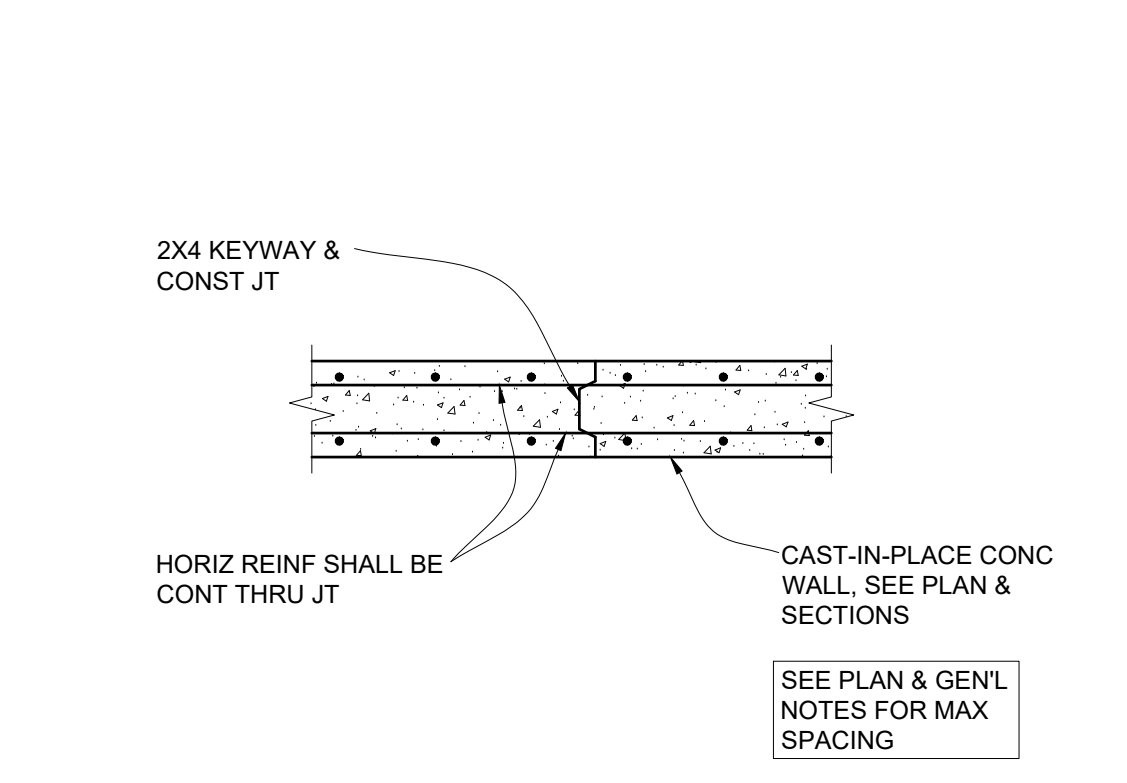
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S3.1 TYP CONTROL/CONST JT NO SCALE



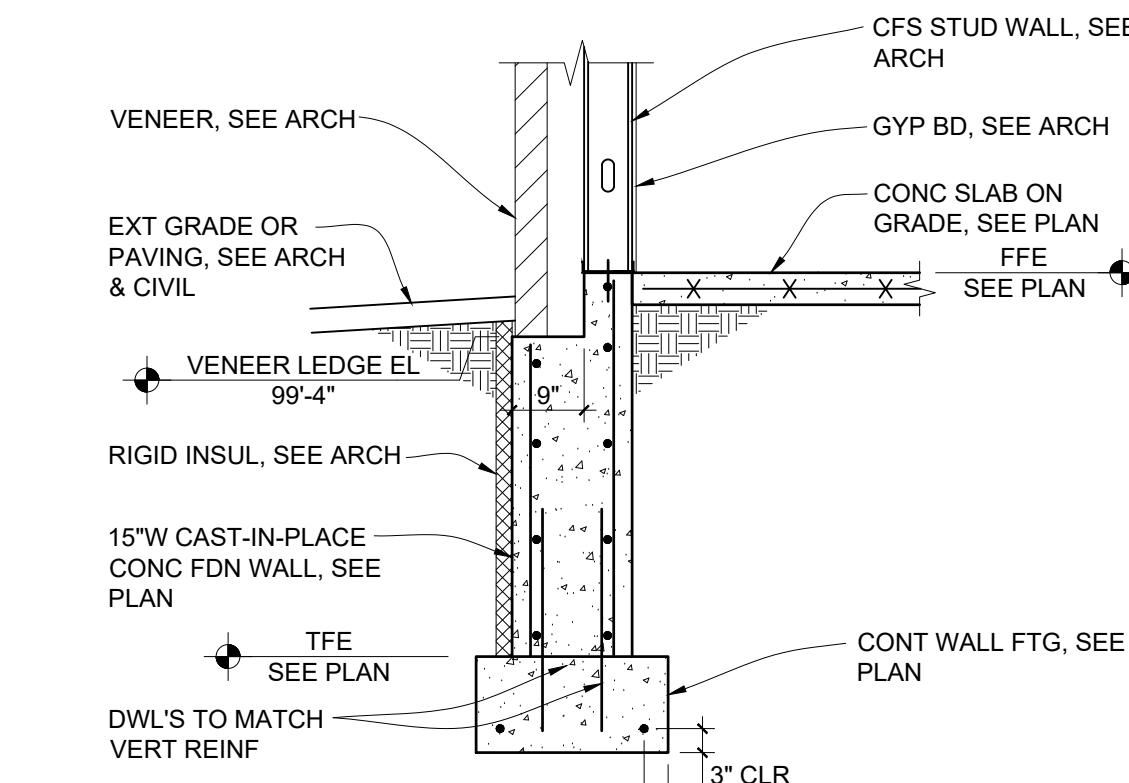
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S3.1 TYP WALL CORNER 1/2\"/>



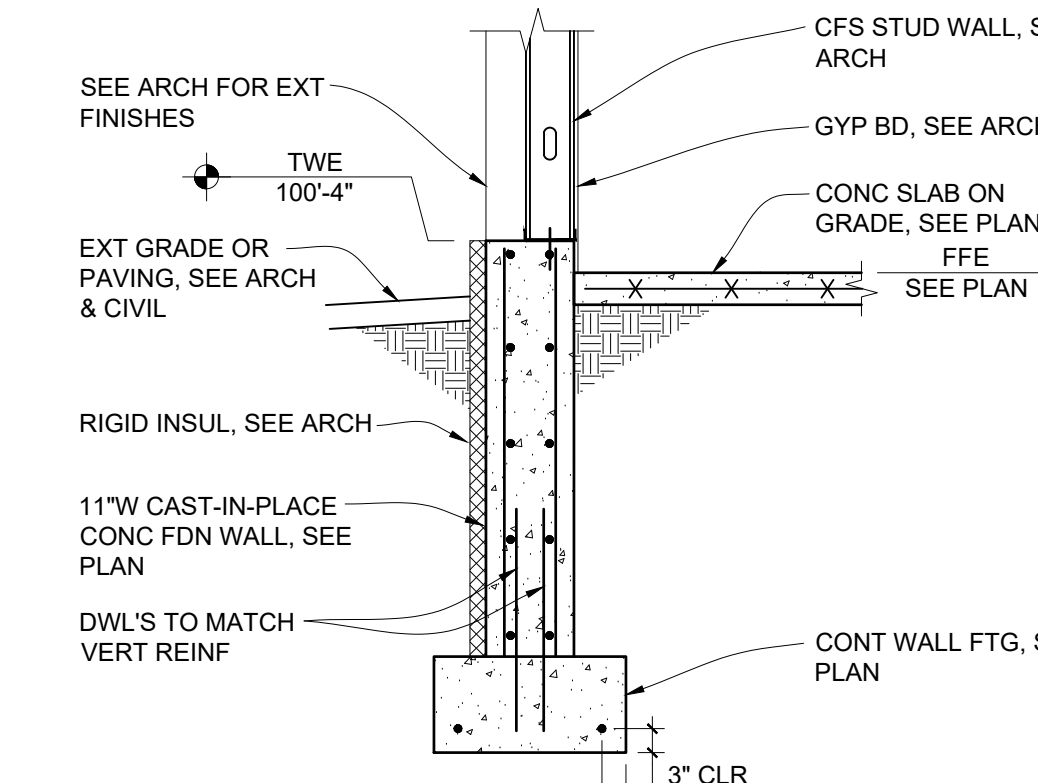
**5 PLAN DETAIL**  
S3.1 TYP WALL INTERSECTION 1/2\"/>



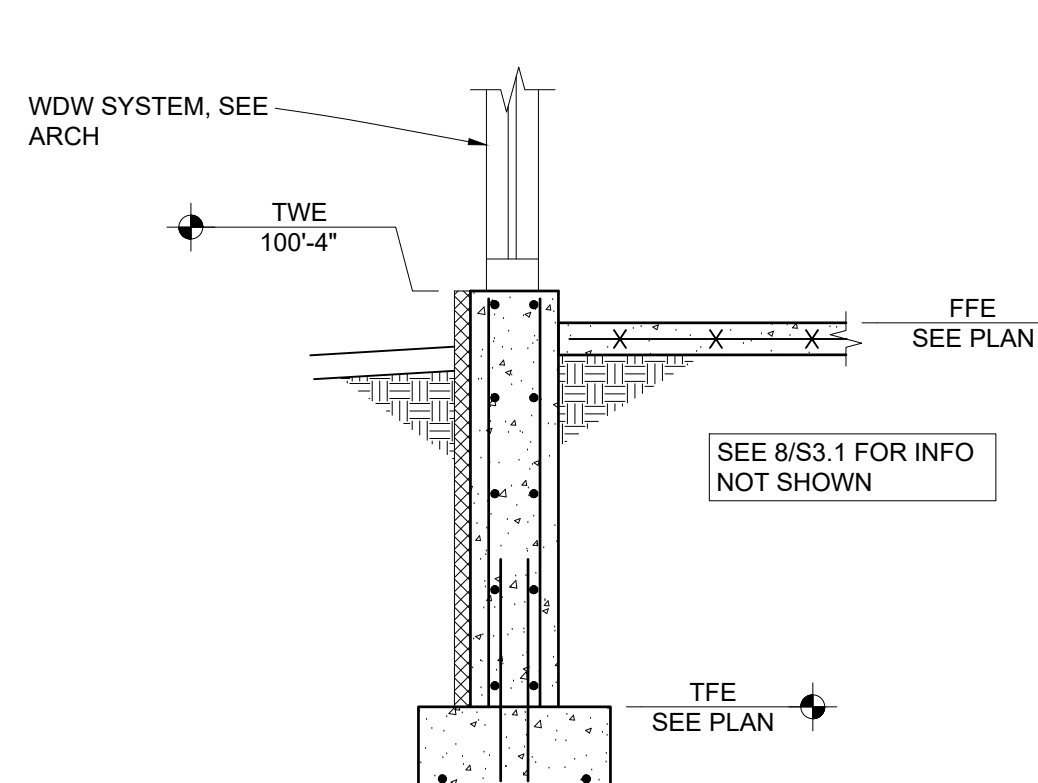
**6 PLAN DETAIL**  
S3.1 WALL CONSTRUCTION JOINT 1/2\"/>



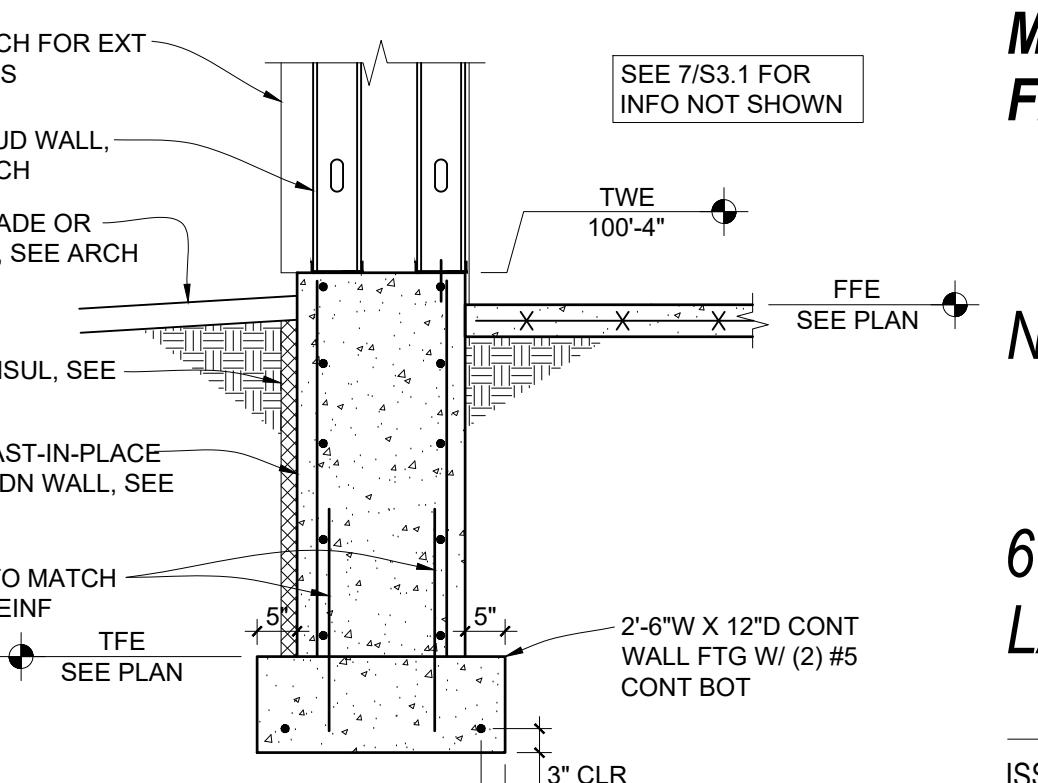
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S3.1 TYP FDN WALL @ VENEER 1/2\"/>



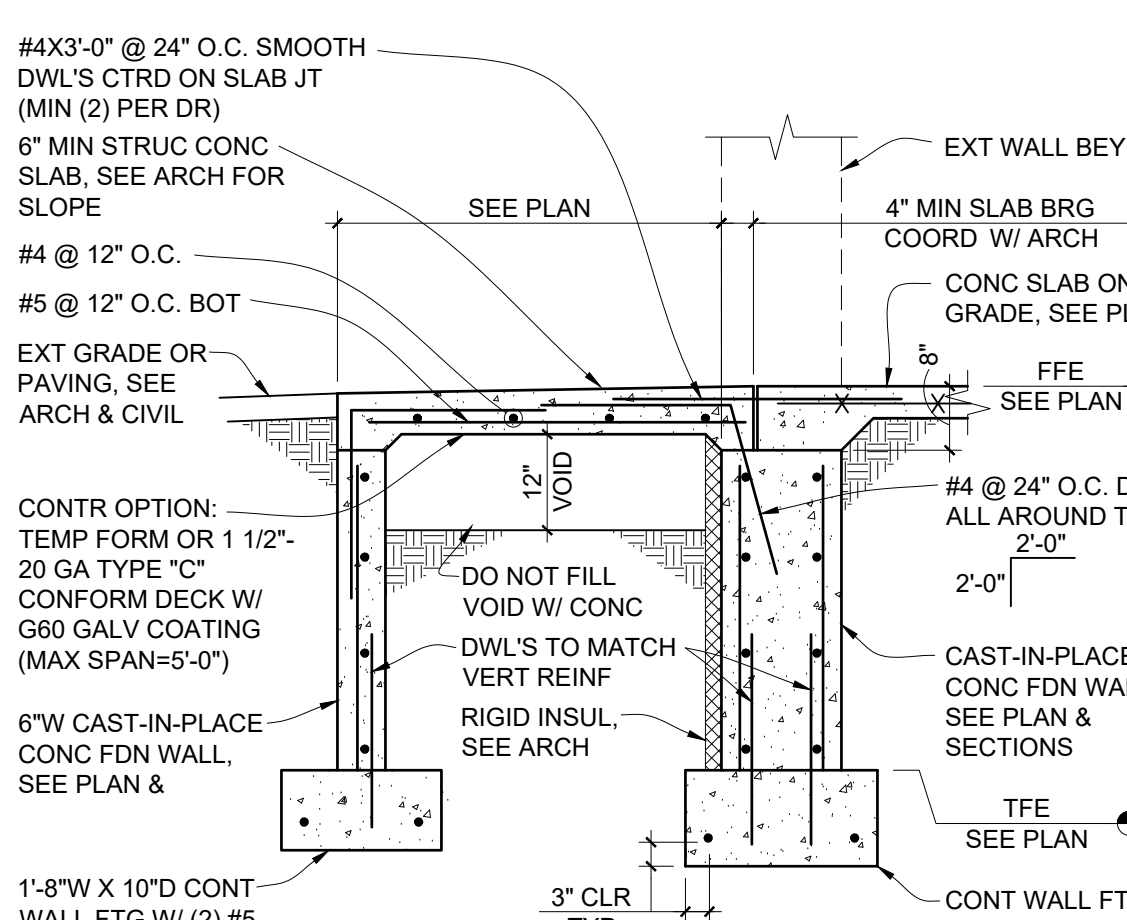
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S3.1 TYP FDN WALL @ METAL PANEL 1/2\"/>



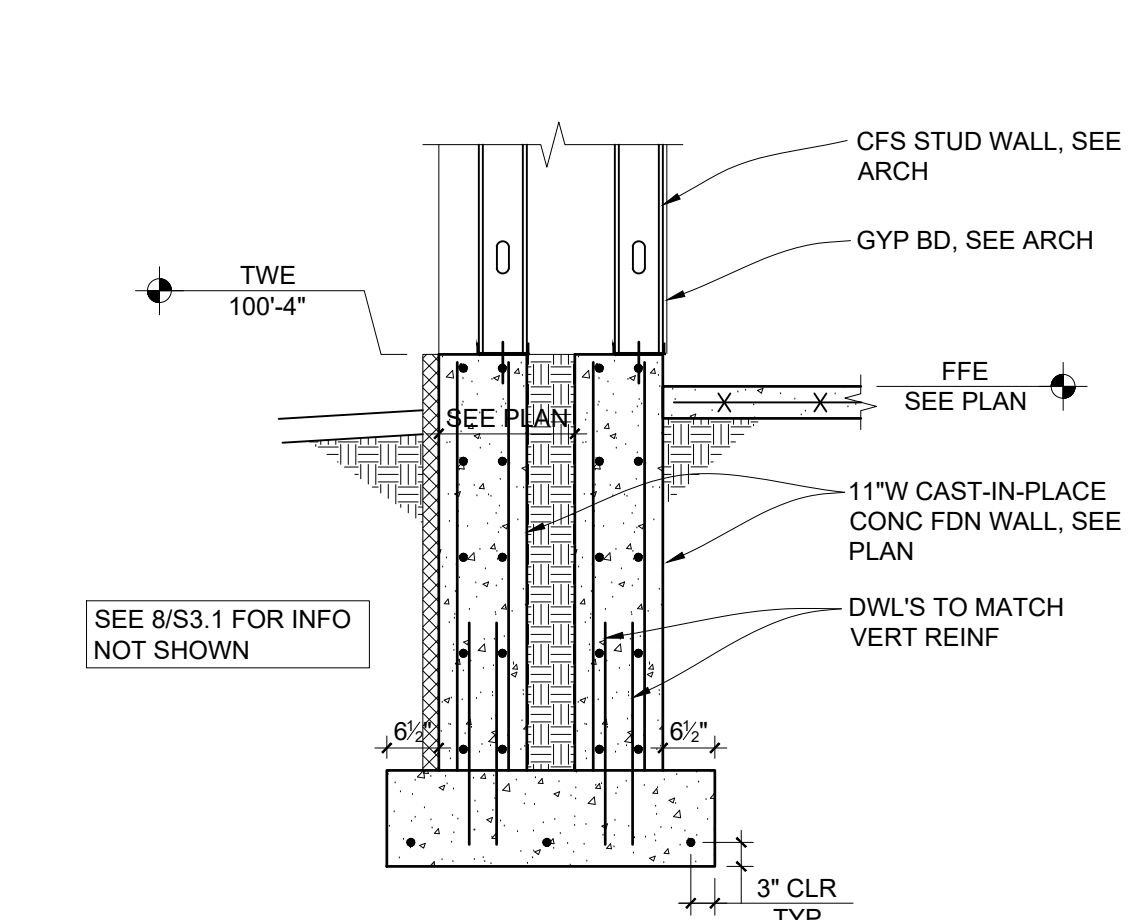
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S3.1 FDN WALL @ WDW SYSTEM 1/2\"/>



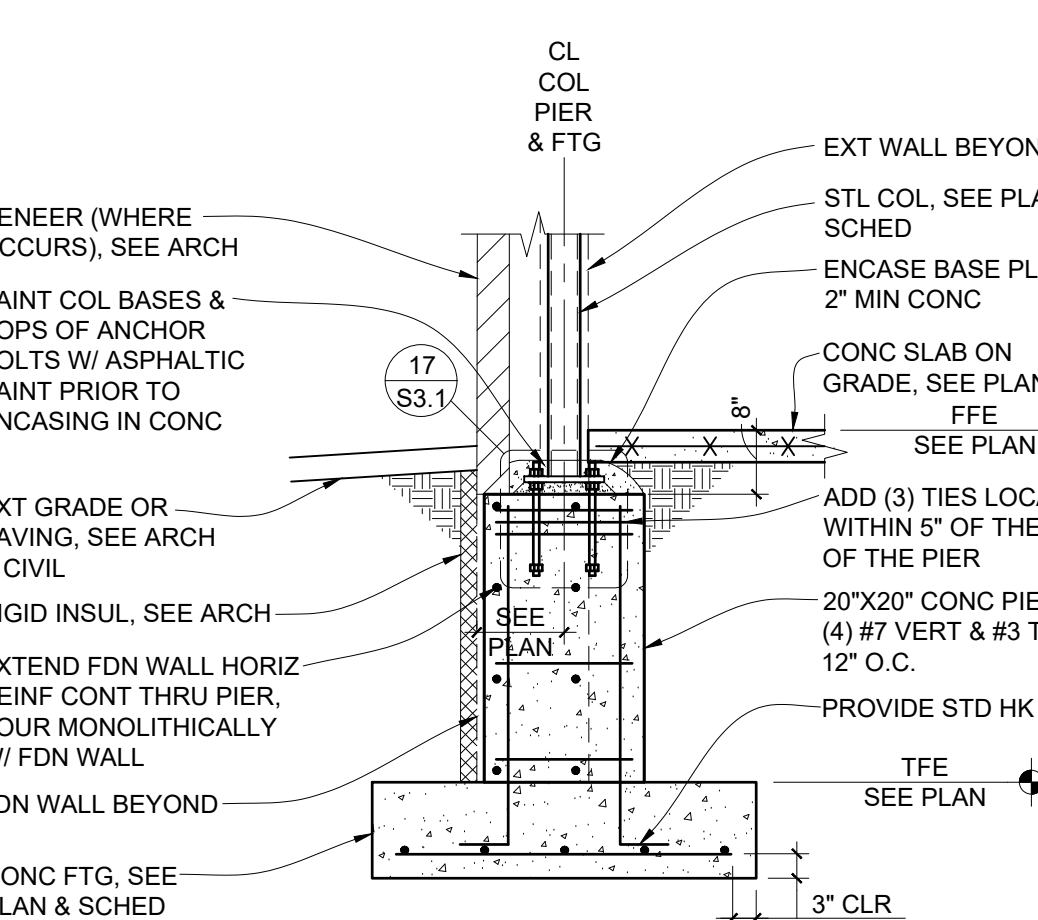
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S3.1 FDN WALL @ DBL WALL 1/2\"/>



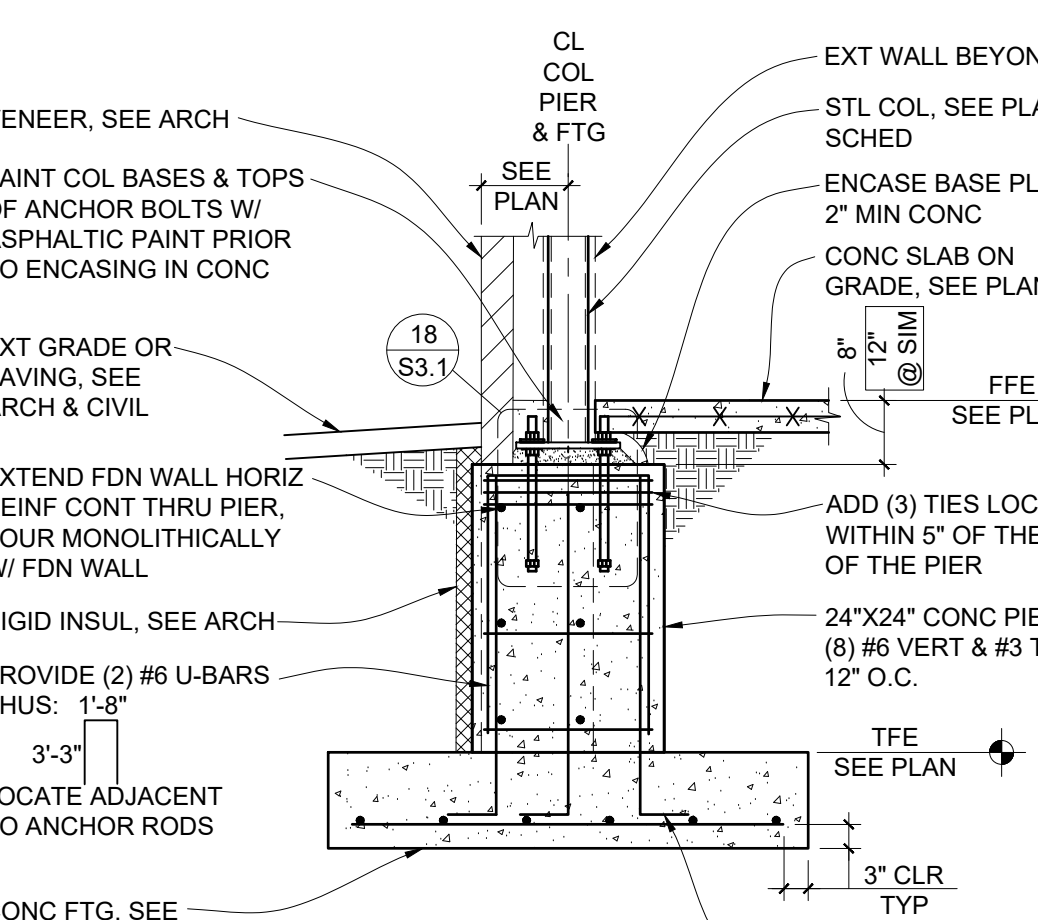
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S3.1 TYP EXT STRUC CONC SLAB 1/2\"/>



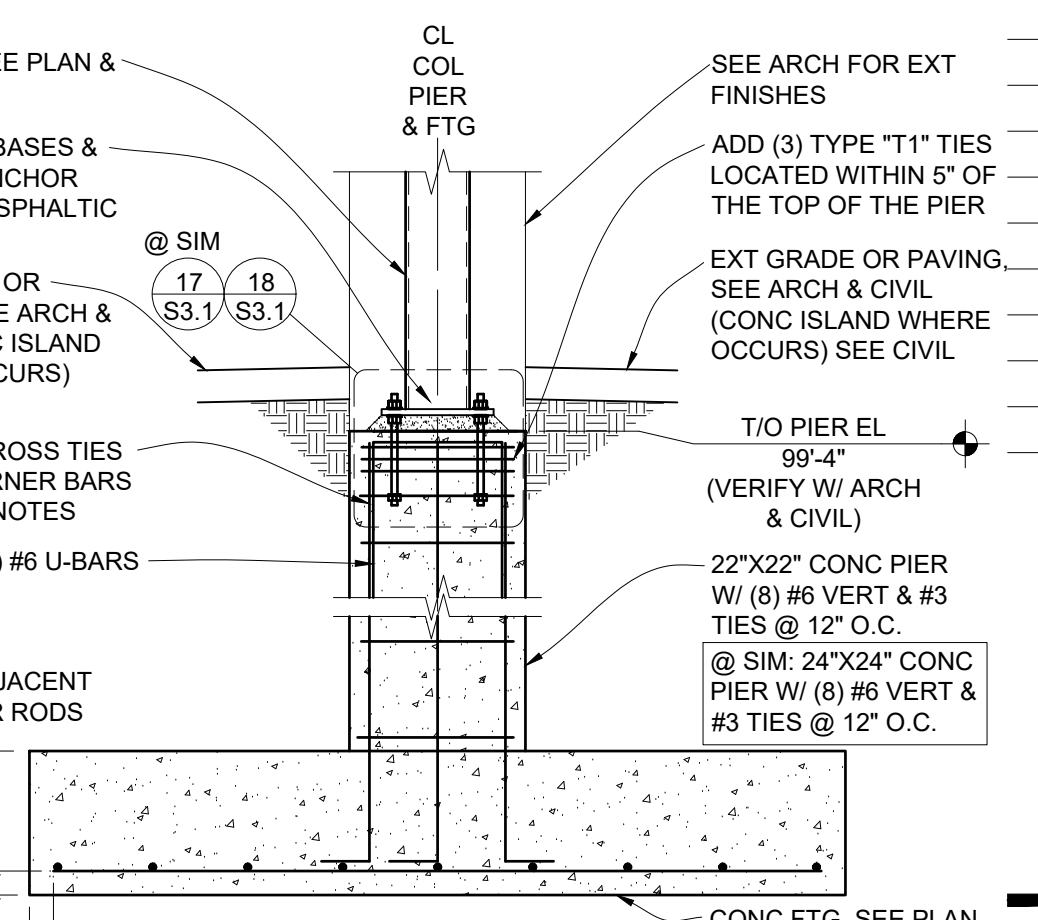
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S3.1 TYP FDN WALL @ SIGN WALL 1/2\"/>



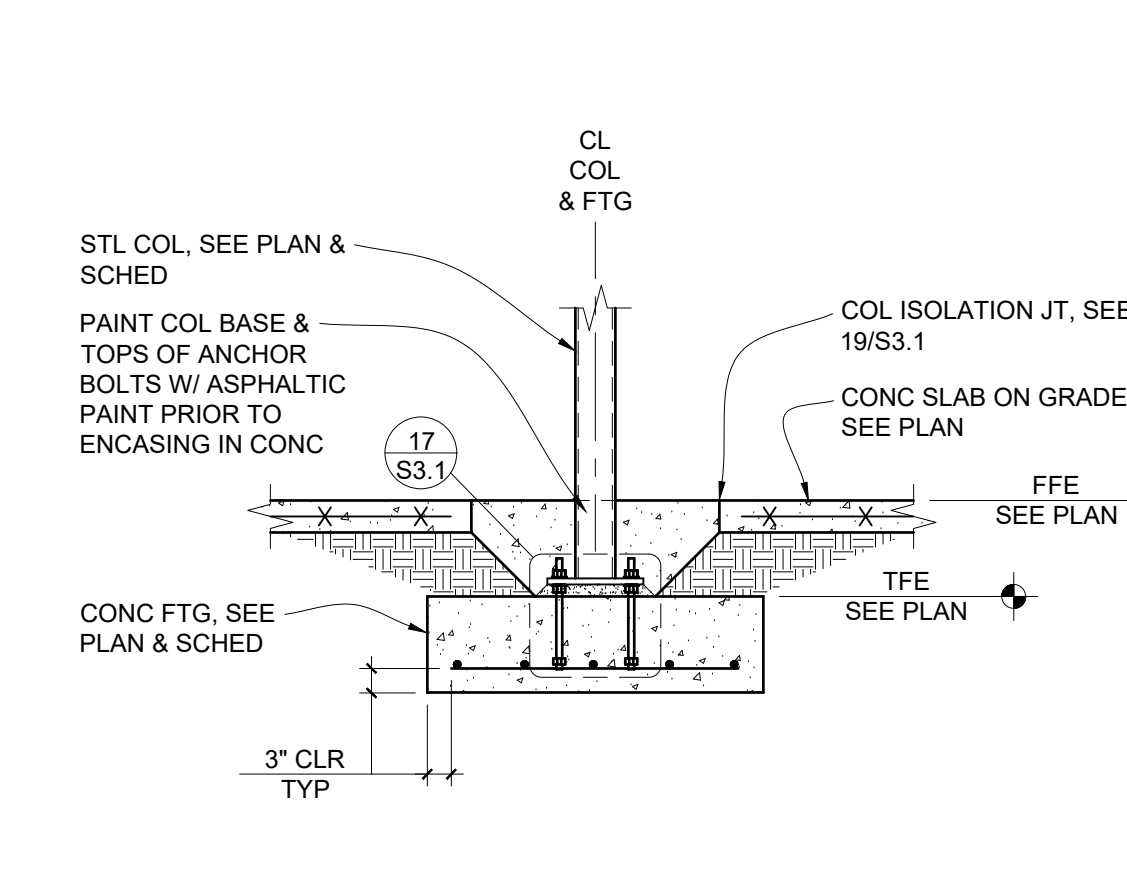
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S3.1 EXT STL COL, PIER & FTG 1/2\"/>



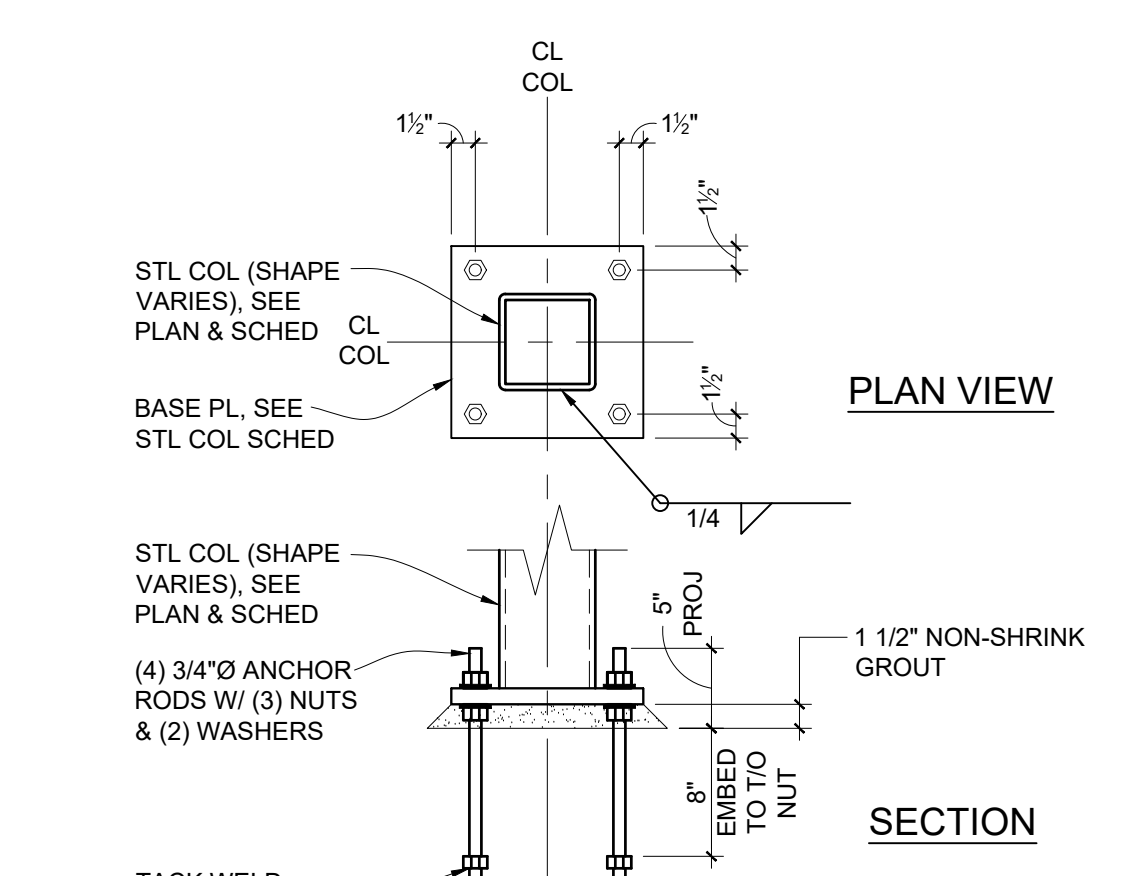
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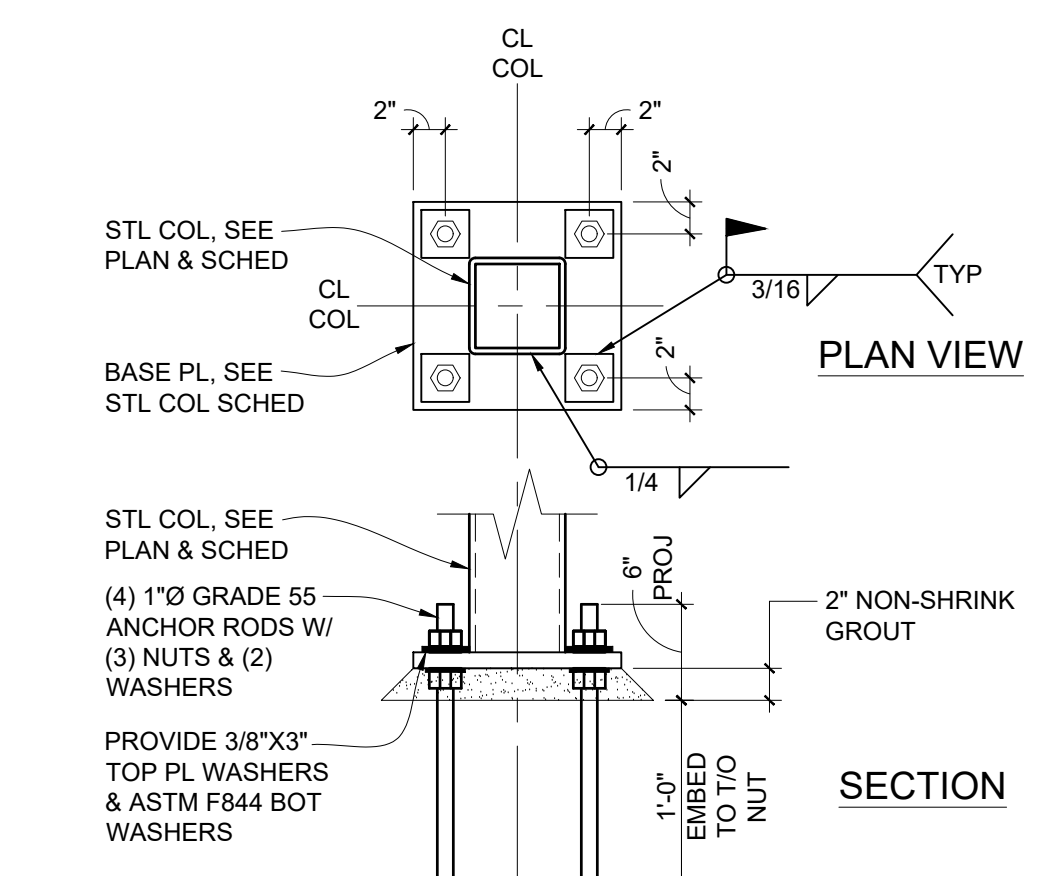
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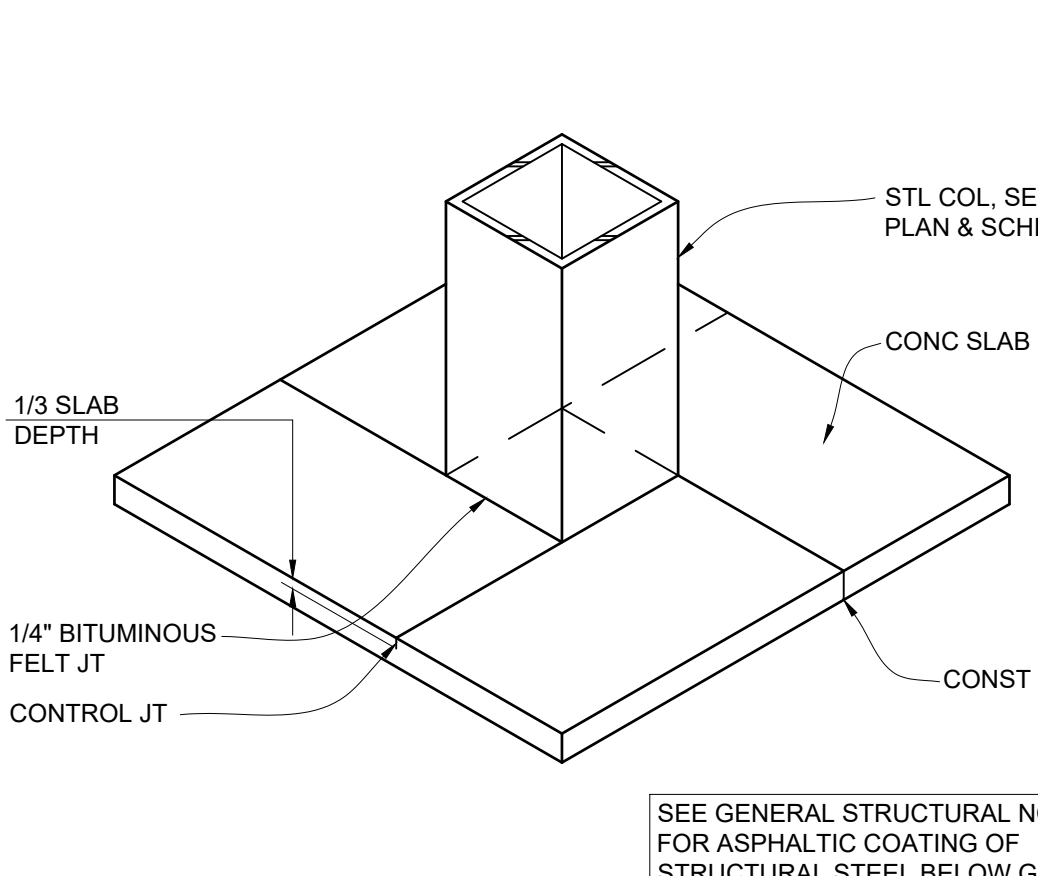
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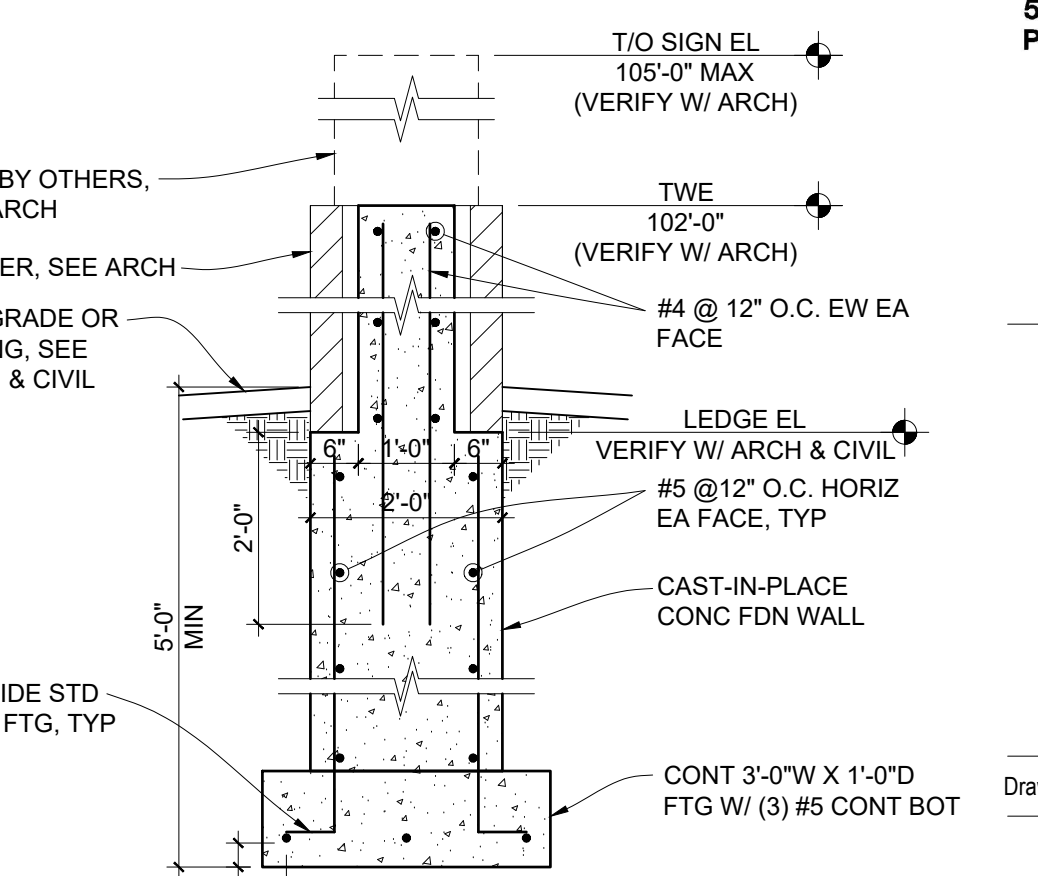
**17 DETAIL**  
S3.1 ANCHOR ROD SETTING 1\"/>



**18 DETAIL**  
S3.1 ANCHOR ROD SETTING 1\"/>



**19 DETAIL**  
S3.1 TYP COL ISOLATION JT NO SCALE



**20 SECTION**  
S3.1 MONUMENT SIGN FDN 1/2\"/>

## PROJECT

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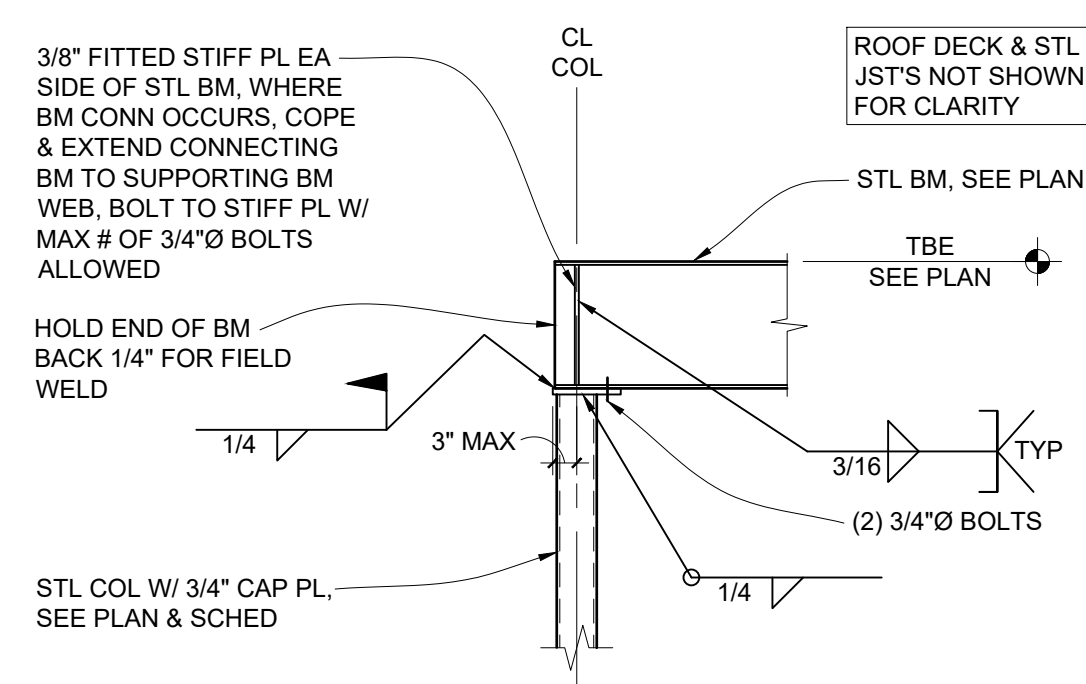
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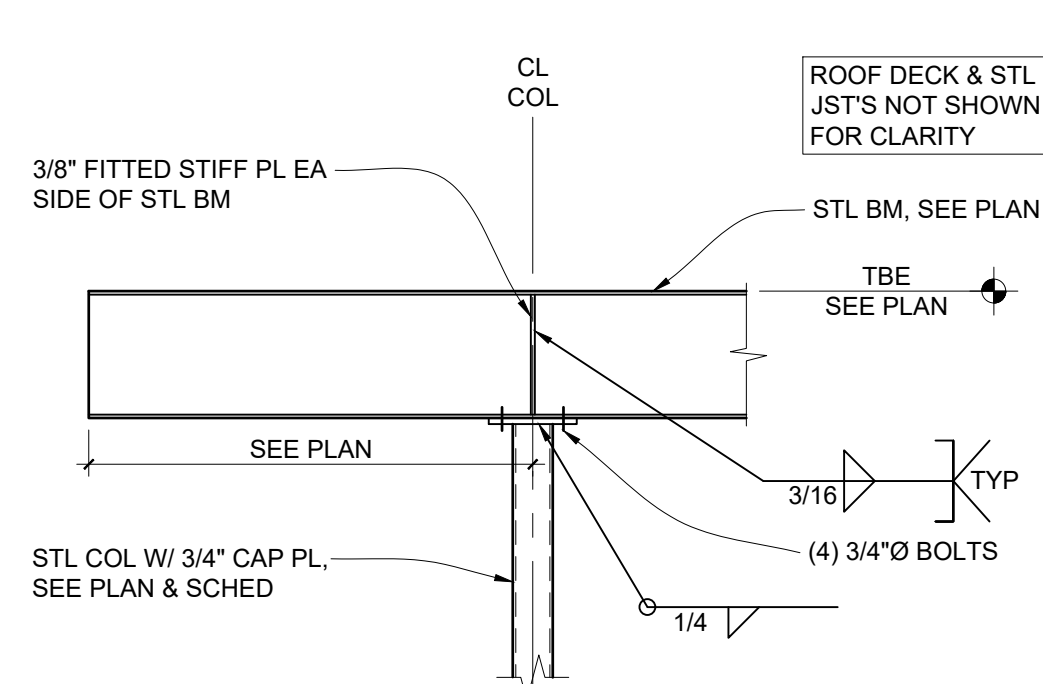
## SECTIONS AND DETAILS

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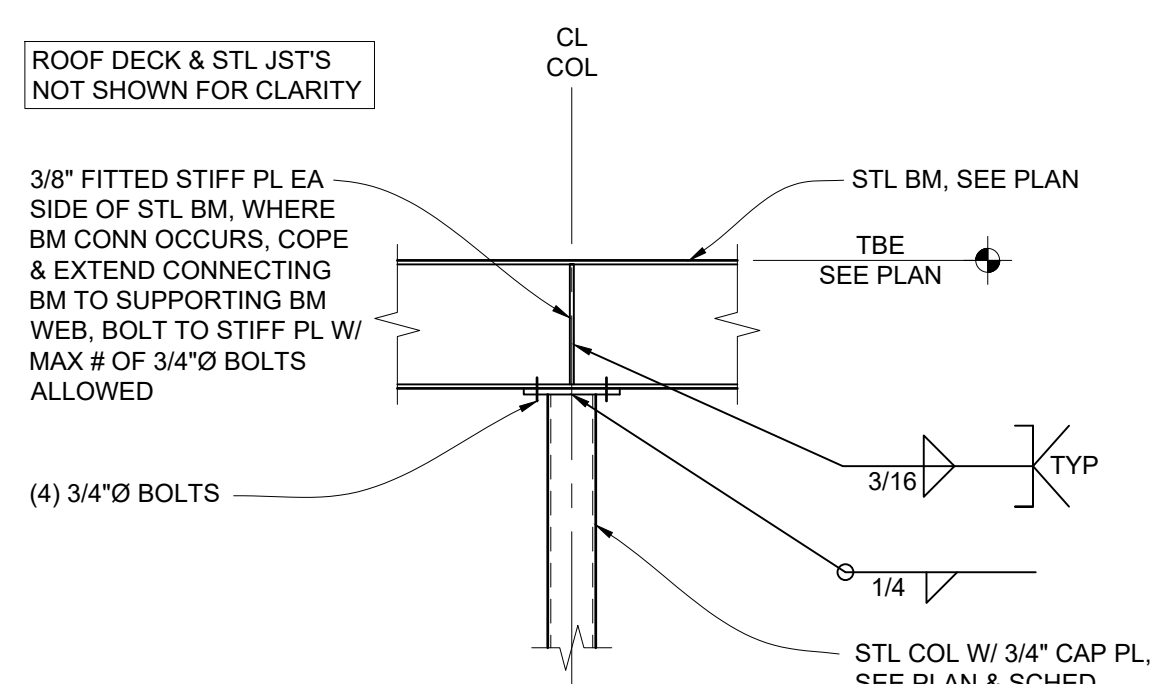
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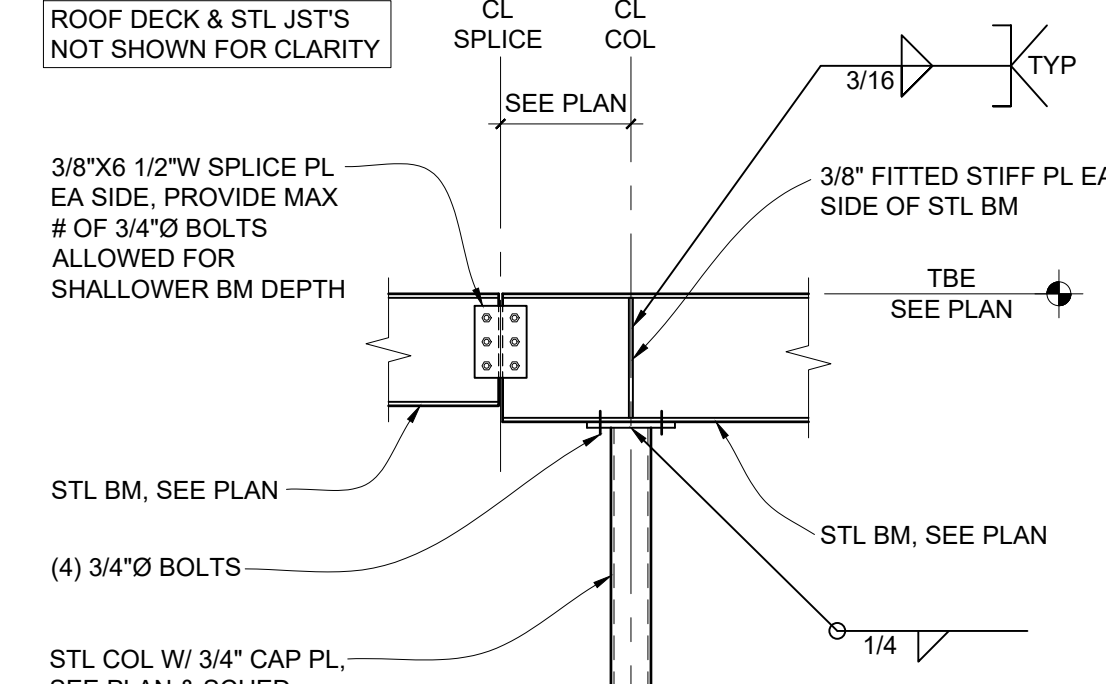
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S3.2 STL BM TO COL CONN 1/2"=1'-0"



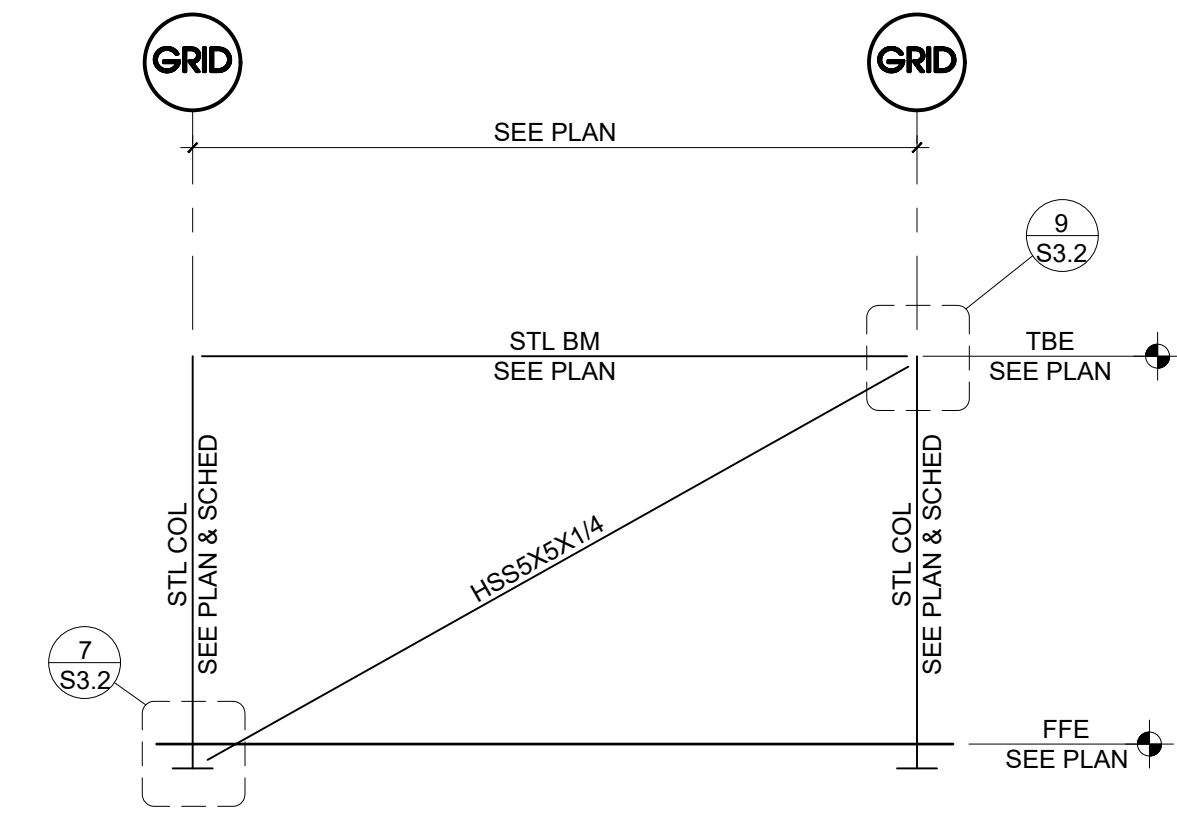
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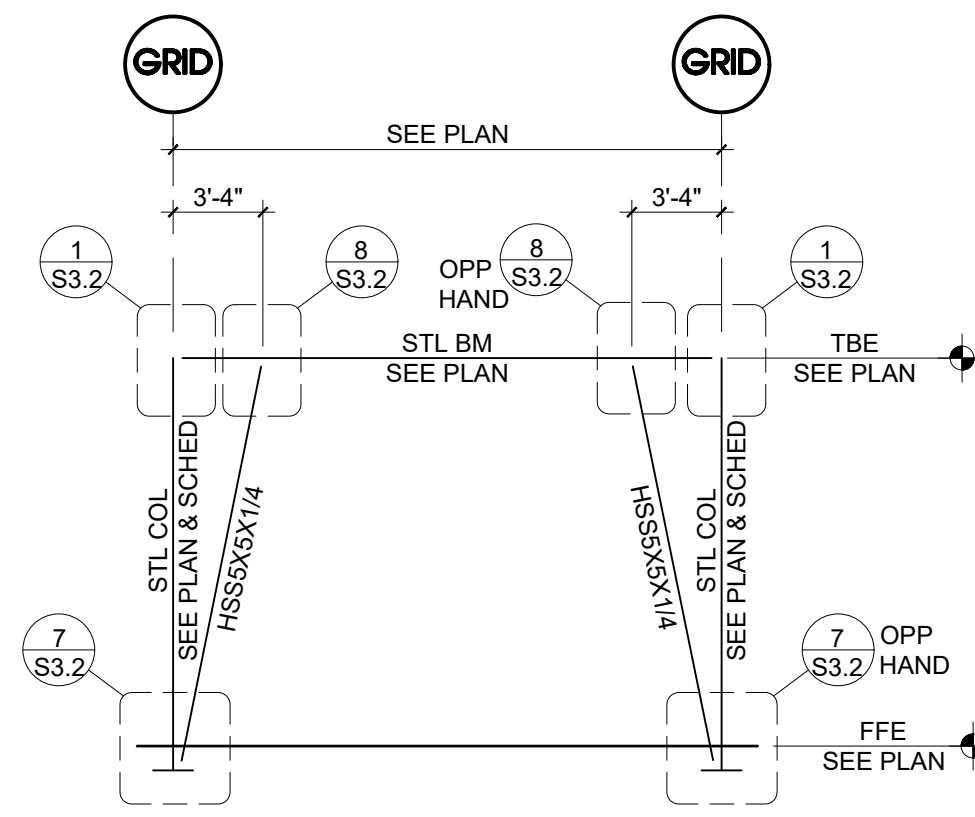
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S3.2 STL BM TO COL CONN 1/2"=1'-0"



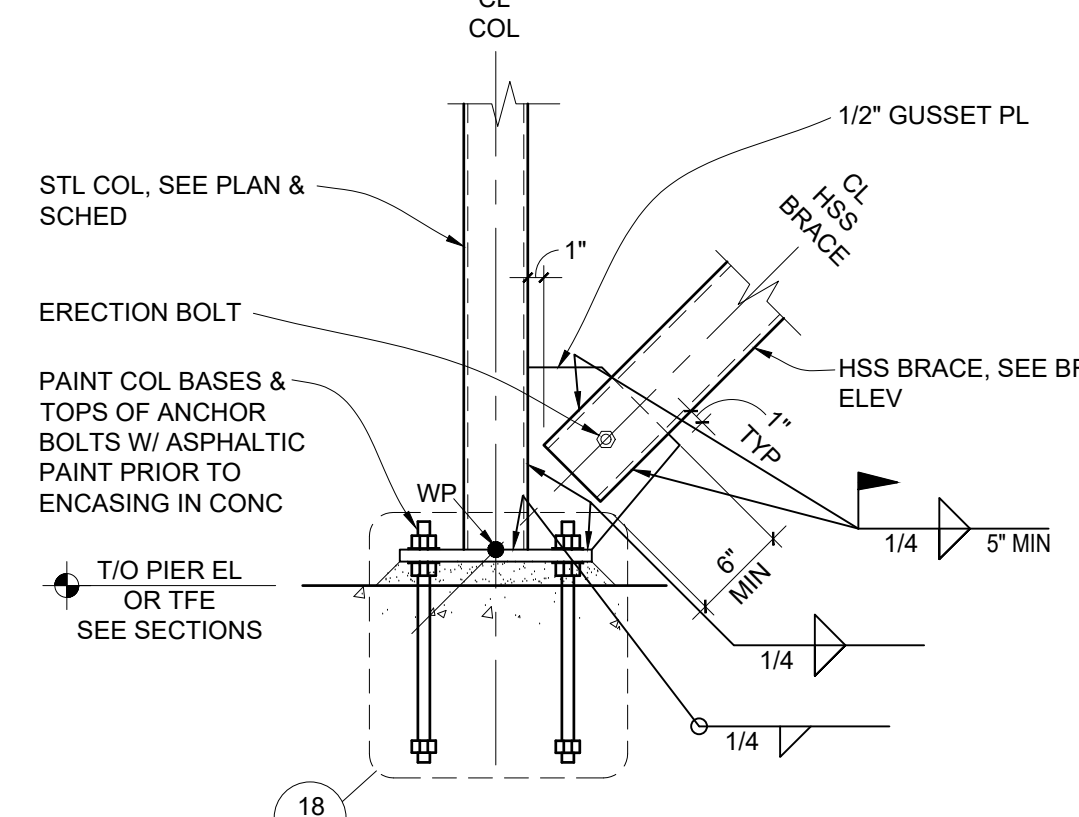
**4 SECTION**  
S3.2 STL BM TO COL CONN & BM SPLICE 1/2"=1'-0"



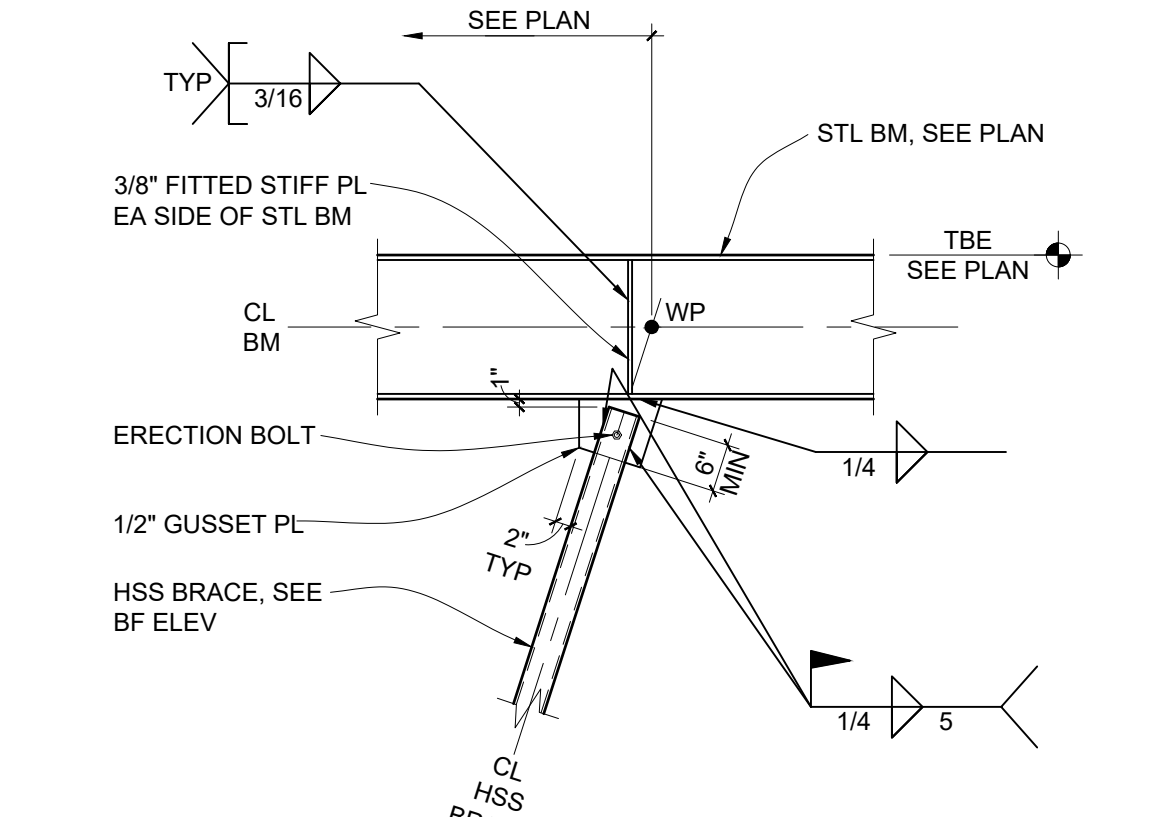
**5 ELEVATION**  
S3.2 BRACED FRAME NO SCALE



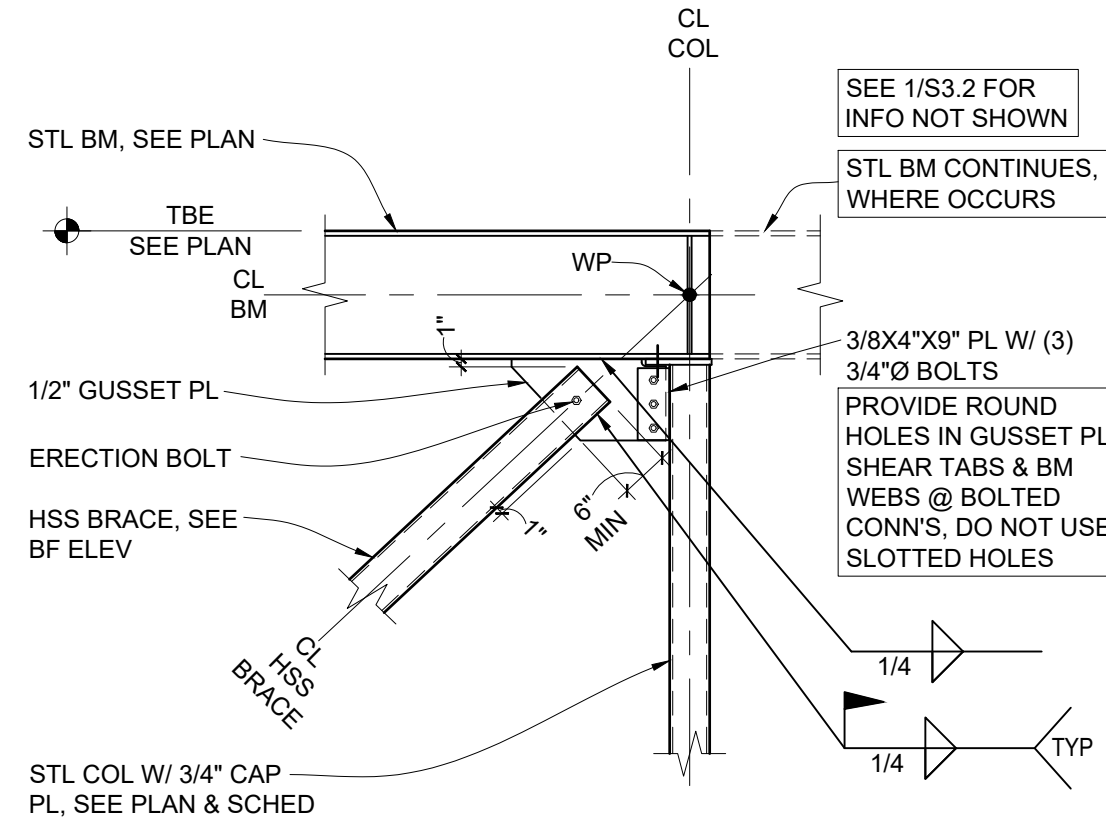
**6 ELEVATION**  
S3.2 BRACED FRAME NO SCALE



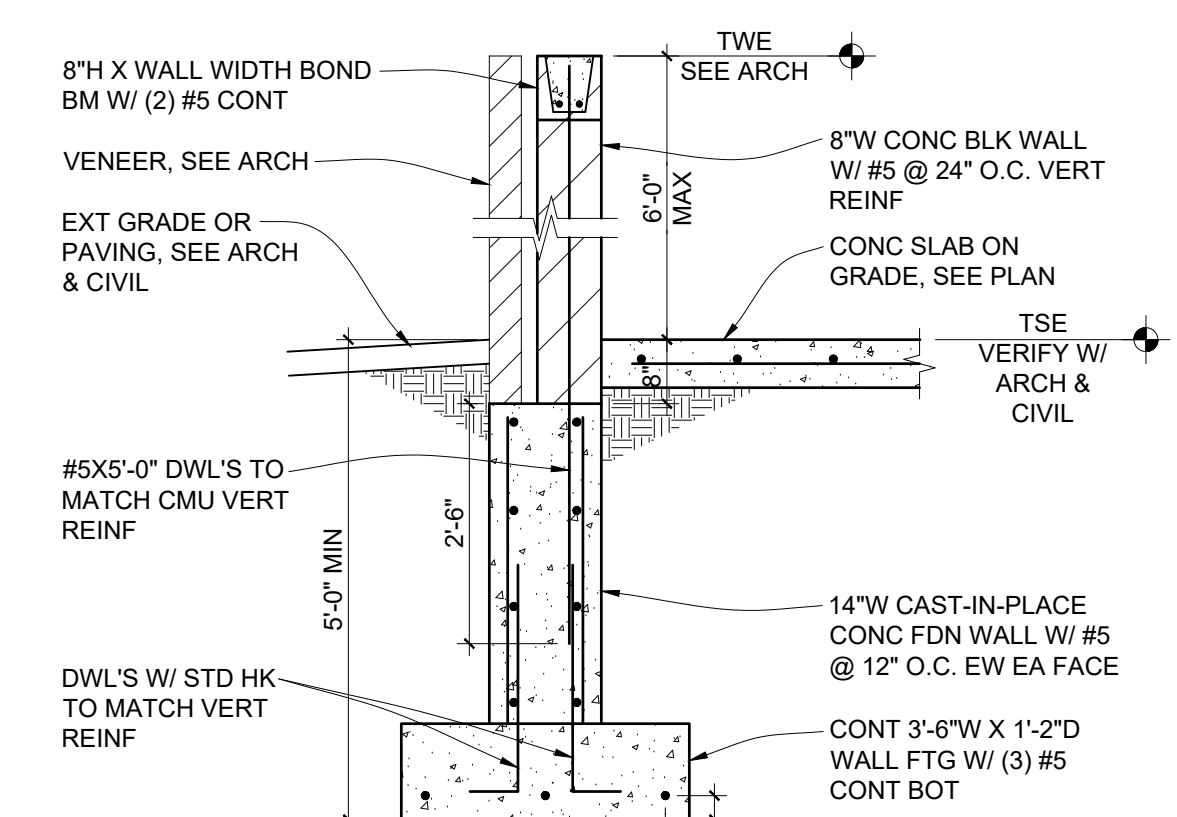
**7 DETAIL**  
S3.2 BRACED FRAME CONN @ BASE 1"=1'-0"



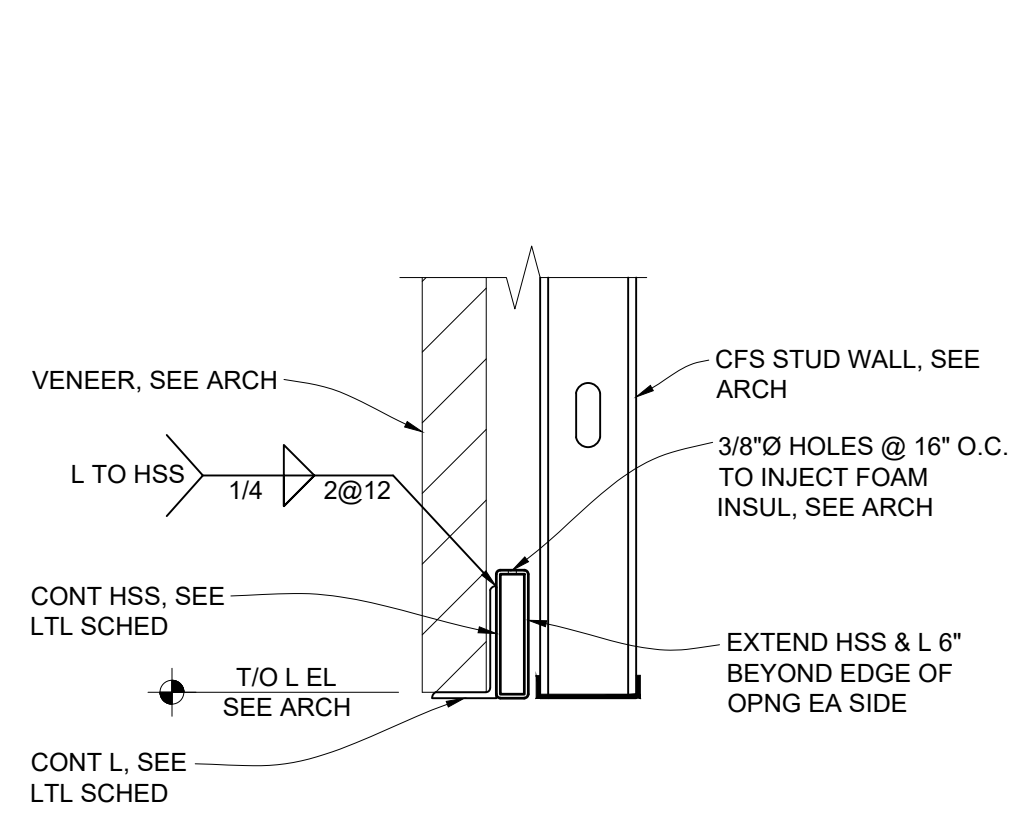
**8 DETAIL**  
S3.2 BRACED FRAME CONN 1/2"=1'-0"



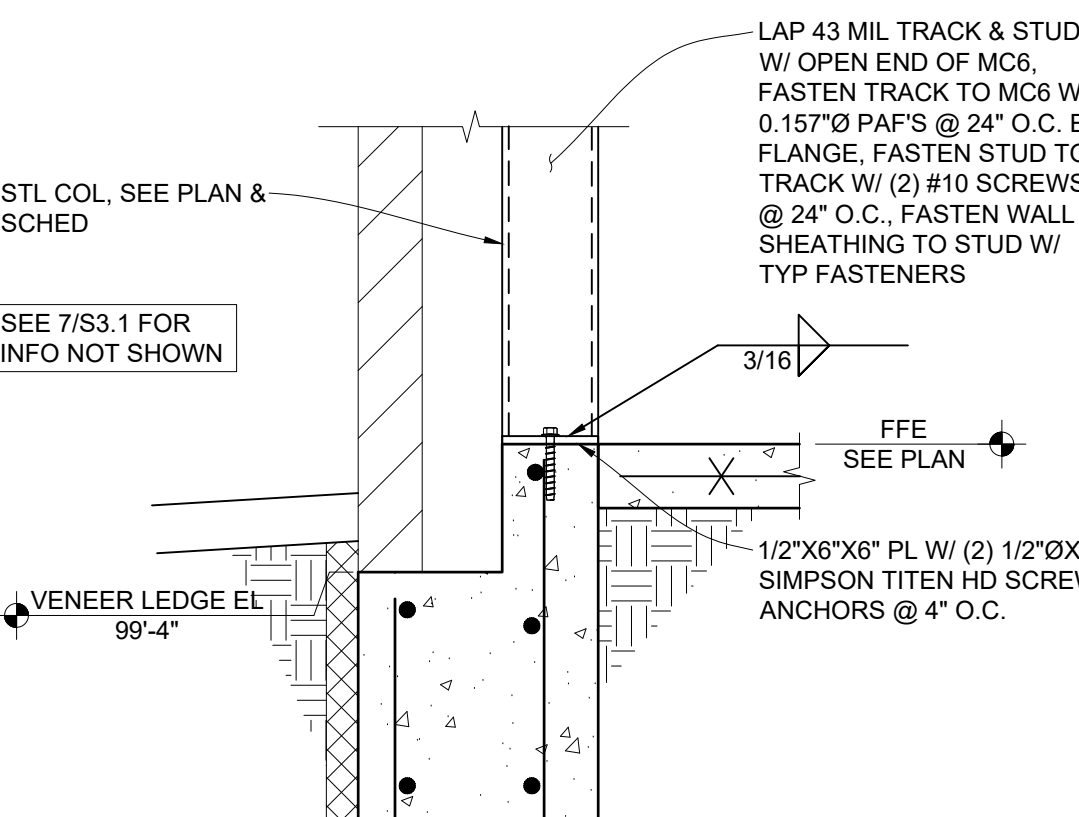
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S3.2 BRACED FRAME CONN 1/2"=1'-0"



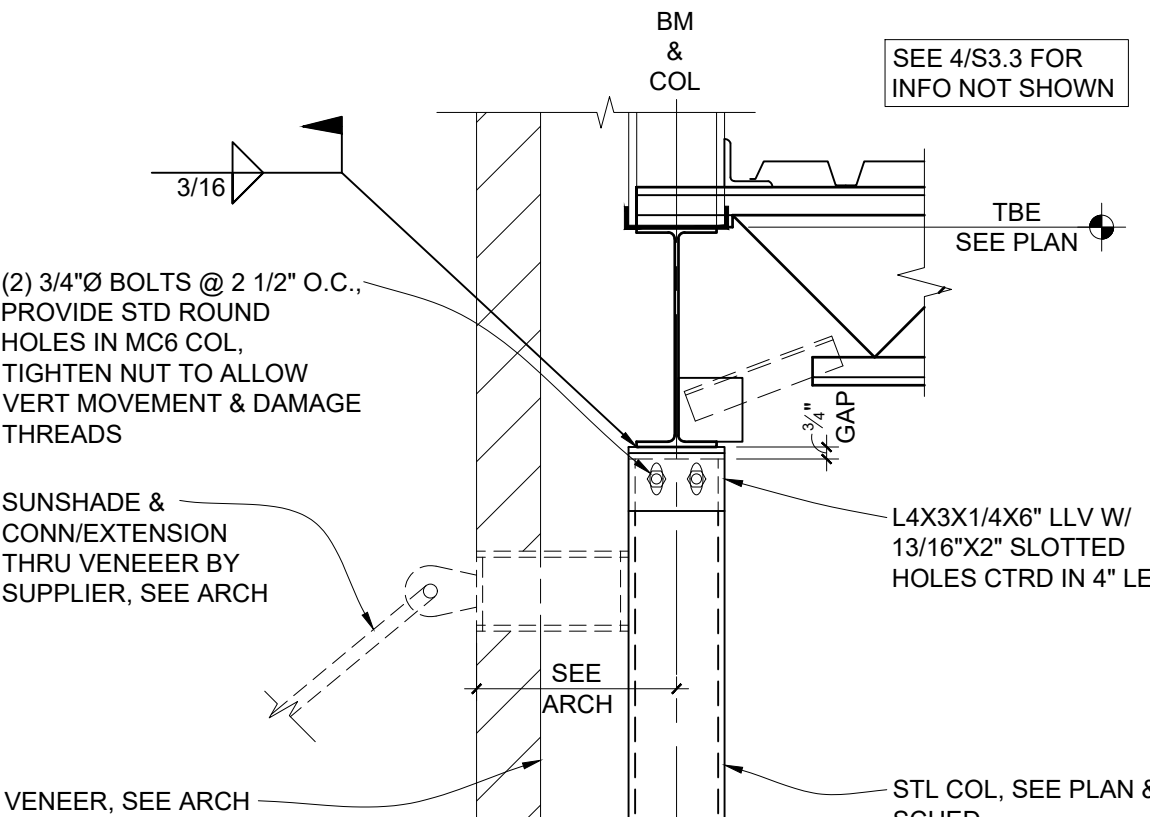
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S3.2 TRASH ENCLOSURE WALL 1/2"=1'-0"



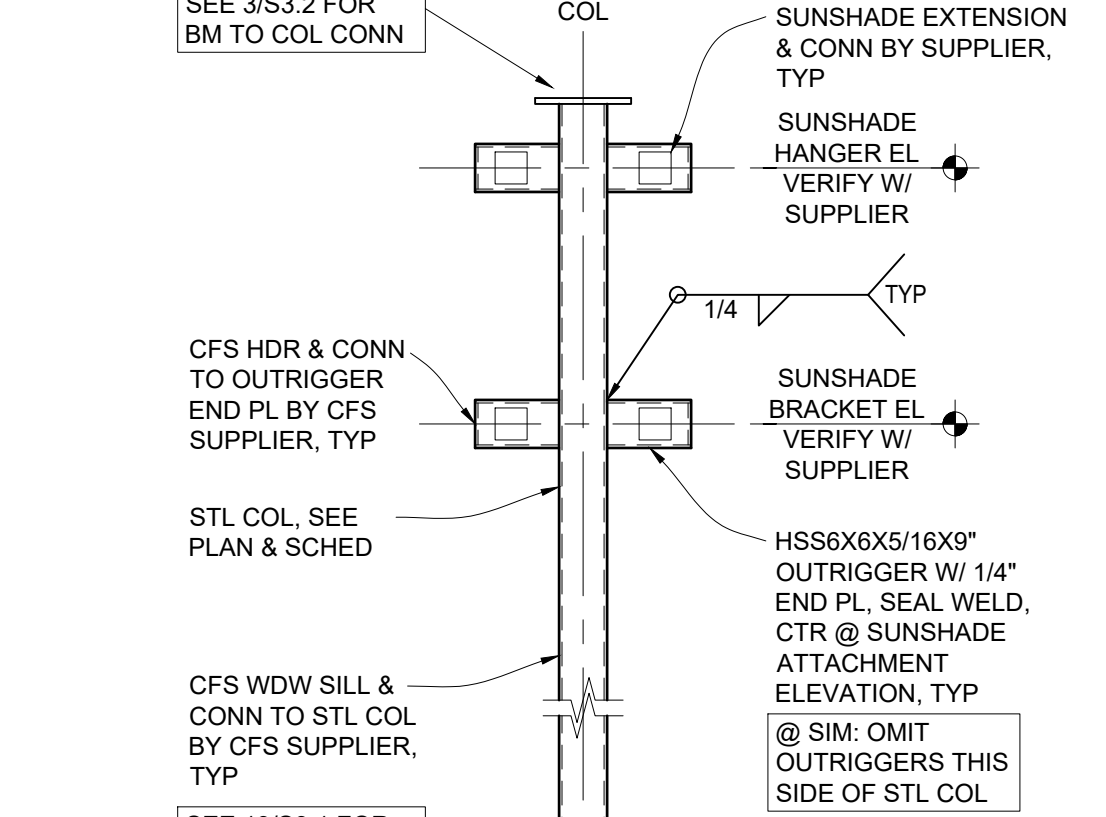
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S3.2 BRICK LTL 1"=1'-0"



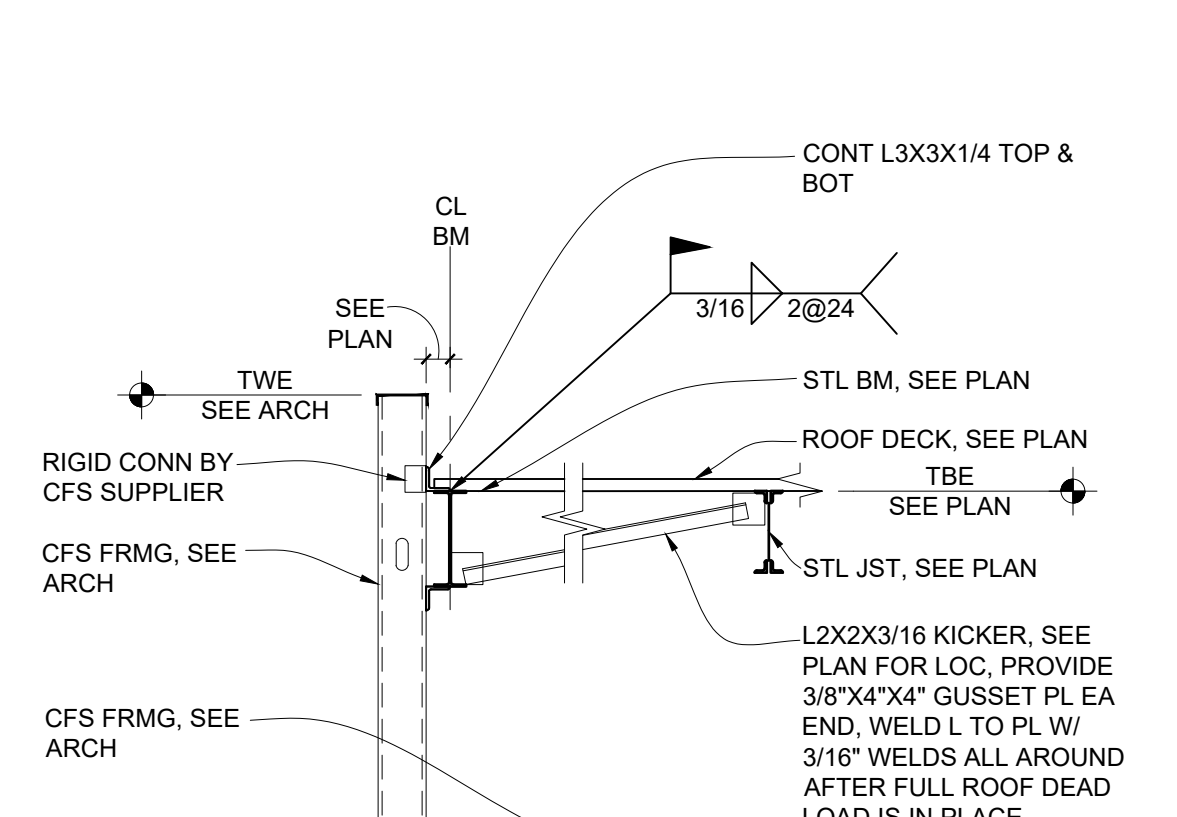
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S3.2 SUNSHADE SUPPORT COL TO FDN WALL CONN 1"=1'-0"



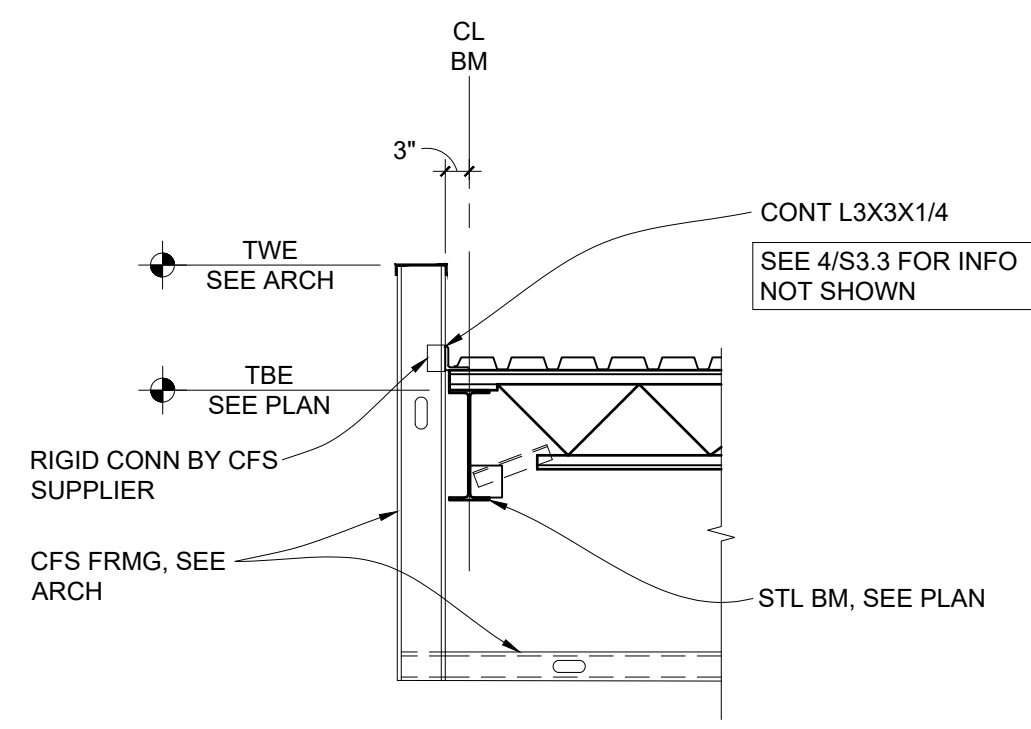
**13 SECTION**  
S3.2 SUNSHADE SUPPORT @ POST 1"=1'-0"



**14 SECTION**  
S3.2 SUNSHADE @ STL COL 1/2"=1'-0"



**15 SECTION**  
S3.2 STL BM @ ROOF EDGE 1/2"=1'-0"



**16 SECTION**  
S3.2 STL JST BRG @ STL BM 1/2"=1'-0"

